THE VIEW OF ROTTERDAM

AFWIJKINGSFORMULIER BYLDIS

Project	VIEW 2 - ROTTERDAM	Nr. 012
Kostenplaats		
Afwijking	**Staalvoorziening EE0101 & SB0101 onbruikbaar door te grote tussenafstand**	
Opsteller	[redacted]	
Datum	**28-08-2025**	
Wijziging	**-**	

Omschrijving afwijking:

Staalvoorziening t.b.v. de 2e draag van EE0101 lijnt niet uit met staalvoorziening in SB0101 waardoor verbinding niet toepasbaar is. Tussenafstand horizontaal tot 38mm.

Oorzaak afwijking:

Afwijking anker posities tijdens productie.

Voorgestelde oplossing/alternatief:

6x Draadeinden M20 diagonaal en 4x draadeind M20 horizontaal door dakrandelement SB0101 boren 200mm inlijmen in EE0101 met HILTI HIT-HY 500 V4, overmaats gat boren Ø32 in SB0101 en draadeind (los) sluiten met stalen buis + sluitring t.b.v. thermische werking van gevelbalk. Alle draadeinden zijn A4-70.

Voor berekening originele verbinding zie SC05.

Consequenties voor de architectonische kwaliteit:

Geen

Consequenties voor de technische kwaliteit en duurzaamheid:

Geen

Consequenties voor de functionele kwaliteit:

Geen

Consequenties voor de uitvoeringsplanning:

Geen

Ondertekening Directievoerder	**Ondertekening Aannemer**
Arcadis	Byldis B.V.
Handtekening [redacted]	Handtekening [redacted]
Naam [redacted]	Naam - [redacted]
Datum 10-09-2025	Datum - 28-08-2025
Plaats Den Bosch	Plaats - Veldhoven

Bijlage(n):

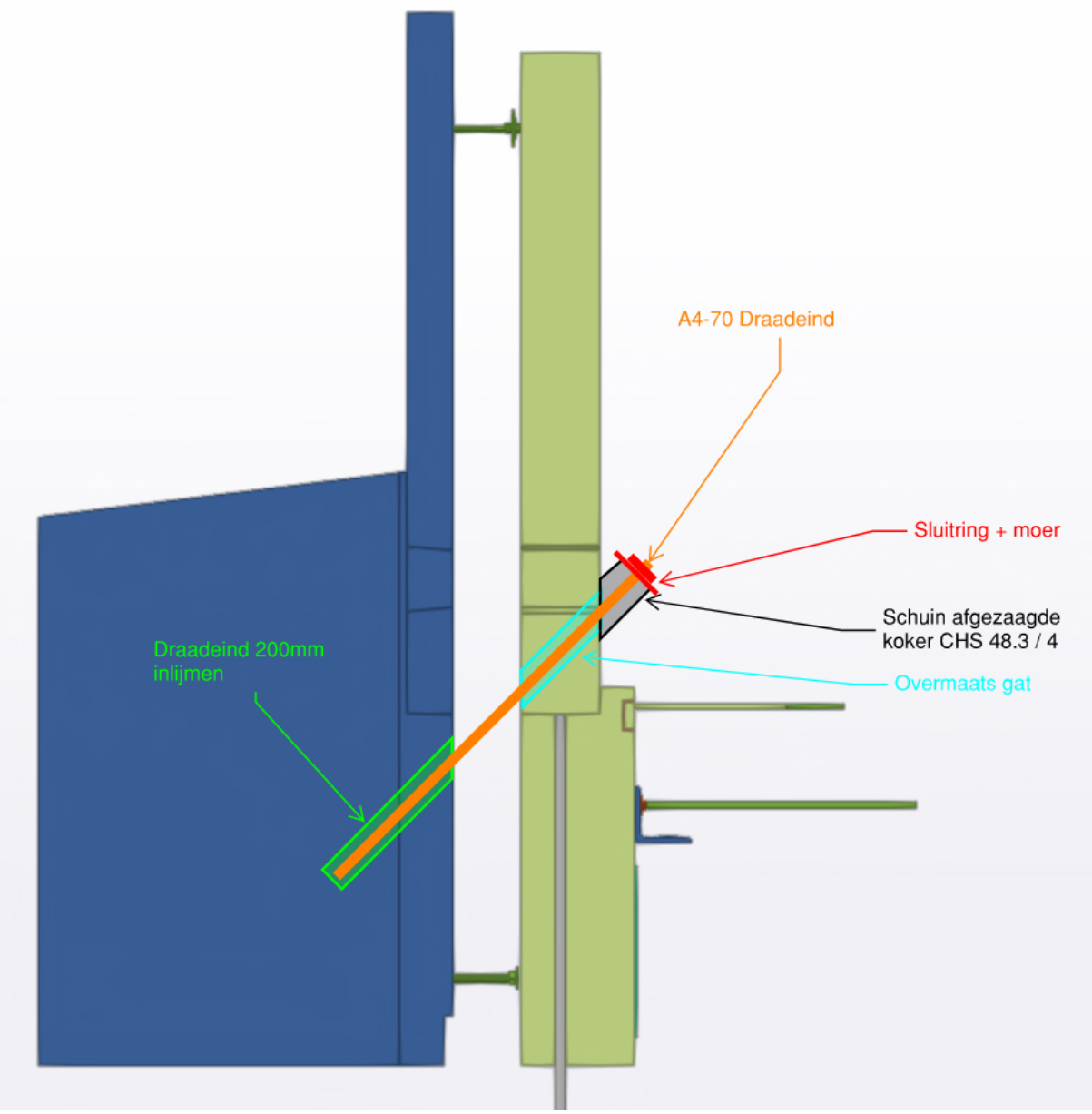
Nr.	Omschrijving	Datum
1	Tekeningen STAL04, BVM06 + BVM09	03-09-25
2	Hilti Uitvoer	25
3	Staafkrachten	25
4	Email toelichting + voorstel oplossing	25
5	SC05 - Berekening Enkelschalige Ementen (verwijzing)	

