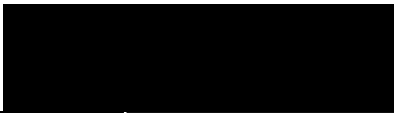
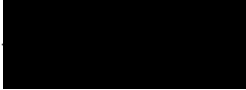
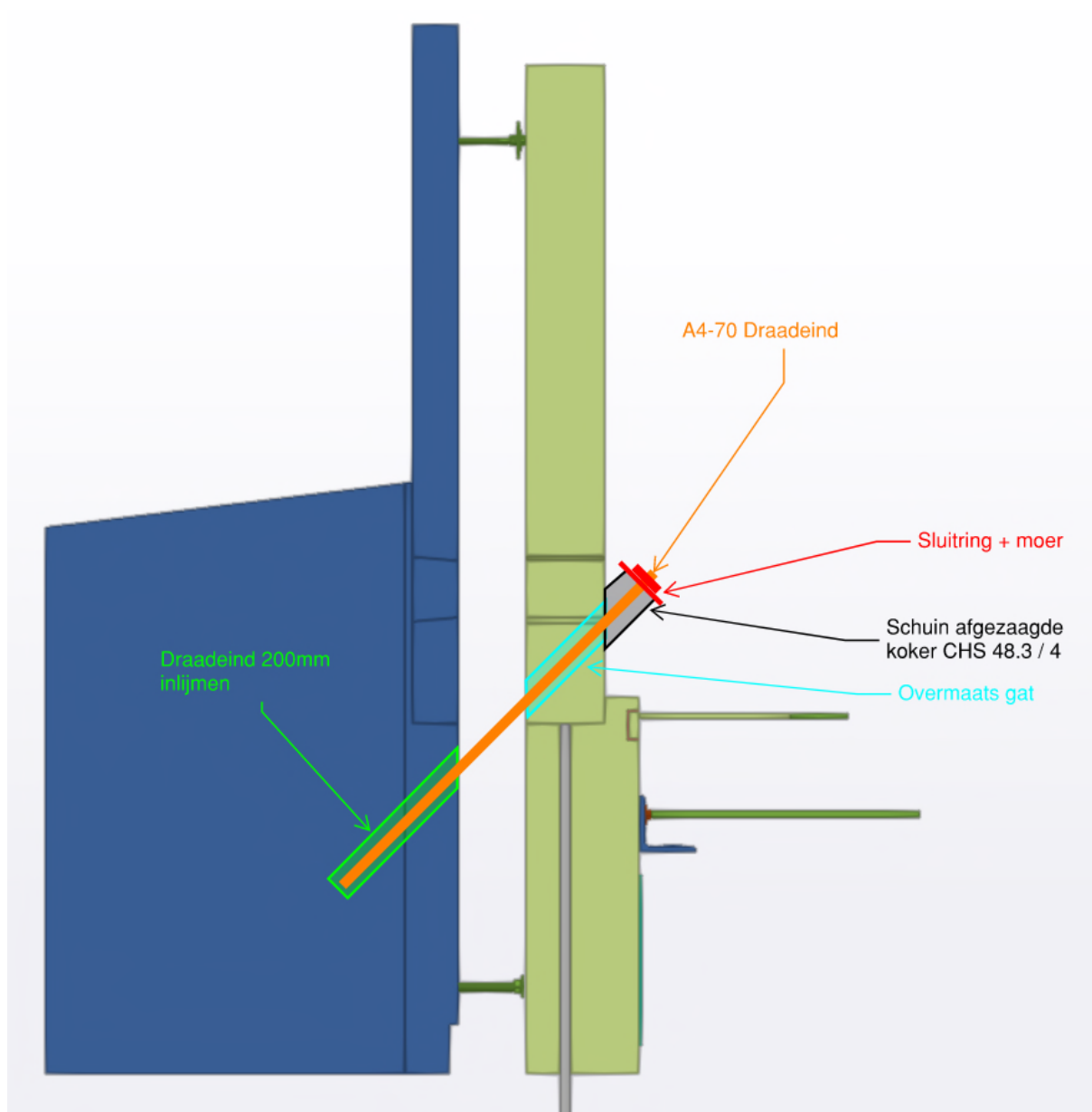


AFWIJKINGSFORMULIER BYLDIS

Project	VIEW 2 - ROTTERDAM	Nr.	012
Kostenplaats			
Afwijking	Staalvoorziening EE0101 & SB0101 onbruikbaar door te grote tussenafstand		
Opsteller			
Datum	28-08-2025		
Wijziging	-		
Omschrijving afwijking:			
Staalvoorziening t.b.v. de 2 ^e draag van EE0101 lijnt niet uit met staalvoorziening in SB0101 waardoor verbinding niet toepasbaar is. Tussenafstand horizontaal tot 38mm.			
Oorzaak afwijking:			
Afwijking anker posities tijdens productie.			
Voorgestelde oplossing/alternatief:			
6x Draadeinden M20 diagonaal en 4x draadeind M20 horizontaal door dakrandelement SB0101 boren 200mm inlijmen in EE0101 met HILTI HIT-HY 500 V4, overmaats gat boren Ø32 in SB0101 en draadeind (los) sluiten met stalen buis + sluitring t.b.v. thermische werking van gevelbalk. Alle draadeinden zijn A4-70. Voor berekening originele verbinding zie SC05.			
Consequenties voor de architectonische kwaliteit:			
Geen			
Consequenties voor de technische kwaliteit en duurzaamheid:			
Geen			
Consequenties voor de functionele kwaliteit:			
Geen			
Consequenties voor de uitvoeringsplanning:			
Geen			
Ondertekening Directievoerder		Ondertekening Aannemer	
Arcadis		Byldis B.V.	
Handtekening 		Handtekening 	
Naam		Naam -	
Datum		Datum - 28-08-2025	
Plaats		Plaats - Veldhoven	
Bijlage(n):			
Nr.	Omschrijving		Datum
1	Tekeningen STAL04, BVM06 + BVM09		03-09-25
2	Hilti Uitvoer		25
3	Staafrachten		25
4	Email toelichting + voorstel oplossing		25
5	SC05 - Berekening Enkelschalige Ementen (verwijzing)		

