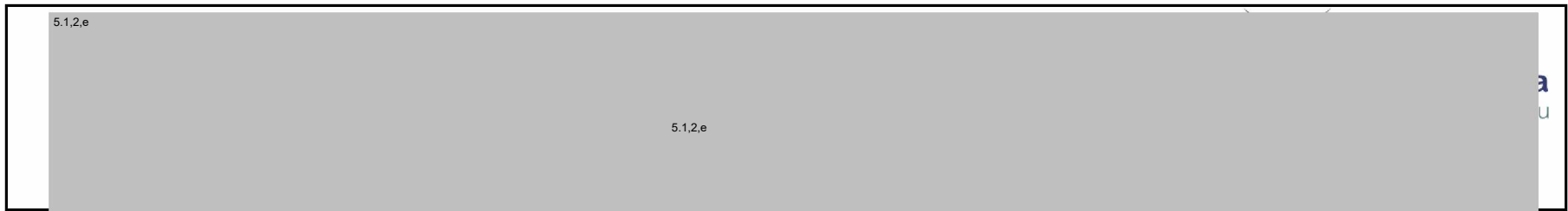


constructie berekening

E//Swap

		Provider		
5.1.2.e		5.1.2.e 5.1.2.e 5.1.2.e 5.1.2.e 5.1.2.e 5.1.2.e		
5.1.2.e		Structural Engineering		
		5.1.2.e 5.1.2.e 5.1.2.e 5.1.2.e 5.1.2.e		
StructuralEngineer	Modelleur	Rev.	Date	Reference
5.1.2.e	5.1.2.e	1.0	03/06/20	5G Ready
	5.1.2.e	1.1	11/06/20	5G Ready
	5.1.2.e	1.2	24/06/20	5G Ready
	5.1.2.e	1.3	16/07/20	5G Ready
	5.1.2.e	1.4	22/08/22	5G Ready
	5.1.2.e	1.5	14/11/23	5G Ready
		2.0	16/05/24	E//Swap

Date	Checked	Paraph
16/05/2024	5.1.2.e	5.1.2.e



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1.0 Conclusie

Verzwaringsen benodigd zie 2.2

berekening Antenne paal 1: 1x SI4518R57v06 + 3x RRU

UC antenne buis Ø60,3*4	0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4	0,08 < 1,0 (ok)
UC RRU paal Ø60,3*4	0,05 < 1,0 (ok)
UC kunststof ankers gemetselde muur	0,43 < 1,0 (ok)
rotatie antennes	0,03 < 1,0 graden (ok)
uitbuiging paal	0,04% < 8,0% (ok)

	U.C. strength		U.C. rotation		U.C. Deflection	
Paal 1	0,43	OK	0,03	Ok	0,00	Ok

berekening Antenne paal 2 : 1x ASI4518R57v06 + 3x RRU

UC antenne buis Ø60,3*4	0,07 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4	0,06 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4	0,06 < 1,0 (ok)
UC uithouder Ø48,3*3,2	0,04 < 1,0 (ok)
UC kunststof ankers gemetselde muur	0,18 < 1,0 (ok)
UC kunststof ankers beton	0,32 < 1,0 (ok)
rotatie antennes	0,02 degrees < 1,0 degrees (ok)
uitbuiging paal	0,32% < 8,0% (ok)

	U.C. strength		U.C. rotation		U.C. Deflection	
Paal 2	0,32	OK	0,02	Ok	0,04	Ok

berekening Antenne paal 3 :1x ASI4518R57v06 + 3x RRU

UC antenna Tube Ø60,3*4	0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4	0,10 < 1,0 (ok)
UC RRU paal Ø60,3*4	0,04 < 1,0 (ok)
UC kunststof ankers gemetselde muur	0,39 < 1,0 (ok)
rotatie antennes	0,03 degrees < 1,0 degrees (ok)
uitbuiging paal	0,04% < 8,0% (ok)

	U.C. strength		U.C. rotation		U.C. Deflection	
Paal 3	0,39	OK	0,03	Ok	0,00	Ok

berekening Antenne paal 4 : 1x ASI4518R57v06 + 3x RRU + 1x dish 300

UC antenne buis Ø60,3*4	0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4	0,10 < 1,0 (ok)
UC RRU paal Ø60,3*4	0,04 < 1,0 (ok)
UC kunststof ankers gemetselde muur	0,33 < 1,0 (ok)
rotatie antennes	0,03 degrees < 1,0 degrees (ok)
rotatie dish:	0,00 degrees < 0,7 degrees (ok)
uitbuiging paal	0,04% < 8,0% (ok)