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10-9-2025

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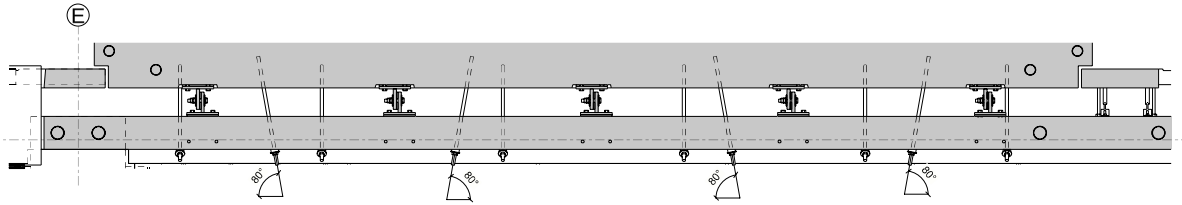
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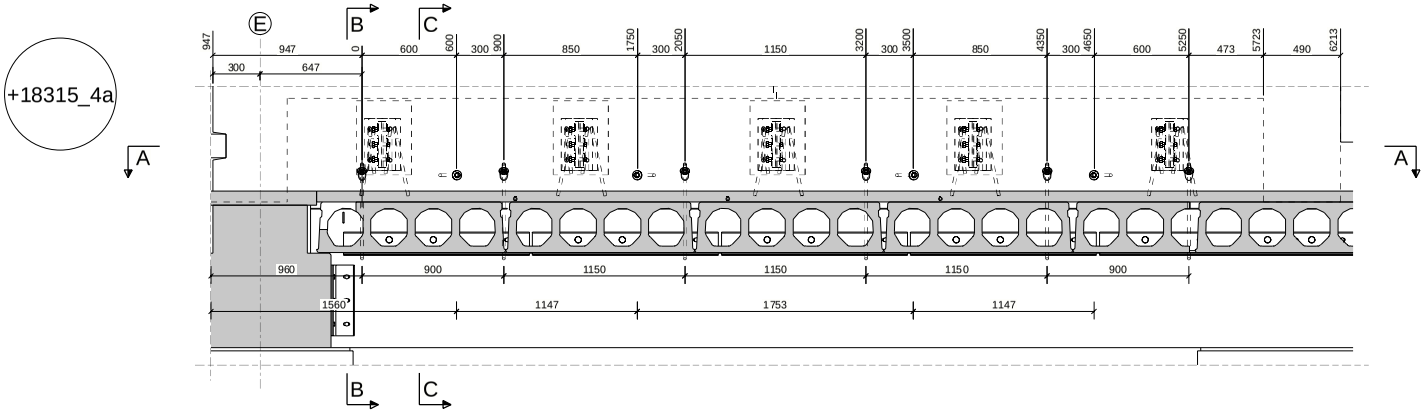
Uitgaande van $\Delta L = \alpha * L_0 * \Delta T = 6.57$mm voor verkorting van elementen.

- $\alpha = 10 * 10^{-6}\ K^{-1}$ (art. 3.1.3 (5) NEN-EN 1992-1-1)
- $L_0 = 16{,}1$ m
- ΔT geeft max(50˚C) = 34.2˚C, min(-25˚C) is 40.8˚C (aanname $T_0 = 15{,}8$˚C op 05-09-25 Rotterdam)

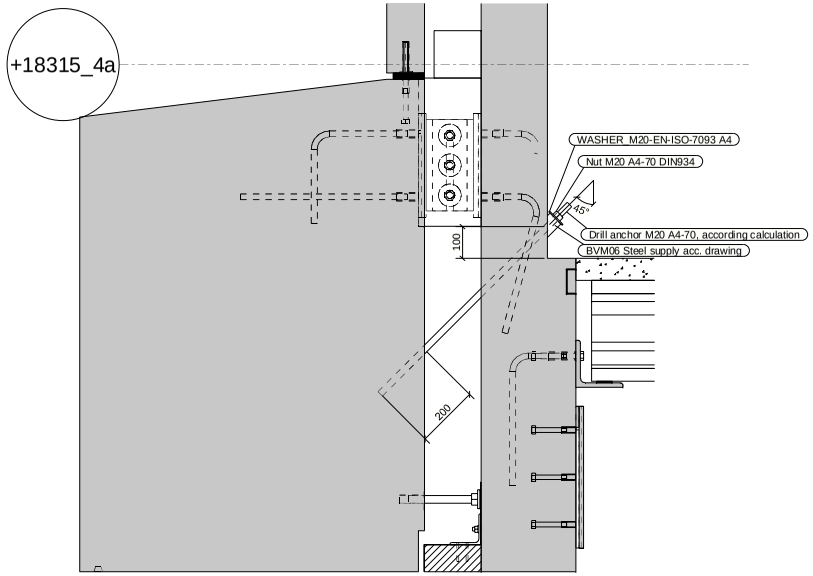
Overmaats gat van Ø32 met M20 draadeind geeft theoretisch 6mm speling, resulterend in een tekort van 0,57mm. Alle gaten zijn geboord met een statief, hierdoor is de passing nauwkeuriger. Door de sluitplaat aantal mm speling te geven t.o.v. het binnenblad kan het draadeind binnen het gat zich bewegen. Alleen bij extreme temperaturen van -25˚C zal het draadeind mogelijk op onderkant gat drukken, echter heeft het draadeind genoeg ruimte zich elastisch te vervormen wanneer genoodzaakt. Hierdoor zal het element nooit gaan trekken aan de dakrand in gebruiksfase, alleen bij het geval van 2e draagweg.