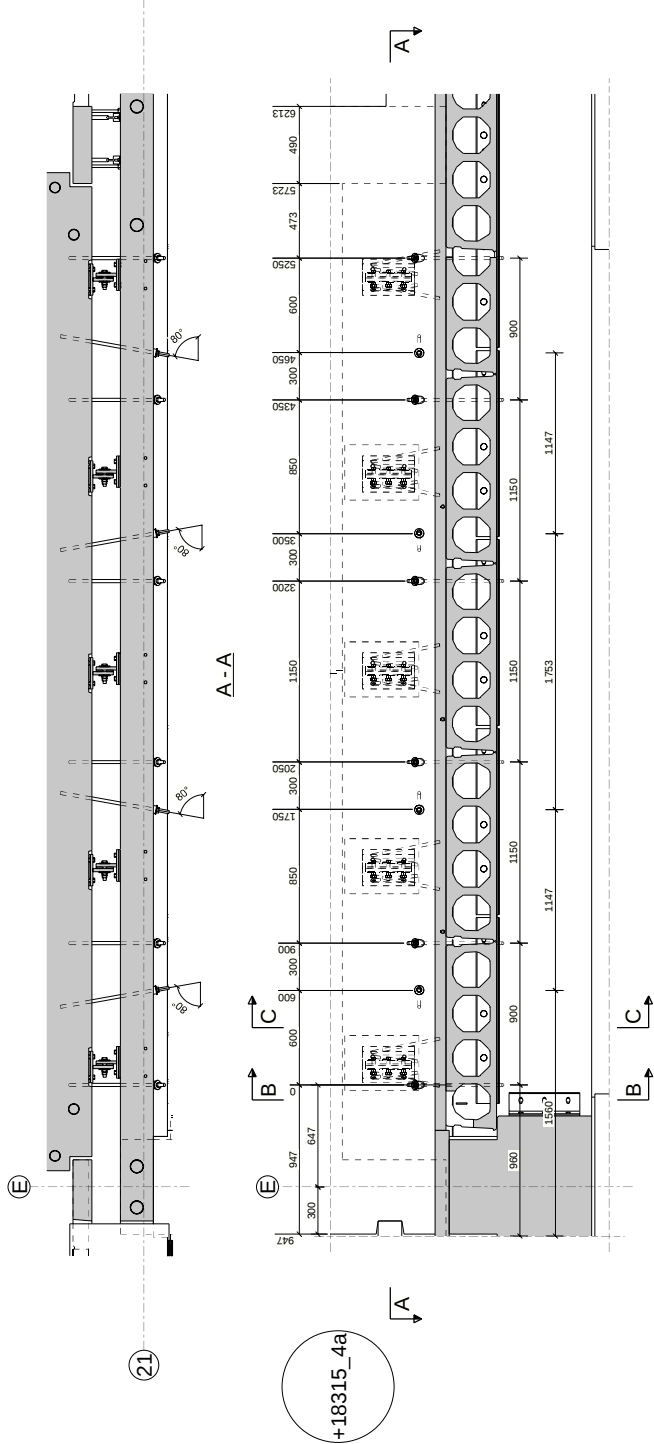




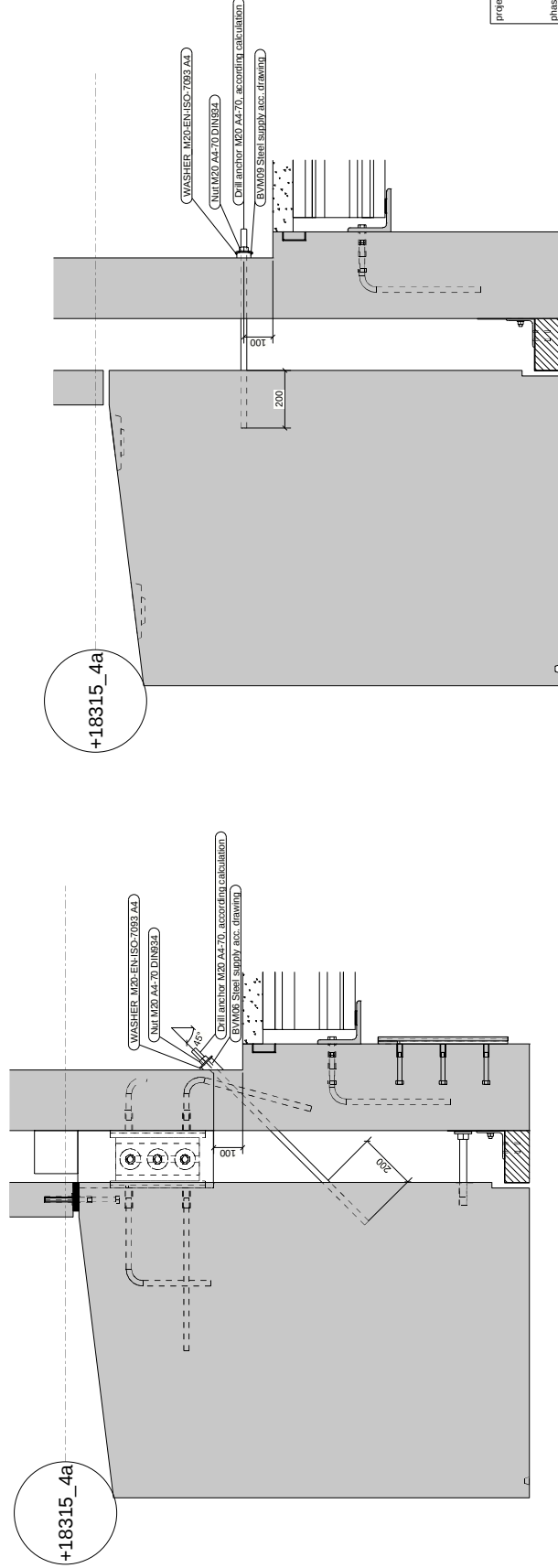
Uitgaande van $\Delta L = \alpha \cdot L_0 \cdot \Delta T = 6.57\text{mm}$ voor verkorting van elementen.