	U.C. strength		U.C. rotation		U.C. Deflection	
Paal 4	0,33	OK	0,03	Ok	0,00	Ok

Dakconstructie zie hoofdstuk 7.0

5.1.2.e
5.1.2.e
constructie berekening
Date 16/05/2024

2.0 Project uitgangspunten

2.1 Projectbeschrijving

De antennes Sector 1;2;3;4 worden vervangen door Rank 7 ASI4518R57v06

Voor elke sector.

	7
Aplicable Antenna Rank	x
# RRU's	3
60MW dish option	x
DC cables	6
fibers	5
1/2" Jumper	12
# Diplexers	

2.2 Antenne Beslissingsboom & Versterking

Scope of Work 5.8 Antenna Upgrade Decision Tree

Nieuwe muur uithouder

2.3 Antenneconfiguratie

Calculated			
Type	number	ABH/DCH	Pole
ASI4518R57v06	4	26,50	P1;P2;P3;P4
Ø300	1	28,05	P4

2.4 Apparatuurconfiguratie

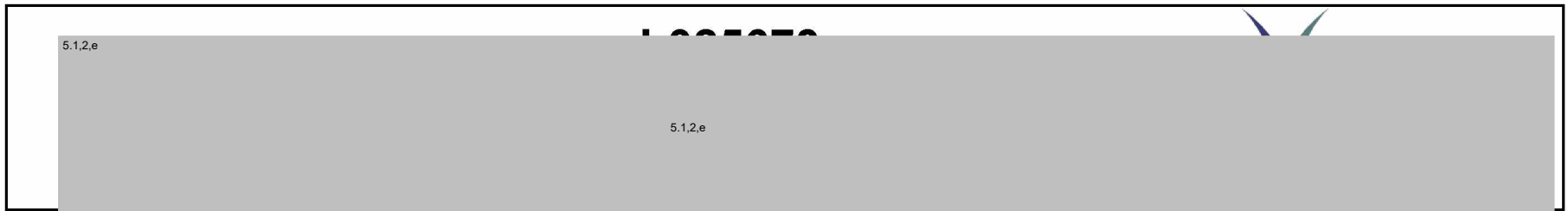
Type	number	Calculated	Position
RRU4486	3	3	op RRU paal
RRU4485	3	3	op RRU paal
RRU4020	3	3	op RRU paal
TP48200A	1	1	op vloer

2.5 Referenties

Beschrijving	company	revision
Drawings	Dynamica	2.0
zero measurements	Dynamica	

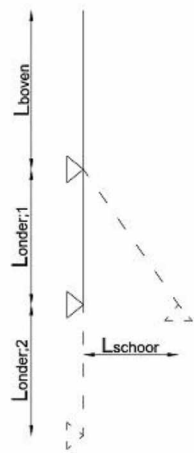
2.6 Aannamen

geen



3.0 Constructieschemas

	Diameter	Thickness	Verjonging	Thickness	L _{above} (m)	L _{below,1} (m)	L _{below,2} (m)	Bracing	L _{bracing} (m)
P1;2;3;4	Ø 60,3	4	-	-	0,81	1,50	0,49	-	-
P2	Ø 60,3	4	-	-	0,63	1,40	0,77	-	-



5.1,2,e
5.1,2,e

4.0 Constructieve uitgangspunten

4.1 Belastingcombinaties en factoren

Gevolgklasse CC2
Betrouwbaarheidsklasse RC2

Ultimate Limit State (ULS)

Blijvende en tijdelijke ontwerp-situaties	Blijvende belastingen		Voor-spanning	Dominante verander-lijke belasting (*)	Tegelijkertijd optredende Veranderlijk belastingen (*)	
	Ongunstig	Gunstig			Belang-rijkste (indien aanwezig)	Andere
(Vgl. 6.10a)	$\gamma_{G,sup} G_{k,j,sup}$	$\gamma_{G,j,inf} G_{k,j,inf}$	$\gamma_P P$		$\gamma_{Q,1} \psi_{0,1} Q_{k,1}$	$\gamma_{Q,j} \psi_{0,j} Q_{k,j}$
(Vgl. 6.10b)	$\xi \gamma_{G,j,sup} G_{k,j,su}$ p	$\gamma_{G,j,inf} G_{k,j,inf}$	$\gamma_P P$	$\gamma_{Q,1} Q_{k,1}$		$\gamma_{Q,j} \psi_{0,j} Q_{k,j}$