A - A

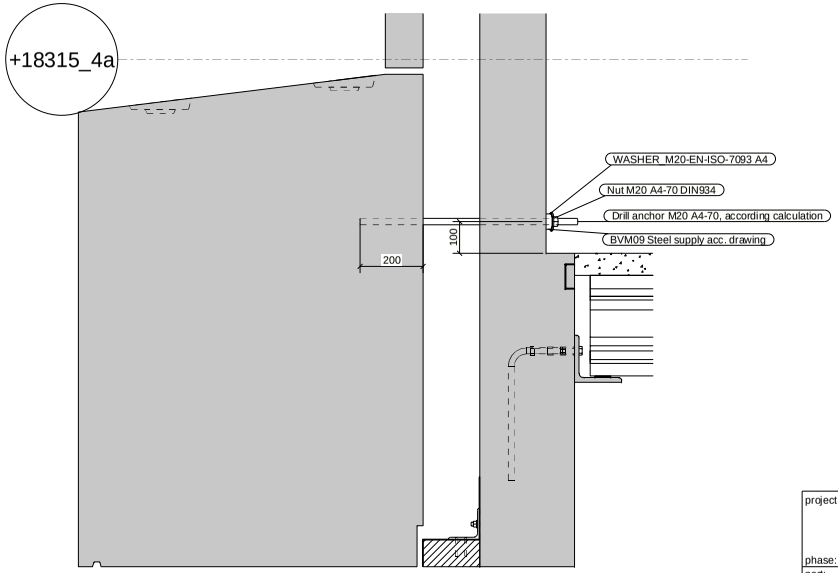


aanzicht binnenzijde wand



B - B

21



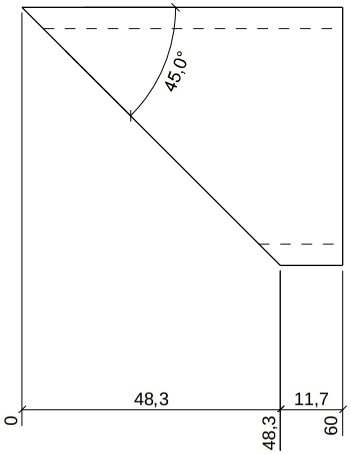
C - C

S = CAST SIDE

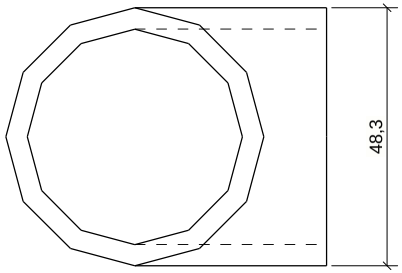
project:	The View II Rotterdam	scale:
phase:	Detailing for manufacturing	author: JPEE
part:	Detail Draadeind 2e draagricihting nivo 4	date: 02-09-25

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project no:	Z0224
sheetnr:	STAL04
state:	Construction



Bovenaanzicht



Vooraanzicht

Regarding:
LDRE- series

STEEL SUPPLY

Execution class according NEN-EN 1090-1: EXC2

Number: 6 x

Finish: Hot Dip Galvanized

Welds: a = - mm (unless stated otherwise)

Welds according to type test: N/A

PARTS PER ASSEMBLY

Pos	Profile	Length	Material	No.	Weight
BVM06	B48.3/4	60	S355J2H	1	0.3
				Total weight:	0.3

project:	The View II Rotterdam	scale:
phase:	Detailing for manufacturing	author: JPEE
part:	Staalvoorziening_BVM-serie	date: 02-09-25

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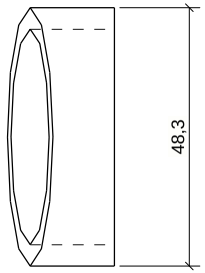
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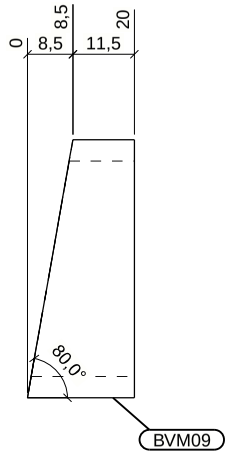
project no: Z0224

sheetnr: BVM06

state: Construction



Bovenaanzicht



Vooraanzicht

Regarding:
LDRE- series

STEEL SUPPLY

Execution class according NEN-EN 1090-1: EXC2

Number: 4 x

Finish: Hot Dip Galvanized

Welds: a = - mm (unless stated otherwise)