- $\alpha = 10 \cdot 10^{-6} \text{ K}^{-1}$ (art. 3.1.3 (5) NEN-EN 1992-1-1)
- $L_0 = 16,1 \text{ m}$
- ΔT geeft $\text{max}(50^\circ\text{C}) = 34.2^\circ\text{C}$, $\text{min}(-25^\circ\text{C})$ is 40.8°C (aanname $T_0 = 15,8^\circ\text{C}$ op 05-09-25 Rotterdam)

Overmaats gat van $\varnothing 32$ met M20 draadeind geeft theoretisch 6mm speling, resulterend in een tekort van 0,57mm. Alle gaten zijn geboord met een statief, hierdoor is de passing nauwkeuriger. Door de sluitplaat aantal mm speling te geven t.o.v. het binnenblad kan het draadeind binnen het gat zich bewegen. Alleen bij extreme temperaturen van -25°C zal het draadeind mogelijk op onderkant gat drukken, echter heeft het draadeind genoeg ruimte zich elastisch te vervormen wanneer genoodzaakt. Hierdoor zal het element nooit gaan trekken aan de dakrand in gebruiksfase, alleen bij het geval van 2^e draagweg.



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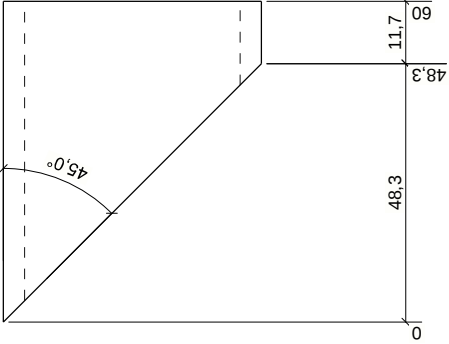
B - B

C - C

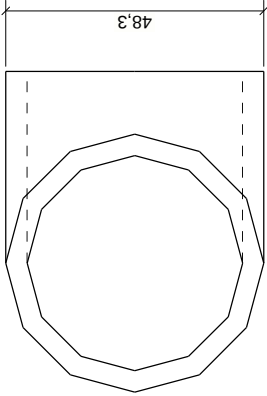
1/2 = CAST SIDE

project:	The View II Rotterdam	scale:	
phase:	Detailing for manufacturing	author:	
part:	Detail Draaieind 2e draagriching nivo 4	date:	02-09-25
project no:	1001 136	project no:	Z0224
sheet no:	5504 RP Veldhoven	sheet no:	STAL04
state:	The Netherlands www.byldis.com	state:	Construction
	T +31 (0)98 134 50 00		
	E info@byldis.com		





Bovenaanzicht

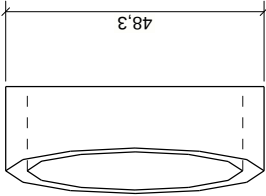


Vooraanzicht

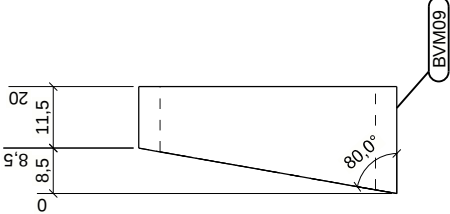
Regarding:
LDRE-series

STEEL SUPPLY			
Execution class according NEN-EN 1090-1: EXC2			
Number: 6 X			
Finish: Hot Dip Galvanized			
Welds: a = - mm (unless stated otherwise)			
Welds according to type test: N/A			
PARTS PER ASSEMBLY			
Pos	Length	Material	No.
BVM06	60	S355J2H	1
Profile			Weight
B48.3/4			0.3
Total weight:			0.3

project:	The View II Rotterdam		phase: Detailing for manufacturing	part: Staalvoorziening_BVM-serie	project no:	Z0224	scale:	
					sheetnr:	BVM06	author:	██████████
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Bovenaanzicht