Table A1.2(B) NEN-EN 1990

	ξ	$\gamma_{G,sup}$	$\gamma_{G,inf}$	$\gamma_{Q,1}$
Vgl 6.10a	nvt	1,35	0,9	1,5
Vgl 6.10b	0,89	1,20	0,9	1,5

Serviceability Limit State (SLS)

Combinatie	Blijvende belastingen G_d		Veranderlijke belastingen Q_d	
	Ongunstig	Gunstig	Overheersende	Andere
Karakteristiek	$G_{k,j,sup}$	$G_{k,j,inf}$	$Q_{k,1}$	$\psi_{0,j} Q_{k,j}$
Frequent	$G_{k,j,sup}$	$G_{k,j,inf}$	$\psi_{1,1} Q_{k,1}$	$\psi_{2,j} Q_{k,j}$
Quasi-blijvend	$G_{k,j,sup}$	$G_{k,j,inf}$	$\psi_{2,1} Q_{k,1}$	$\psi_{2,j} Q_{k,j}$

Table A1.4 NEN-EN 1990

	wind	snow	building	office	roof
ψ_0	0	0	0,4	0,5	0
ψ_1	0,2	0,2	0,5	0,5	0
ψ_2	0	0	0,3	0,3	0

γ_g	γ_q
1,00	1,00

4.2 Referentieperiode

Uiterste grenstoestand Gebouwconstructies 50 jaar

veranderlijke vloerbelastingen gebouwen

$$\psi_t = \left\{ 1 + \frac{1 - \psi_0}{9} \ln \left(\frac{t}{t_0} \right) \right\} = \begin{matrix} \text{house} & \text{office} & \text{roof} & \text{wind} \\ 1,00 & 1,00 & 1,00 & 1,00 \end{matrix}$$

Uiterste grenstoestand Antenneconstructies 15 jaar

Windbelasting

t =	15	years	(referentieperiode)
p =	0,064	--	(art. 4.2 Basiswaarden NEN-EN 1991-1-4)
c _{prob} =	0,917	--	(art. 4.2 Basiswaarden NEN-EN 1991-1-4)
V _{b,t}	22,45	m/s	(afh. van referentieperiode van t jaren)
V _{m,t}	20,40	m/s	(afh. van referentieperiode van t jaren)
n =	0,500	--	(Tabel NB.2 NEN-EN 1991-1-4:20011/NB:2011)
K =	0,281	--	(Tabel NB.2 NEN-EN 1991-1-4:20011/NB:2011)
p _{w,t} =	0,71	kN/m ²	
ψ _t = p _{w,t} /p _w =	0,84	--	(A1.1 NEN-EN 1990)

Bruikbaarheidsgrenstoestand Hoekverdraaiing Antenneconstructi 1 jaar

$$\psi_t = \left\{ 1 + \frac{1 - \psi_0}{9} \ln \left(\frac{t}{t_0} \right) \right\} = \begin{matrix} \text{wind} \\ 0,57 \end{matrix}$$

4.3 Toelaatbare hoekverdraaiing en verplaatsing

Maximaal toelaatbare hoekverdraaiing antennes	1,00	graden
Maximaal toelaatbare hoekverdraaiing dishes	0,70	graden
Maximaal toelaatbare verplaatsing top paal	8%	