Welds according to type test: N/A

PARTS PER ASSEMBLY

Pos	Profile	Length	Material	No.	Weight
BVM09	B48.3/4	20	S355J2H	1	0.1
				Total weight:	0.1

project:	The View II Rotterdam	scale:
phase:	Detailing for manufacturing	author: JPEE
part:	Staalvoorziening_BVM-serie	date: 02-09-25

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project no: Z0224

sheetnr: BVM09

state: Construction

Company:		Page:	1
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - 28 Aug 2025	Date:	28-08-2025
Fastening Point:			

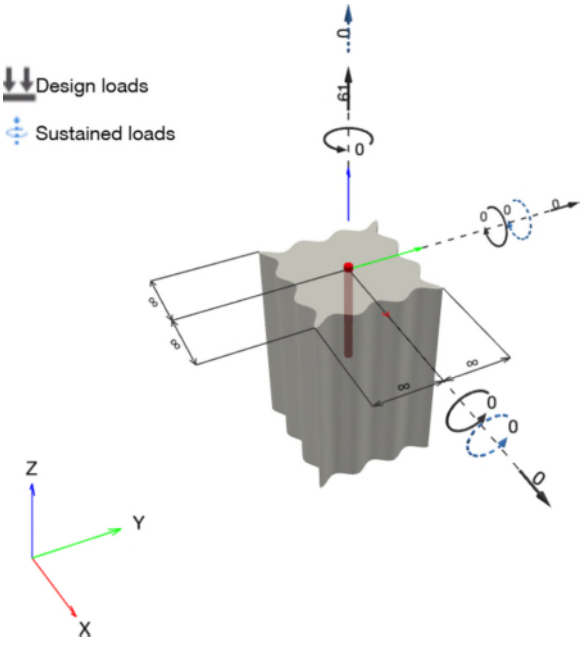
Specifier's comments:

1 Input data



Anchor type and size:	**HIT-RE 500 V4 + HAS-U 8.8 M16**
Return period (service life in years):	50
Item number:	2237090 HAS-U 8.8 M16x260 (insert) / 2287557 HIT-RE 500 V4 (mortar)
Specification text:	Hilti HAS-U 8.8 threaded rod with HIT-RE 500 V4 injection mortar with 200 mm embedment hef, M16, Steel galvanized, Hammer drill bit installation per ETA 20/0541,
Effective embedment depth:	$h_{ef,act}$ = 200.0 mm ($h_{ef,limit}$ = - mm)
Material:	8.8
Approval No.:	ETA 20/0541
Issued I Valid:	09-06-2023 | -
Proof:	Design Method EN 1992-4, Chemical
Stand-off installation:	
Profile:	
Base material:	cracked concrete, C50/60, $f_{c,cyl}$ = 50.00 N/mm^2; h =1,090.0 mm, Temp. short/long: 40/24 °C, partial material safety factor γ_c = 1.500
Installation:	**Hammer drilled hole, Installation condition: Dry**
Reinforcement:	No reinforcement or Reinforcement spacing >= 150 mm (any Ø) or >= 100 mm (Ø <= 10 mm) no longitudinal edge reinforcement Reinforcement to control splitting acc. to EN 1992-4, 7.2.1.7 (2) b) 2) present

Geometry [mm] & Loading [kN, kNm]



Input data and results must be checked for conformity with the existing conditions and for plausibility!
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1.1 Load combination

Case	Description	Forces [kN] / Moments [kNm]	Seismic	Fire	Max. Util. Anchor [%]
1	Combination 1	N = 61.000; V_x = 0.000; V_y = 0.000; M_x = 0.000; M_y = 0.000; M_z = 0.000; N_{sus} = 0.000; $M_{x,sus}$ = 0.000; $M_{y,sus}$ = 0.000;	no	no	76

2 Load case/Resulting anchor forces

Anchor reactions [kN]
Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	61.000	0.000	0.000	0.000

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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3 Tension load (EN 1992-4, Section 7.2.1)

	Load [kN]	Capacity [kN]	Utilization β_N [%]	Status
Steel failure*	61.000	83.733	73	OK
Combined pullout-concrete cone failure**	61.000	80.797	76	OK
Concrete Breakout failure**	61.000	102.667	60	OK
Splitting failure**	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (anchors in tension)

3.1 Steel failure

$$N_{Ed} \leq N_{Rd,s} = \frac{N_{Rk,s}}{\gamma_{Ms}}$$ EN 1992-4, Table 7.1

$N_{Rk,s}$ [kN]	γ_{Ms}	$N_{Rd,s}$ [kN]	N_{Ed} [kN]
125.600	1.500	83.733	61.000

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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3.2 Combined pullout-concrete cone failure

$N_{Ed} \le N_{Rd,p} = \frac{N_{Rk,p}}{\gamma_{Mp}}$ EN 1992-4, Table 7.1

$N_{Rk,p} = N^0_{Rk,p} \cdot \frac{A_{p,N}}{A^0_{p,N}} \cdot \psi_{g,Np} \cdot \psi_{s,Np} \cdot \psi_{re,Np} \cdot \psi_{ec1,Np} \cdot \psi_{ec2,Np}$ EN 1992-4, Eq. (7.13)

$N^0_{Rk,p} = \psi_{sus} \cdot \tau_{Rk} \cdot \pi \cdot d \cdot h_{ef}$ EN 1992-4, Eq. (7.14)