Voorraanzicht

Regarding:
LDRE-series

STEEL SUPPLY				
Execution class according NEN-EN 1090-1: EXC2				
Number: 4 X				
Finish: Hot Dip Galvanized				
Welds: a = - mm (unless stated otherwise)				
Welds according to type test: N/A				
PARTS PER ASSEMBLY				
Pos	Length	Material	No	Weight
BVM09	20	S355J2H	1	0.1
Profile				Total weight:
B48.3/4				0.1

project:	The View II		scale:
	Rotterdam		
phase:	Detailing for manufacturing		author:
part:	Staalvoorziening_BVM-serie		date: 02-09-25
 Locht 126 5504 RP Veldhoven The Netherlands www.byldis.com T +31 (0)88 134 50 00 E info@byldis.com			project no: Z0224
			sheetnr: BVM09
			state: Construction

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Design: Concrete - 28 Aug 2025
Fastening Point:

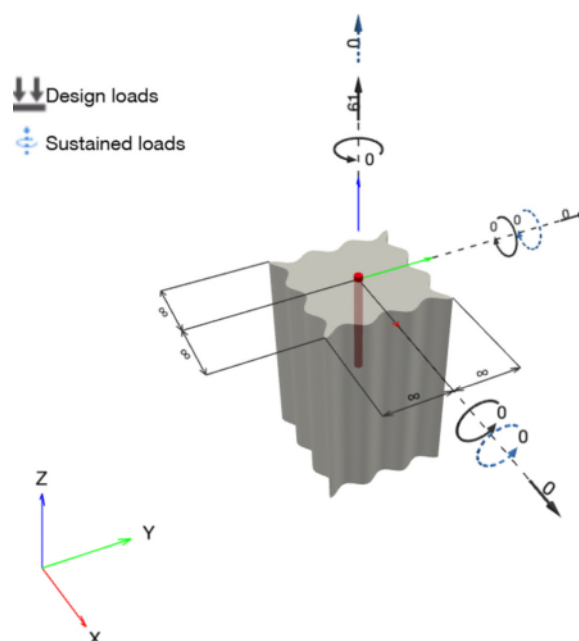
Page: 1
Specifier:
E-Mail:
Date: 28-08-2025

Specifier's comments:

1 Input data

Anchor type and size:	HIT-RE 500 V4 + HAS-U 8.8 M16	
Return period (service life in years):	50	
Item number:	2237090 HAS-U 8.8 M16x260 (insert) / 2287557 HIT-RE 500 V4 (mortar)	
Specification text:	Hilti HAS-U 8.8 threaded rod with HIT-RE 500 V4 injection mortar with 200 mm embedment hef, M16, Steel galvanized, Hammer drill bit installation per ETA 20/0541,	
Effective embedment depth:	$h_{ef,act} = 200.0 \text{ mm}$ ($h_{ef,limit} = - \text{mm}$)	
Material:	8.8	
Approval No.:	ETA 20/0541	
Issued Valid:	09-06-2023 -	
Proof:	Design Method EN 1992-4, Chemical	
Stand-off installation:		
Profile:		
Base material:	cracked concrete, C50/60, $f_{c,cyl} = 50.00 \text{ N/mm}^2$; $h = 1,090.0 \text{ mm}$, Temp. short/long: 40/24 °C, partial material safety factor $\gamma_c = 1.500$	
Installation:	Hammer drilled hole, Installation condition: Dry	
Reinforcement:	No reinforcement or Reinforcement spacing $\geq 150 \text{ mm}$ (any $\emptyset$) or $\geq 100 \text{ mm}$ ($\emptyset \leq 10 \text{ mm}$) no longitudinal edge reinforcement Reinforcement to control splitting acc. to EN 1992-4, 7.2.1.7 (2) b) 2) present	

Geometry [mm] & Loading [kN, kNm]



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1.1 Load combination

Case	Description	Forces [kN] / Moments [kNm]	Seismic	Fire	Max. Util. Anchor [%]
1	Combination 1	N = 61.000; $V_x = 0.000$; $V_y = 0.000$; $M_x = 0.000$; $M_y = 0.000$; $M_z = 0.000$; $N_{sUS} = 0.000$; $M_{x,sUS} = 0.000$; $M_{y,sUS} = 0.000$;	no	no	76

2 Load case/Resulting anchor forces

Anchor reactions [kN]

Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	61.000	0.000	0.000	0.000

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Fastening Point:			

3 Tension load (EN 1992-4, Section 7.2.1)

	Load [kN]	Capacity [kN]	Utilization β_N [%]	Status
Steel failure*	61.000	83.733	73	OK
Combined pullout-concrete cone failure**	61.000	80.797	76	OK
Concrete Breakout failure**	61.000	102.667	60	OK
Splitting failure**	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (anchors in tension)

3.1 Steel failure

$$N_{Ed} \leq N_{Rd,s} = \frac{N_{Rk,s}}{\gamma_{Ms}} \quad \text{EN 1992-4, Table 7.1}$$

$N_{Rk,s}$ [kN]	γ_{Ms}	$N_{Rd,s}$ [kN]	N_{Ed} [kN]
125.600	1.500	83.733	61.000

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3.2 Combined pullout-concrete cone failure

$$N_{Ed} \leq N_{Rd,p} = \frac{N_{Rk,p}}{\gamma_{Mp}} \quad \text{EN 1992-4, Table 7.1}$$

$$N_{Rk,p} = N_{Rk,p}^0 \cdot \frac{A_{p,N}}{A_{p,N}^0} \cdot \psi_{g,Np} \cdot \psi_{s,Np} \cdot \psi_{re,Np} \cdot \psi_{ec1,Np} \cdot \psi_{ec2,Np} \quad \text{EN 1992-4, Eq. (7.13)}$$

$$N_{Rk,p}^0 = \psi_{sus} \cdot \tau_{Rk} \cdot \pi \cdot d \cdot h_{ef} \quad \text{EN 1992-4, Eq. (7.14)}$$