5.1.2.e	
	5.1.2.e

4.4 Normen en richtlijnen

Document	Titel
NEN-EN 1990+NB	Eurocode: Grondslagen van het constructief ontwerp
NEN-EN 1991-1-1+NB	Eurocode 1: Belastingen op constructies - Deel 1-1: Algemene belastingen -
NEN-EN 1991-1-3+NB	Eurocode 1: Belastingen op constructies - Deel 1-3: Sneeuwbelasting
NEN-EN 1991-1-4+NB	Eurocode 1: Belastingen op constructies - Deel 1-4: Windbelasting
NEN-EN 1992-1-1+NB	Eurocode 2: Ontwerp en berekening van betonconstructies - Deel 1-1: Algemene regeles en regels voor gebouwen
NEN-EN 1993-1-1+NB	Eurocode 3: Ontwerp en berekening van staalconstructies - Deel 1-1: Algemene regeles en regels voor gebouwen
NEN-EN 1993-1-8+NB	Eurocode 3: Ontwerp en berekening van staalconstructies - Deel 1-8: Ontwerp en berekening van verbindingen
NEN-EN 1993-3-1+NB	Eurocode 3: Ontwerp en berekening van staalconstructies - Deel 3-1: Torens, masten en schoorstenen-Torens en masten
NEN-EN 1993-3-2+NB	Eurocode 3: Ontwerp en berekening van staalconstructies - Deel 3-2: Torens, masten en schoorstenen-Schoorstenen
NEN-EN 1995-1-1+NB	Eurocode 5: Ontwerp en berekening van houtconstructies
NEN-EN 1996-1-1+NB	Eurocode 6: Ontwerp en berekening van constructies van metselwerk
NEN-EN 1997-1-1+NB	Eurocode 7 Geo technische ontwerp
NEN 8700	Beoordeling constructieve veiligheid van een bestaand bouwwerk bij verbouw en afkeuren – Grondslagen'
NEN 8701	Beoordeling van de constructieve veiligheid een bestaand bouwwerk bij verbouwen en afkeuren - Belastingen
NEN 2767	Conditiemeting van gebouwen, de methodiek

4.5 Materialen

	$f_{y,rep}$ N/mm2	$f_{u,rep}$ N/mm2	E_{rep} N/mm2	ρ_{staal} kN/m3	
Staal	S235JRG2	235	360	210000	78,5

	$f_{y,b}$ N/mm2	$f_{u,b}$ N/mm2		
Bouten	8.8	640	800	Nordlock
Ankers	4.6	240	400	Nordlock

	f_{ck}	f_{cd}	f_{ctd}	f_{ctm}	E_{cm}	ρ kN/m3	
Beton	C20/25	20	13,3	1,03	2,21	30000	24

	$f_{m,0,rep}$	$f_{t,0,rep}$	$f_{t,90,rep}$	$f_{c,0,rep}$	$f_{c,90,rep}$	$f_{v,0,rep}$	$E_{0,ser,rep}$	$E_{0,u,rep}$	
Hout	C18	18	11	0,5	18	2,2	2	9000	6000

Metselwerk	Brick	15	N/mm2
	Sandstone	CS16	N/mm2
	Mortar	M5-10	N/mm2

5.1,2,e	
	5.1,2,e

5.0 Belastingen

5.1 windbelasting

Omgeving : bebouwd
Windgebied : 3

h_{max} = 29,3 m
 h_{min} = 7,0 m
 z_0 = 0,500 m
 z_{min} = 7,0 m
 k_r = 0,22 --
 c_r = 0,91 --
 c_0 = 1 --
 V_b = 24,5 m/s
 V_m = 22,3 m/s
 I_v = 0,25 --
 p_w = 0,84 kN/m²

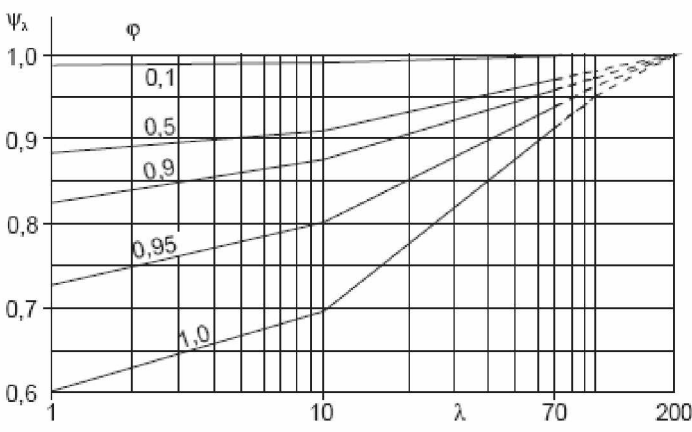
5.2 Krachtcoëfficiënt C_f & $C_s C_d$ factor

article 7.9.2 NEN-EN 1991-1-4

$C_f = C_{f,0} \cdot \psi_\lambda$
 $b_{pole} = 60,3 \text{ mm}$
 $L = 0,81 \text{ m}$
(≤ 15 , for $L \geq 15 \text{ m}$ zie table 7.16)