$\psi_{sus} = 1$ EN 1992-4, Eq. (7.14a)

$s_{cr,Np} = 7.3 \cdot d \cdot \sqrt{\psi_{sus} \cdot \tau_{Rk}} \le 3 \cdot h_{ef}$ EN 1992-4, Eq. (7.15)

$\psi_{g,Np} = \psi^0_{g,Np} - \left(\frac{s}{s_{cr,Np}}\right)^{0,5} \cdot \left(\psi^0_{g,Np} - 1\right) \ge 1.00$ EN 1992-4, Eq. (7.17)

$\psi^0_{g,Np} = \sqrt{n} - (\sqrt{n} - 1) \cdot \left(\frac{\tau_{Rk}}{\tau_{Rk,c}}\right)^{1,5} \ge 1.00$ EN 1992-4, Eq. (7.18)

$\tau_{Rk,c} = \frac{k_3}{\pi \cdot d} \cdot \sqrt{h_{ef} \cdot f_{ck}}$ EN 1992-4, Eq. (7.19)

$\psi_{s,Np} = 0.7 + 0.3 \cdot \frac{c}{c_{cr,Np}} \le 1.00$ EN 1992-4, Eq. (7.20)

$\psi_{ec1,Np} = \frac{1}{1 + \left(\frac{2 \cdot e_{c1,N}}{s_{cr,Np}}\right)} \le 1.00$ EN 1992-4, Eq. (7.21)

$\psi_{ec2,Np} = \frac{1}{1 + \left(\frac{2 \cdot e_{c2,N}}{s_{cr,Np}}\right)} \le 1.00$ EN 1992-4, Eq. (7.21)

$A_{p,N}$ [mm²]	$A^0_{p,N}$ [mm²]	$\tau_{Rk,ucr,20}$ [N/mm²]	$s_{cr,Np}$ [mm]	$c_{cr,Np}$ [mm]	c_{min} [mm]	$f_{c,cyl}$ [N/mm²]
231,918	231,918	17.00	481.6	240.8	∞	50.00

ψ_c	$\tau_{Rk,cr}$ [N/mm²]	k_3	$\tau_{Rk,c}$ [N/mm²]	$\psi^0_{g,Np}$	$\psi_{g,Np}$
1.096	12.06	7.700	15.32	1.000	1.000

$e_{c1,N}$ [mm]	$\psi_{ec1,Np}$	$e_{c2,N}$ [mm]	$\psi_{ec2,Np}$	$\psi_{s,Np}$	$\psi_{re,Np}$
0.0	1.000	0.0	1.000	1.000	1.000

ψ^0_{sus}	α_{sus}	ψ_{sus}
0.880	0.000	1.000

$N^0_{Rk,p}$ [kN]	$N_{Rk,p}$ [kN]	γ_{Mp}	$N_{Rd,p}$ [kN]	N_{Ed} [kN]
121.196	121.196	1.500	80.797	61.000

Group anchor ID
1

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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Company:		Page:	5
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - 28 Aug 2025	Date:	28-08-2025
Fastening Point:			

3.3 Concrete Breakout failure

$N_{Ed} \le N_{Rd,c} = \frac{N_{Rk,c}}{\gamma_{Mc}}$ EN 1992-4, Table 7.1

$N_{Rk,c} = N^0_{Rk,c} \cdot \frac{A_{c,N}}{A^0_{c,N}} \cdot \psi_{s,N} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{M,N}$ EN 1992-4, Eq. (7.1)

$N^0_{Rk,c} = k_1 \cdot \sqrt{f_{ck}} \cdot h_{ef}^{1,5}$ EN 1992-4, Eq. (7.2)

$A^0_{c,N} = s_{cr,N} \cdot s_{cr,N}$ EN 1992-4, Eq. (7.3)

$\psi_{s,N} = 0.7 + 0.3 \cdot \frac{c}{c_{cr,N}} \le 1.00$ EN 1992-4, Eq. (7.4)

$\psi_{ec1,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{N,1}}{s_{cr,N}}\right)} \le 1.00$ EN 1992-4, Eq. (7.6)

$\psi_{ec2,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{N,2}}{s_{cr,N}}\right)} \le 1.00$ EN 1992-4, Eq. (7.6)

$\psi_{M,N} = 1$ EN 1992-4, Eq. (7.7)

$A_{c,N}$ [mm²]	$A^0_{c,N}$ [mm²]	$c_{cr,N}$ [mm]	$s_{cr,N}$ [mm]	$f_{c,cyl}$ [N/mm²]
360,000	360,000	300.0	600.0	50.00

$e_{c1,N}$ [mm]	$\psi_{ec1,N}$	$e_{c2,N}$ [mm]	$\psi_{ec2,N}$	$\psi_{s,N}$	$\psi_{re,N}$	z [mm]
0.0	1.000	0.0	1.000	1.000	1.000	0.0

$\psi_{M,N}$	k_1	$N^0_{Rk,c}$ [kN]	γ_{Mc}	$N_{Rd,c}$ [kN]	N_{Ed} [kN]
1.000	7.700	154.000	1.500	102.667	61.000

Group anchor ID
1

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4 Shear load (EN 1992-4, Section 7.2.2)