$$\psi_{sus} = 1 \quad \text{EN 1992-4, Eq. (7.14a)}$$

$$s_{cr,Np} = 7.3 \cdot d \cdot \sqrt{\psi_{sus} \cdot \tau_{Rk}} \leq 3 \cdot h_{ef} \quad \text{EN 1992-4, Eq. (7.15)}$$

$$\psi_{g,Np} = \psi_{g,Np}^0 \cdot \left(\frac{s}{s_{cr,Np}} \right)^{0.5} \cdot (\psi_{g,Np}^0 - 1) \geq 1.00 \quad \text{EN 1992-4, Eq. (7.17)}$$

$$\psi_{g,Np}^0 = \sqrt{n} - (\sqrt{n} - 1) \cdot \left(\frac{\tau_{Rk}}{\tau_{Rk,c}} \right)^{1.5} \geq 1.00 \quad \text{EN 1992-4, Eq. (7.18)}$$

$$\tau_{Rk,c} = \frac{k_3}{\pi \cdot d} \cdot \sqrt{h_{ef} \cdot f_{ck}} \quad \text{EN 1992-4, Eq. (7.19)}$$

$$\psi_{s,Np} = 0.7 + 0.3 \cdot \frac{c}{c_{cr,Np}} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.20)}$$

$$\psi_{ec1,Np} = \frac{1}{1 + \left(\frac{2 \cdot e_{c1,N}}{s_{cr,Np}} \right)} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.21)}$$

$$\psi_{ec2,Np} = \frac{1}{1 + \left(\frac{2 \cdot e_{c2,N}}{s_{cr,Np}} \right)} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.21)}$$

$A_{p,N}$ [mm ²]	$A_{p,N}^0$ [mm ²]	$\tau_{Rk,ucr,20}$ [N/mm ²]	$s_{cr,Np}$ [mm]	$c_{cr,Np}$ [mm]	c_{min} [mm]	$f_{c,cyl}$ [N/mm ²]
231,918	231,918	17.00	481.6	240.8	∞	50.00
ψ_c	$\tau_{Rk,cr}$ [N/mm ²]	k_3	$\tau_{Rk,c}$ [N/mm ²]	$\psi_{g,Np}^0$	$\psi_{g,Np}$	
1.096	12.06	7.700	15.32	1.000	1.000	
$e_{c1,N}$ [mm]	$\psi_{ec1,Np}$	$e_{c2,N}$ [mm]	$\psi_{ec2,Np}$	$\psi_{s,Np}$	$\psi_{re,Np}$	
0.0	1.000	0.0	1.000	1.000	1.000	
ψ_{sus}^0	α_{sus}	ψ_{sus}				
0.880	0.000	1.000				
$N_{Rk,p}^0$ [kN]	$N_{Rk,p}$ [kN]	γ_{Mp}	$N_{Rd,p}$ [kN]	N_{Ed} [kN]		
121.196	121.196	1.500	80.797	61.000		

Group anchor ID

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3.3 Concrete Breakout failure

$$N_{Ed} \leq N_{Rd,c} = \frac{N_{Rk,c}}{\gamma_{Mc}} \quad \text{EN 1992-4, Table 7.1}$$

$$N_{Rk,c} = N_{Rk,c}^0 \cdot \frac{A_{c,N}}{A_{c,N}^0} \cdot \psi_{s,N} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{M,N} \quad \text{EN 1992-4, Eq. (7.1)}$$

$$N_{Rk,c}^0 = k_1 \cdot \sqrt{f_{ck}} \cdot h_{ef}^{1.5} \quad \text{EN 1992-4, Eq. (7.2)}$$

$$A_{c,N}^0 = s_{cr,N} \cdot s_{cr,N} \quad \text{EN 1992-4, Eq. (7.3)}$$

$$\psi_{s,N} = 0.7 + 0.3 \cdot \frac{c}{c_{cr,N}} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.4)}$$

$$\psi_{ec1,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{N,1}}{s_{cr,N}} \right)} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{ec2,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{N,2}}{s_{cr,N}} \right)} \leq 1.00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{M,N} = 1 \quad \text{EN 1992-4, Eq. (7.7)}$$

$A_{c,N} [\text{mm}^2]$	$A_{c,N}^0 [\text{mm}^2]$	$c_{cr,N} [\text{mm}]$	$s_{cr,N} [\text{mm}]$	$f_{c,cyl} [\text{N/mm}^2]$		
360.000	360.000	300.0	600.0	50.00		
$e_{c1,N} [\text{mm}]$	$\psi_{ec1,N}$	$e_{c2,N} [\text{mm}]$	$\psi_{ec2,N}$	$\psi_{s,N}$	$\psi_{re,N}$	$z [\text{mm}]$
0.0	1.000	0.0	1.000	1.000	1.000	0.0
$\psi_{M,N}$	k_1	$N_{Rk,c}^0 [\text{kN}]$	γ_{Mc}	$N_{Rd,c} [\text{kN}]$	$N_{Ed} [\text{kN}]$	
1.000	7.700	154.000	1.500	102.667	61.000	
Group anchor ID						
1						

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4 Shear load (EN 1992-4, Section 7.2.2)

	Load [kN]	Capacity [kN]	Utilization β_v [%]	Status
Steel failure (without lever arm)*	N/A	N/A	N/A	N/A
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout failure*	N/A	N/A	N/A	N/A
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (relevant anchors)

When the input edge distance is set to "infinity", edge breakout verification is not performed in that direction

5 Displacements (highest loaded anchor)

Short term loading:

N_{Sk}	=	45.185 [kN]	δ_N	=	0.3596 [mm]
V_{Sk}	=	0.000 [kN]	δ_V	=	0.0000 [mm]
			δ_{NV}	=	0.3596 [mm]

Long term loading:

N_{Sk}	=	45.185 [kN]	δ_N	=	0.8540 [mm]
V_{Sk}	=	0.000 [kN]	δ_V	=	0.0000 [mm]
			δ_{NV}	=	0.8540 [mm]

Comments: Tension displacements are valid with half of the required installation torque moment for uncracked concrete! Shear displacements are valid without friction between the concrete and the baseplate! The gap due to the drilled hole and clearance hole tolerances are not included in this calculation!