ψ_λ art. 7.13

table 7.16: $\lambda = 13 \rightarrow \text{fig. 7.3 } \psi_\lambda = 0,72$
 $\phi = 1$



$C_{f,0}$ figure 7.28

$k = 0,05$ (ruwheidshoogte smooth steel table 7.13)
 $k/b = 0,83$

$Re = b \cdot V(ze) / \nu$ (Reynolds number)

$V(ze) = \sqrt{\frac{2 \cdot q_p}{\rho}} = 11,61 \text{ m/s}$ $q_p = 0,84 \text{ kN/m}^2$
 $\rho = 1,25 \text{ kG/m}^3$

$\nu = 0,000015 \text{ m}^2/\text{s}$
 $b = 0,0603 \text{ m}$ (Diameter off the pole)

$Re = 46673$ (Reynolds number)

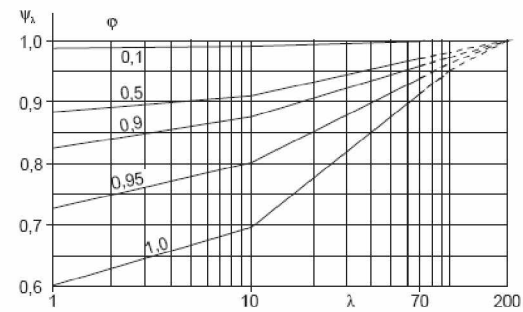
$Re < 1,7 \cdot 10^5$ $C_{f,0} = 1,2$
 $1,7 \cdot 10^5 < Re < 4,0 \cdot 10^5$ $C_{f,0} = \frac{0,11}{(Re/10^6)^{1,4}} = -$
 $Re > 4,0 \cdot 10^5$ $C_{f,0} = \frac{1,2 + \frac{0,18 \cdot \log(10 \cdot k/b)}{1 + 0,4 \cdot \log(Re/10^6)}}{1} = -$

$C_{f,0} = 1,20$ ($= > 0,4$)

$C_f = C_{f,0} \cdot \psi_\lambda = 0,86$

$C_s C_d = 1,00$ art. 6.2 NEN-EN 1991-1-4

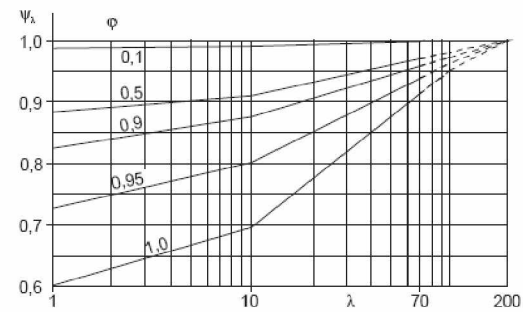
	h (m)	b (m)	d (m)
Afmetingen kast:	1,60	0,70	0,65

$$C_f = C_{f,0} * \Psi_\lambda * \Psi_r$$

$$\psi_r = 1,00$$
$$\psi_{\lambda} = 0,67$$
 $\lambda = 4,57$

d/b =	0,93	$0,1 < d/b \leq 0,1$	$C_{f,0} = 2,0$	-
		$0,2 < d/b \leq 0,1$	$C_{f,0} = 0,875(d/b - 0,2) + 2,00 =$	-
C _{f,0} =	2,17	$0,6 < d/b \leq 0,1$	$C_{f,0} = 0,50(d/b - 0,6) + 2,35 =$	-
		$0,7 < d/b \leq 1$	$C_{f,0} = -1,00(d/b - 0,7) + 2,40 =$	2,17
		$1,0 < d/b \leq 2$	$C_{f,0} = -0,45(d/b - 1,0) + 2,10 =$	-
		$2,0 < d/b \leq 5$	$C_{f,0} = -0,217(d/b - 2,0) + 1,65 =$	-
		$5 < d/b \leq 10$	$C_{f,0} = -0,02d/b + 1,0 =$	-
		$d/b > 10$	$C_{f,0} =$	-