	Load [kN]	Capacity [kN]	Utilization β_V [%]	Status
Steel failure (without lever arm)*	N/A	N/A	N/A	N/A
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout failure*	N/A	N/A	N/A	N/A
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (relevant anchors)
When the input edge distance is set to "infinity", edge breakout verification is not performed in that direction

5 Displacements (highest loaded anchor)

Short term loading:

N_{Sk} = 45.185 [kN] δ_N = 0.3596 [mm]

V_{Sk} = 0.000 [kN] δ_V = 0.0000 [mm]

δ_{NV} = 0.3596 [mm]

Long term loading:

N_{Sk} = 45.185 [kN] δ_N = 0.8540 [mm]

V_{Sk} = 0.000 [kN] δ_V = 0.0000 [mm]

δ_{NV} = 0.8540 [mm]

Comments: Tension displacements are valid with half of the required installation torque moment for uncracked concrete! Shear displacements are valid without friction between the concrete and the baseplate! The gap due to the drilled hole and clearance hole tolerances are not included in this calculation!

The acceptable anchor displacements depend on the fastened construction and must be defined by the designer!

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6 Warnings

- The anchor design methods in PROFIS Engineering require rigid baseplates per current regulations (AS 5216:2021, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the baseplate are not considered - the baseplate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required baseplate thickness with CBFEM to limit the stress of the baseplate based on the assumptions explained above. The proof if the rigid baseplate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- The equations presented in this report are based on metric units. When inputs are displayed in imperial units, the user should be aware that the equations remain in their metric format.
- Checking the transfer of loads into the base material is required in accordance with EN 1992-4, Annex A!
- The design is only valid if the clearance hole in the fixture is not larger than the value given in Table 6.1 of EN 1992-4! For larger diameters of the clearance hole see section 6.2.2 of EN 1992-4!
- The accessory list in this report is for the information of the user only. In any case, the instructions for use provided with the product have to be followed to ensure a proper installation.
- For the determination of the $\psi_{re,v}$ (concrete edge failure) the minimum concrete cover defined in the design settings is used as the concrete cover of the edge reinforcement.
- Characteristic bond resistances depend on short- and long-term temperatures.
- Edge reinforcement is not required to avoid splitting failure
- Load transfer from supplementary reinforcement to the structural member shall be verified by the responsible structural engineer.
- With supplementary reinforcement and post-installed anchors, please ensure that in the jobsite the rebars are not drilled through.
- The characteristic bond resistances depend on the return period (service life in years): 50

Fastening meets the design criteria!

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7 Installation data

Baseplate, steel: -

Profile: -

Hole diameter in the fixture: -

Plate thickness (input): -

Drilling method: Hammer drilled
Cleaning: Compressed air cleaning of the drilled hole according to instructions for use is required

Anchor type and size: HIT-RE 500 V4 + HAS-U 8.8 M16
Item number: 2237090 HAS-U 8.8 M16x260 (insert) / 2287557 HIT-RE 500 V4 (mortar)

Maximum installation torque: 80 Nm

Hole diameter in the base material: 18.0 mm

Hole depth in the base material: 200.0 mm

Minimum thickness of the base material: 236.0 mm

Hilti HAS-U 8.8 threaded rod with HIT-RE 500 V4 injection mortar with 200 mm embedment hef, M16, Steel galvanized, Hammer drill bit installation per ETA 20/0541

7.1 Recommended accessories

Drilling	Cleaning	Setting
• Suitable Rotary Hammer • Properly sized drill bit	• Compressed air with required accessories to blow from the bottom of the hole • Proper diameter wire brush	• Dispenser including cassette and mixer • Torque wrench

Coordinates Anchor [mm]