The acceptable anchor displacements depend on the fastened construction and must be defined by the designer!

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6 Warnings

- The anchor design methods in PROFIS Engineering require rigid baseplates per current regulations (AS 5216:2021, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the baseplate are not considered - the baseplate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required baseplate thickness with CBFEM to limit the stress of the baseplate based on the assumptions explained above. The proof if the rigid baseplate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- The equations presented in this report are based on metric units. When inputs are displayed in imperial units, the user should be aware that the equations remain in their metric format.
- Checking the transfer of loads into the base material is required in accordance with EN 1992-4, Annex A!
- The design is only valid if the clearance hole in the fixture is not larger than the value given in Table 6.1 of EN 1992-4! For larger diameters of the clearance hole see section 6.2.2 of EN 1992-4!
- The accessory list in this report is for the information of the user only. In any case, the instructions for use provided with the product have to be followed to ensure a proper installation.
- For the determination of the $\psi_{re,v}$ (concrete edge failure) the minimum concrete cover defined in the design settings is used as the concrete cover of the edge reinforcement.
- Characteristic bond resistances depend on short- and long-term temperatures.
- Edge reinforcement is not required to avoid splitting failure
- Load transfer from supplementary reinforcement to the structural member shall be verified by the responsible structural engineer.
- With supplementary reinforcement and post-installed anchors, please ensure that in the jobsite the rebars are not drilled through.
- The characteristic bond resistances depend on the return period (service life in years): 50

Fastening meets the design criteria!

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7 Installation data

Baseplate, steel: -

Profile: -

Hole diameter in the fixture: -

Plate thickness (input): -

Drilling method: Hammer drilled

Cleaning: Compressed air cleaning of the drilled hole according to instructions for use is required

Anchor type and size: HIT-RE 500 V4 + HAS-U 8.8 M16

Item number: 2237090 HAS-U 8.8 M16x260 (insert) / 2287557 HIT-RE 500 V4 (mortar)

Maximum installation torque: 80 Nm

Hole diameter in the base material: 18.0 mm

Hole depth in the base material: 200.0 mm

Minimum thickness of the base material: 236.0 mm

Hilti HAS-U 8.8 threaded rod with HIT-RE 500 V4 injection mortar with 200 mm embedment hef, M16, Steel galvanized, Hammer drill bit installation per ETA 20/0541

7.1 Recommended accessories

Drilling	Cleaning	Setting
- Suitable Rotary Hammer - Properly sized drill bit	- Compressed air with required accessories to blow from the bottom of the hole - Proper diameter wire brush	- Dispenser including cassette and mixer - Torque wrench

Coordinates Anchor [mm]