1,45

	h (m)	b (m)	d (m)
Afmetingen	5,00	0,10	0,10

$$C_f = C_{f,0} * \Psi_\lambda * \Psi_r$$

$$\psi_r = 1,00$$
$$\psi_A = 0,92$$
 $\lambda = 70,00$
$$C_{f,o} = 1,20$$

1,10

5.1,2,e	
	5.1,2,e

5.5 Antennebelasting

De windbelasting van de antennes wordt bepaald volgens de documentatie van de antenneleverancier (windtunnelonderzoek)

Uit de gegeven windlast en de windsnelheid kan de effectieve antenne-oppervlakte bepaald worden.

Stuwdruk (p_w) = $1/2 * 1,25 * (v_w/3,6)^2$

effectieve oppervlakte A_{eff} = F_(N) / P

F_{antenna;rep} : 1 antenna $\gamma_{SLS,q} \times p_w \times A_{eff, rear} =$
2 antennas $\gamma_{SLS,q} \times p_w \times A_{eff, rear} + (A_{eff, frontal} + A_{eff, lateral}) / 2 =$
3 antennas $\gamma_{SLS,q} \times p_w \times (A_{eff, rear} + A_{eff, frontal} + A_{eff, lateral}) =$

antenna type	number	Pole	H (mm)	B (mm)	D (mm)	G (kg)	G+5kg	F _{Frontal} (N)	F _{Lateral} (N)	F _{Rear} (N)
ASI4518R57v06	1	1;2;3;4	2769	369	226	41,7	46,7	585	695,00	1060
A _{eff,frontal} (m2)	A _{eff,lateral} (m2)	A _{eff,rear} (m2)	v _{wind} (km/h)	P _{N/m2}	F _{rear,rep} kN	bracket	F _{lateral,rep} kN	bracket	F _{frontal,rep} kN	bracket
0,54	0,64	0,98	150	1085	0,82	0,41	0,54	0,27	0,45	0,23

5.6 Dishbelasting

Dishtype	number	Pole	C _f	A _{eff} (m2)	F _{dish;rep} kN
Ø 300	1	4	1,50	0,11	0,12

5.7 Windbelasting antennepaal/uthouder

Pole	D (mm)	C _{f,paal}	B _{ladder}	B _{feeders}	C _f	q _{wind} kN/m'
nvt						

5.8 Belasting RRUS, TMA's e.d.

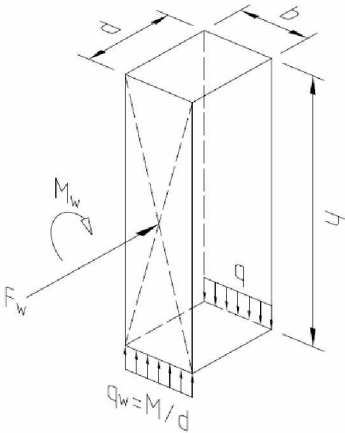
Type	aantal	Paal	H (mm)	B (mm)	D (mm)	G (kg)	Cf	F _{frontal,rep} kN	F _{lateral,rep} kN
RRU4486	3		522	397	149	27,1	1,45	0,25	0,10

Type	aantal	Paal	H (mm)	B (mm)	D (mm)	G (kg)	Cf	F _{frontal,rep} kN	F _{lateral,rep} kN
RRU4485	3		474,5	397	144,5	25	1,45	0,23	0,08

Type	aantal	Paal	H (mm)	B (mm)	D (mm)	G (kg)	Cf	F _{frontal,rep} kN	F _{lateral,rep} kN
RRU4020	3		495	404	163	23,5	1,45	0,24	0,10

5.9 Belasting Apparatuurkasten

Type	number	H (mm)	B (mm)	D (mm)	G (kN)	q _{eg;rep} kN/m'
TP48200A	1	1600	700	650	296	2,11
		Cf	F _{frontal,rep} kN	M _{frontal,rep} kNm	q _{wind,rep} kN/m'	
		1,45	1,37	1,10	+/-1,69	