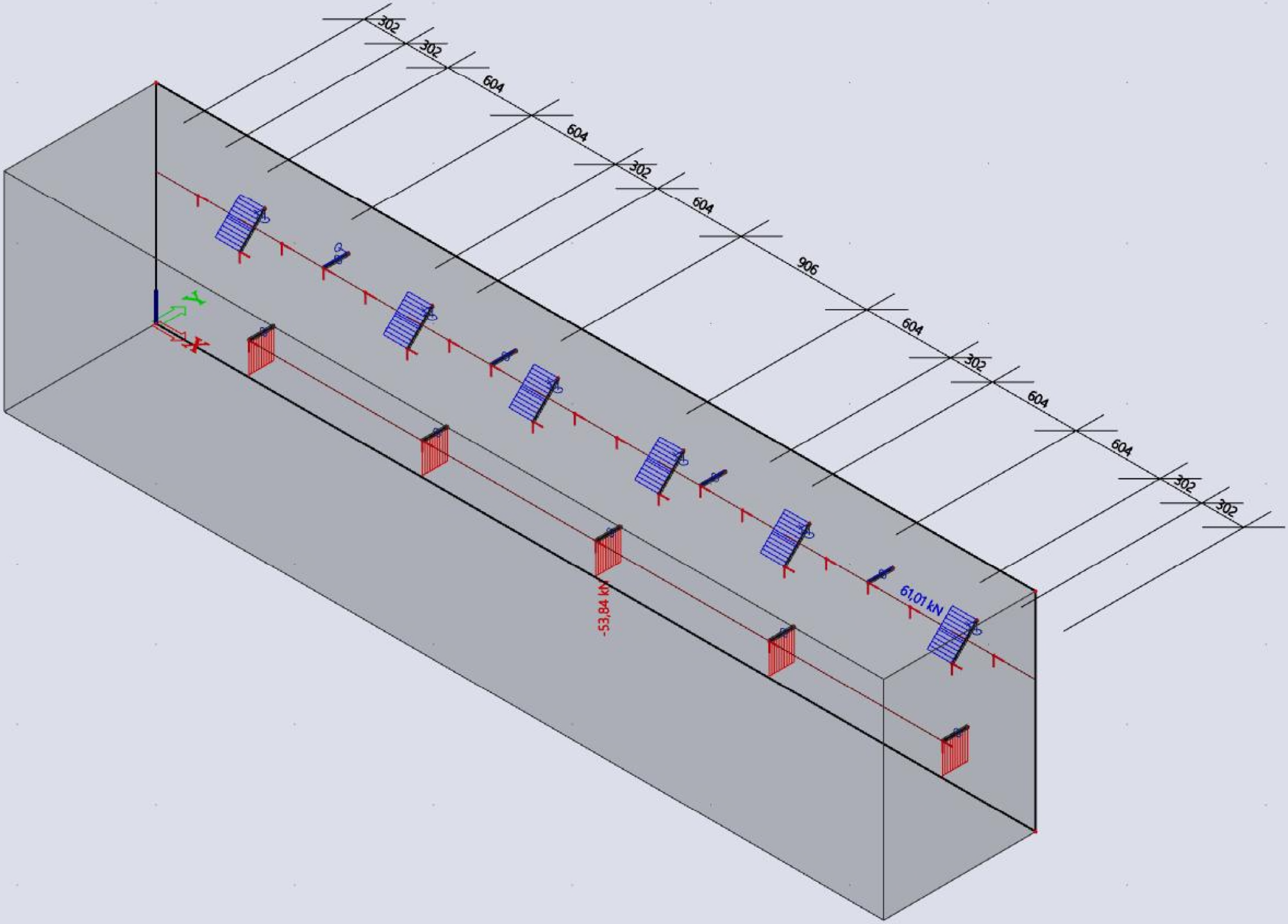
Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	0.0	0.0	-	-	-	-

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906
604
302
604
604
302
302
61,01 kN
-53,84 kN

[redacted]

From: [redacted]
Sent: woensdag 3 september 2025 14:36
To: [redacted]
Cc:
Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

Hallo [redacted]

De staalvoorziening van de onderste kolom is ontworpen om het hele gewicht te dragen in eindsituatie, de overige staalvoorzieningen zijn t.b.v. de 2e draagweg en staan temperatuur zettingen toe d.m.v. slobgaten.
Doordat er een overmaats gat wordt geboord in het binnenblad kan de balk zowel naar boven uitzetten als naar onder krimpen binnen dit overmaatse gat zonder krachten over te dragen aan het binnenblad.

Daarnaast kan de hoofd draagconstructie ook wel deze krachten op opvangen omdat de maatgevende krachten vanuit de andere as W01 zijn toegepast, dit is een significant zwaardere balk 36,5 ton t.o.v. 25,5 ton op de betreffende as 21.

Met vriendelijke groet / Kind regards,

[redacted]

BYLDIS

T +31 (0)88 134 50 00
[redacted]

Locht 126
5504 RP Veldhoven
The Netherlands

www.byldis.com

From: [redacted]
Sent: woensdag 3 september 2025 09:49
To: [redacted]
Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

Beste [redacted]

Dit is voor mij wel te volgen, maar wat gebeurd er als de temperatuur daalt en de kolom wordt korter?

Krijg je dan geen schade aan bovenzijde, of is deze kracht op te nemen door de hoofddraagconstructie.

Met vriendelijke groeten,

[redacted]
Senior Designer Engineer
Arcadis Nederland B.V.
Weena 505 | 3013 AL Rotterdam | The Netherlands
[redacted]
www.arcadis.com

[redacted]

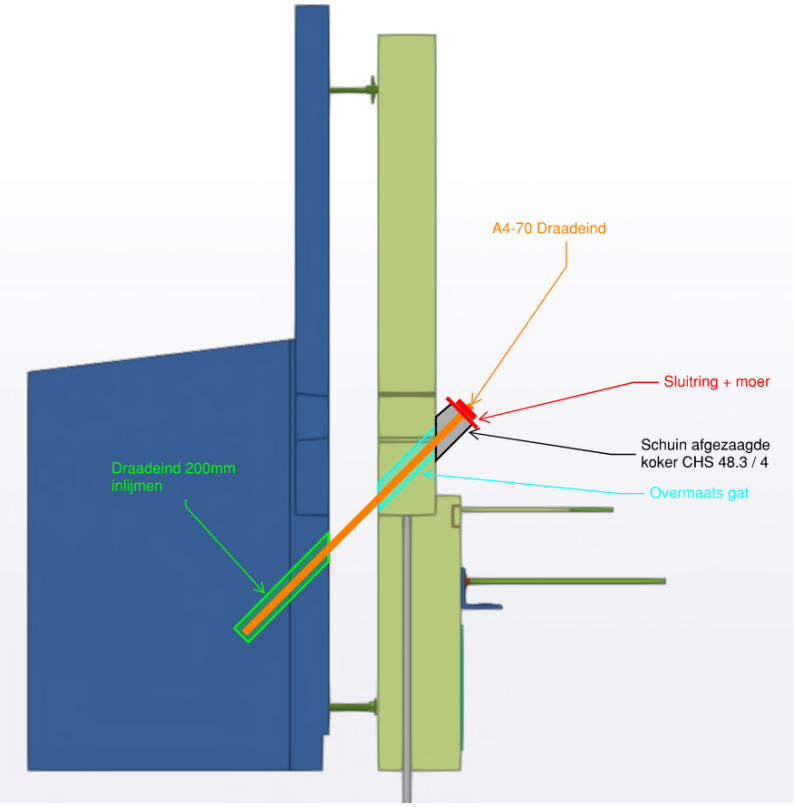
Arcadis Warning: Exercise caution with email messages from external sources such as this message. Always verify the sender and avoid clicking on links or scanning QR codes unless certain of their authenticity.