Anchor	x	y	c _{-x}	c _{+x}	c _{-y}	c _{+y}
1	0.0	0.0	-	-	-	-

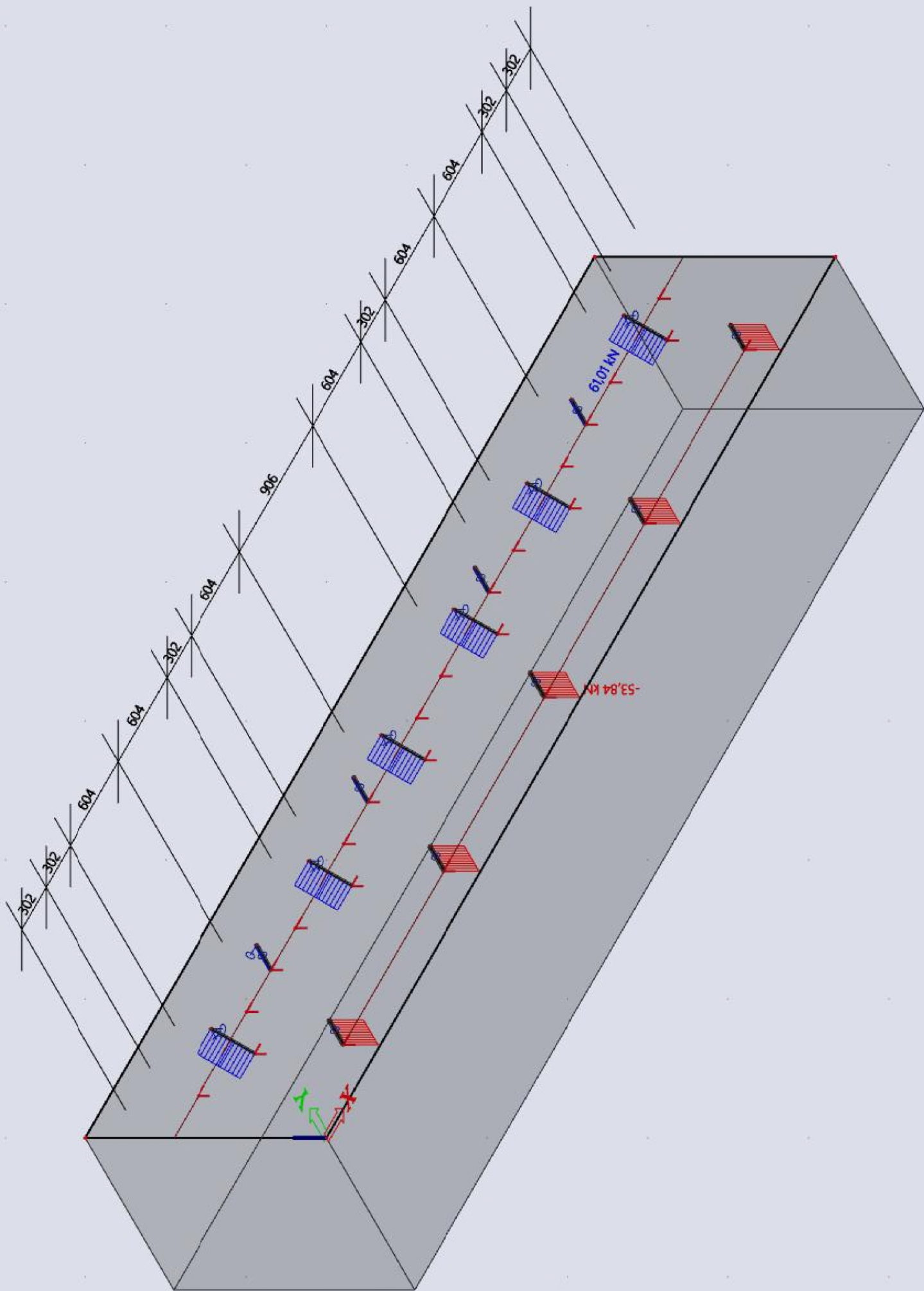


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8 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
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Joran van de Hoef

From: [REDACTED]
Sent: woensdag 3 september 2025 14:36
To: [REDACTED]
Cc: [REDACTED]
Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

Hallo [REDACTED]

De staalvoorziening van de onderste kolom is ontworpen om het hele gewicht te dragen in eindsituatie, de overige staalvoorzieningen zijn t.b.v. de 2^e draagweg en staan temperatuur zettingen toe d.m.v. slobgaten. Doordat er een overmaat gat wordt geboord in het binnenblad kan de balk zowel naar boven uitzetten als naar onder krimpen binnen dit overmaatse gat zonder krachten over te dragen aan het binnenblad.

Daarnaast kan de hoofd draagconstructie ook wel deze krachten op opvangen omdat de maatgevende krachten vanuit de andere as W01 zijn toegepast, dit is een significant zwaardere balk 36,5 ton t.o.v. 25,5 ton op de betreffende as 21.

Met vriendelijke groet / Kind regards,

[REDACTED]
Structural Engineer



From: [REDACTED]
Sent: woensdag 3 september 2025 09:49
To: [REDACTED]
Cc: [REDACTED]
Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

Beste [REDACTED]

Dit is voor mij wel te volgen, maar wat gebeurt er als de temperatuur daalt en de kolom wordt korter?

Krijg je dan geen schade aan bovenzijde, of is deze kracht op te nemen door de hoofddraagconstructie.

Met vriendelijke groeten,

ir. [REDACTED]
Senior Designer Engineer
Arcadis Nederland B.V.
Weena 505 | 3013 AL Rotterdam | The Netherlands
M [REDACTED]
www.arcadis.com

From: [REDACTED]
Sent: dinsdag 2 september 2025 16:10
To: [REDACTED]
Cc: [REDACTED]
Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

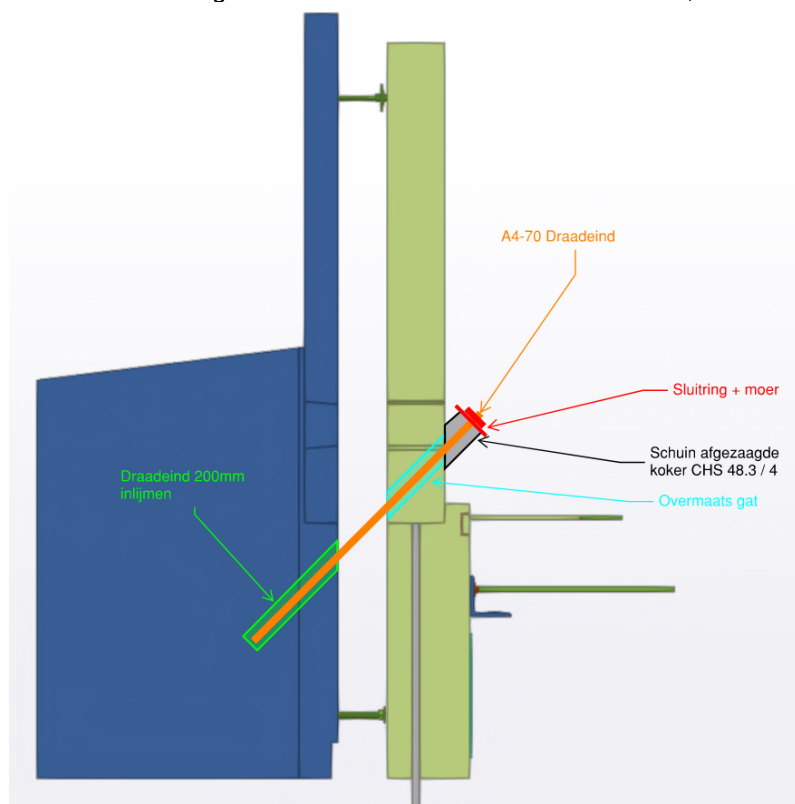
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Hallo [REDACTED]

Zoals besproken zullen wij ervoor zorgen dat er boorzones aangehouden worden waarbij geen wapening geraakt zal worden. Ook worden er RVS A4-70, minimaal M20, draadeinden toegepast.