5.10 Valbelasting

nvt



6.0 Berekeningen

Voor de computerberekening wordt verwezen naar bijlage 1

6.1 berekening Antenne paal 1: 1x SI4518R57v06 + 3x RRU

UC antenne buis Ø60,3*4 0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4 0,08 < 1,0 (ok)
UC RRU paal Ø60,3*4 0,05 < 1,0 (ok)

rotatie antennes 0,000 radial = 0,03 graden < 1,0 graden (ok)
uitbuiging paal 0 mm 0,04% < 8,0% (ok)
H_{pole}= 810 mm

Berekening kunststof ankers gemetselde muur

M12 8.8 70mm limed

N_{Ed,1}= 0,24 kN n= 4
V_{Ed,y}= 0,00 kN N_{tEd}=N_{Ed,1}/n+N_{Ed,2} = 0,60 kN
V_{Ed,z}= 0,62 kN V_{Ed}=V_{Ed}/n = 0,16 kN
V_{Ed}= 0,62 kN V/N_{Rd}= 1,40 kN
M_{Ed}= 0,13 kNm
e= 0,13 m UC kunststof ankers gemetselde muur 0,43 < 1,0 (ok)
n_t= 2 tension anchors
N_{Ed,2}= 0,54 kN

HUD-1 Plastic anchor											
Design resistance											
Anchor size		5x25		6x30		8x40		10x50		12x60	14x70
Screw type ^{d)}		W	C	W	C	W	C	W	C	W	W
Size		4	4	5	5	6	6	8	8	10	12
DIN		96		96		96		96		571	571
Solid clay brick	N _{RU} [kN]	0,24	0,08	0,49	0,21	0,84	0,49	1,12	-	1,40	1,40 ^{e)}
	Mz 20	V _{RU} [kN]	0,34	-	0,42	-	0,62	-	-	-	-

6.2 berekening Antenne paal 2 : 1x ASI4518R57v06 + 3x RRU

UC antenne buis Ø60,3*4 0,07 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4 0,06 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4 0,06 < 1,0 (ok)
UC uithouder Ø48,3*3,2 0,04 < 1,0 (ok)

rotatie antennes 0,000 radial = 0,02 degrees < 1,0 degrees (ok)
uitbuiging paal 2 mm 0,32% < 8,0% (ok)
H_{pole}= 630 mm

Berekening kunststof ankers gemetselde muur

M12 8.8 70mm limed

N_{Ed,1}= 0,35 kN n= 4
V_{Ed,y}= 0,02 kN N_{tEd}=N_{Ed,1}/n+N_{Ed,2} = 0,25 kN
V_{Ed,z}= 0,74 kN V_{Ed}=V_{Ed}/n = 0,19 kN
V_{Ed}= 0,74 kN V/N_{Rd}= 1,40 kN
M_{Ed}= 0,07 kNm
e= 0,20 m UC kunststof ankers gemetselde muur 0,18 < 1,0 (ok)
n_t= 2 tension anchors
N_{Ed,2}= 0,17 kN

HUD-1 Plastic anchor											
Design resistance											
Anchor size		5x25		6x30		8x40		10x50		12x60	14x70
Screw type ^{a)}		W	C	W	C	W	C	W	C	W	W
Size		4	4	5	5	6	6	8	8	10	12
DIN		96		96		96		96		571	571
Solid clay brick	N _{Re}	0,24	0,08	0,49	0,21	0,84	0,49	1,12	-	1,40	1,40 ^{c)}
	Mz 20	V _{Re} [kN]	0,34	-	0,42	-	0,62	-	-	-	-

Berekening kunststof ankers beton

M12 8.8 70mm limed

N_{Ed,1}= 0,00 kN n= 4
V_{Ed,y}= 0,31 kN N_{tEd}=N_{Ed,1}/n+N_{Ed,2} = 0,42 kN
V_{Ed,z}= 0,46 kN V_{Ed}=V_{Ed}/n = 0,14 kN
V_{Ed}= 0,55 kN V/N_{Rd}= 1,33 kN
M_{Ed}= 0,13 kNm
e= 0,15 m UC kunststof ankers beton 0,32 < 1,0 (ok)
n_t= 2 tension anchors
N_{Ed,2}= 0,42 kN