Hallo [redacted]

Zoals besproken zullen wij ervoor zorgen dat er boorzones aangehouden worden waarbij geen wapening geraakt zal worden. Ook worden er RVS A4-70, minimaal M20, draadeinden toegepast.

Betreft de thermische werking van de verbinding stellen wij het volgende voor:

- Overmaats gat Ø32 in binnenblad, geeft verticaal en horizontaal vrijheid tot uitzetten
- Schuin afgezaagde koker + sluitring + moer om draadeind onder 45 graden te bevestigen op binnenblad, geeft ook diagonaal vrijheid tot uitzetten
- Verbinding werkt alleen in trek en niet meer in druk, zoals bedoeld



Graag ontvangen wij z.s.m. een reactie op het voorstel.

Met vriendelijke groet / Kind regards,

[redacted]



[redacted]

Locht 126
5504 RP Veldhoven
The Netherlands

www.byldis.com



[redacted]

Hoi [redacted]

Ik had hier nog een paar vragen over. Gisteren al telefonisch kort besproken.

*niet doorboren van essentiele wapening.
*RVS toepassen ivm oncontroleerbaarheid spouw.

Daarnaast is mijn vraag:

*Gaat de diagonale inlijming geen problemen opleveren ivm thermische werking van het buienblad tov het binnenblad

Met vriendelijke groeten,

[redacted]

[redacted]

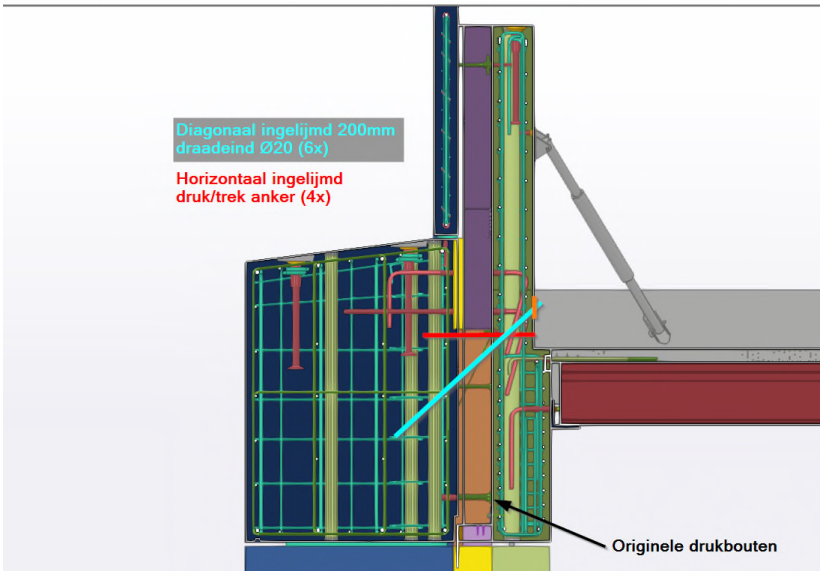
Subject: The View II - 2e draagweg voorziening portaalbalk EE0101
Importance: High

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Hallo [redacted]

Als afsluiter van de montage lopen we tegen een nieuwe uitdaging aan wat betreft de stalen t-profielen welke de enkelschalige portaalbalk EE0101 verbindt met het dakrandelement SB0101 t.b.v. de 2e draagweg.
Na het monteren van de balk blijkt een tussenafstand tot 38mm te zijn ontstaan, dit is uiteraard niet te overbruggen of op te nemen door de bouten welke deze staalplaten moeten verbinden, zie afbeelding.

Er zal een alternatieve verbinding moeten worden toegepast t.b.v. 2e draagweg omdat deze staalplaten nu onbruikbaar zijn.
Ons voorstel is om diagonaal draadeinden door dakrand SB0101 te boren en in EE0101 in te lijmen en met een sluitplaat aan SB0101 op te hangen. Gecombineerd met horizontale draadeinden t.b.v. de druk/trek welke ingelijmd worden in het dakrandelement SB0101. Bijgevoegd de inlijmcontrole van de Ø20 diagonale draadeind met Hilti HIT-HY 500 V4.
De krachten op deze verbinding zijn lager dan in de berekening van Dakranden – M01 omdat hier maatgevend as W01 is aangehouden waar de balk op as 21 significant lichter is. Hiermee zal het dakrand element nog steeds ruim voldoen.
Horizontaal 53,84 < 54kN en verticaal 61,01kN < 78,8kN



Graag horen wij snel of deze oplossingsrichting tot een akkoord kan leiden zodat we alvast de benodigdheden kunnen regelen.

Met vriendelijke groet / Kind regards,

[redacted]

[redacted]
Locht 126
5504 RP Veldhoven
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