Betreft de thermische werking van de verbinding stellen wij het volgende voor:

- Overmaats gat Ø32 in binnenblad, geeft verticaal en horizontaal vrijheid tot uitzetten
- Schuin afgezaagde koker + sluitring + moer om draadeind onder 45 graden te bevestigen op binnenblad, geeft ook diagonaal vrijheid tot uitzetten
- Verbinding werkt alleen in trek en niet meer in druk, zoals bedoeld



Graag ontvangen wij z.s.m. een reactie op het voorstel.

Met vriendelijke groet / Kind regards,

[REDACTED]
Structural Engineer



T
M
E

[REDACTED]
Locht 126
5504 RP Veldhoven
The Netherlands

www.byldis.com



From: [REDACTED]
Sent: dinsdag 2 september 2025 14:45

To: [REDACTED]

Cc: [REDACTED]
[REDACTED]

Subject: RE: The View II - 2e draagweg voorziening portaalbalk EE0101

Hoi [REDACTED]

Ik had hier nog een paar vragen over. Gisteren al telefonisch kort besproken.

*niet doorboren van essentiële wapening.

*RVS toepassen ivm oncontroleerbaarheid spouw.

Daarnaast is mijn vraag:

*Gaat de diagonale inlijming geen problemen opleveren ivm thermische werking van het buienblad tov het binnenblad

Met vriendelijke groeten,

ir. [REDACTED]
Senior Designer Engineer
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Weena 505 | 3013 AL Rotterdam | The Netherlands
M [REDACTED]
www.arcadis.com

From: [REDACTED]
Sent: donderdag 28 augustus 2025 15:27

To: [REDACTED]

Cc: [REDACTED]
[REDACTED]

Subject: The View II - 2e draagweg voorziening portaalbalk EE0101

Importance: High

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Hallo [REDACTED]

Als afsluiter van de montage lopen we tegen een nieuwe uitdaging aan wat betreft de stalen t-profielen welke de enkelschalige portaalbalk EE0101 verbindt met het dakrandelement SB0101 t.b.v. de 2^e draagweg.

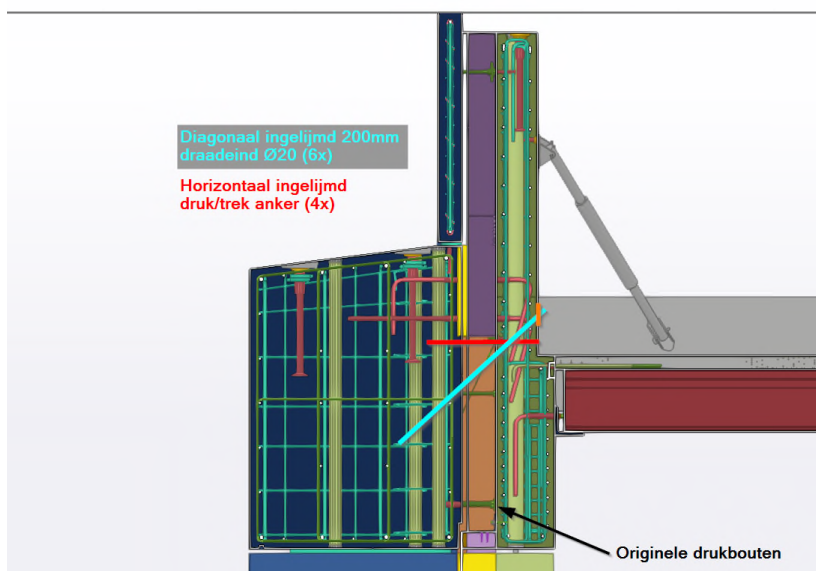
Na het monteren van de balk blijkt een tussenafstand tot 38mm te zijn ontstaan, dit is uiteraard niet te overbruggen of op te nemen door de bouten welke deze staalplaten moeten verbinden, zie afbeelding.

Er zal een alternatieve verbinding moeten worden toegepast t.b.v. 2^e draagweg omdat deze staalplaten nu onbruikbaar zijn.

Ons voorstel is om diagonaal draadeinden door dakrand SB0101 te boren en in EE0101 in te lijmen en met een sluitplaat aan SB0101 op te hangen. Gecombineerd met horizontale draadeinden t.b.v. de druk/trek welke ingelijmd worden in het dakrandelement SB0101. Bijgevoegd de inlijmcontrole van de Ø20 diagonale draadeind met Hilti HIT-HY 500 V4.

De krachten op deze verbinding zijn lager dan in de berekening van Dakranden – M01 omdat hier maatgevend as W01 is aangehouden waar de balk op as 21 significant lichter is. Hiermee zal het dakrand element nog steeds ruim voldoen.

Horizontaal 53,84 < 54kN en verticaal 61,01kN < 78,8kN



Graag horen wij snel of deze oplossingsrichting tot een akkoord kan leiden zodat we alvast de benodigdheden kunnen regelen.

Met vriendelijke groet / Kind regards,

[REDACTED]
Structural Engineer



T
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