Anchor size		5x25		6x30		8x40		10x50		12x60	14x70
Screw type ^{d)}		W	C	W	C	W	C	W	C	W	W
Size		4	4	5	5	6	6	8	8	10	12
DIN		96		96		96		96		571	571
Concrete ≥ C16/20	N _{Rd} [kN]	0,42	0,14	0,77	0,49	1,19	0,70	1,96	-	2,80	4,20
	V _{Rd} [kN]	0,56	-	1,26	-	1,75	-	3,08	-	4,20	7,84



6.3 berekening Antenne paal 3 :1x ASI4518R57v06 + 3x RRU

UC antenna Tube Ø60,3*4 0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4 0,10 < 1,0 (ok)
UC RRU paal Ø60,3*4 0,04 < 1,0 (ok)

rotatie antennes 0,0005 radial = 0,03 degrees < 1,0 degrees (ok)
uitbuiging paal 0,3 mm 0,04% < 8,0% (ok)
H_{pole}= 810 mm

Berekening kunststof ankers gemetselde muur

M12 8.8 70mm limed

N_{Ed,1}= 0,43 kN
V_{Ed,y}= 0,73 kN
V_{Ed,z}= 0,43 kN
V_{Ed}= 0,85 kN
M_{Ed}= 0,17 kNm
e= 0,20 m
n_t= 2 tension anchors
N_{Ed,2}= 0,44 kN

n= 4
N_{tEd}=N_{Ed,1}/n+N_{Ed,2} = 0,54 kN
V_{Ed}=V_{Ed}/n = 0,21 kN
V/N_{Rd}= 1,40 kN

HUD-1 Plastic anchor											
Design resistance											
Anchor size	5x25		6x30		8x40		10x50		12x60	14x70	
Screw type ^{a)}	W	C	W	C	W	C	W	C	W	W	
Size	4	4	5	5	6	6	8	8	10	12	
DIN	96		96		96		96		571	571	
Solid clay brick	N _{Rd} [kN]	0,24	0,08	0,49	0,21	0,84	0,49	1,12	-	1,40	1,40 ^{c)}
Mz 20	V _{Rd} [kN]	0,34	-	0,42	-	0,62	-	-	-	-	-

UC kunststof ankers gemetselde muur 0,39 < 1,0 (ok)

6.4 berekening Antenne paal 4 : 1x ASI4518R57v06 + 3x RRU + 1x dish 300

UC antenne buis Ø60,3*4 0,10 < 1,0 (ok)
UC muur uithouder Ø60,3*4 / K60*4 0,10 < 1,0 (ok)
UC RRU paal Ø60,3*4 0,04 < 1,0 (ok)

rotatie antennes 0,000 radial = 0,03 degrees < 1,0 degrees (ok)
rotatie dish: 0,000 radial = 0,00 degrees < 0,7 degrees (ok)
uitbuiging paal 0 mm 0,04% < 8,0% (ok)
H_{pole}= 810 mm

Berekening kunststof ankers gemetselde muur

M12 8.8 70mm limed

N_{Ed,1}= 0,31 kN
V_{Ed,y}= 0,02 kN
V_{Ed,z}= 0,66 kN
V_{Ed}= 0,66 kN
M_{Ed}= 0,15 kNm
e= 0,20 m
n_t= 2 tension anchors
N_{Ed,2}= 0,38 kN

n= 4
N_{tEd}=N_{Ed,1}/n+N_{Ed,2} = 0,46 kN
V_{Ed}=V_{Ed}/n = 0,17 kN
V/N_{Rd}= 1,40 kN

HUD-1 Plastic anchor											
Design resistance											
Anchor size	5x25		6x30		8x40		10x50		12x60	14x70	
Screw type ^{a)}	W	C	W	C	W	C	W	C	W	W	
Size	4	4	5	5	6	6	8	8	10	12	
DIN	96		96		96		96		571	571	
Solid clay brick	N _{Rd} [kN]	0,24	0,08	0,49	0,21	0,84	0,49	1,12	-	1,40	1,40 ^{c)}
Mz 20	V _{Rd} [kN]	0,34	-	0,42	-	0,62	-	-	-	-	-

UC kunststof ankers gemetselde muur 0,33 < 1,0 (ok)



7.0 Berekening Bouwconstructie (aansluitingen, wanden, dak ..)

RRU's op muur:

De metselwerk muur is voldoende om RRU's rechtop te zetten.

Vloer constructie bij apparatuur

appartuur ligt binnen in het gebouw naast de wand
de vloer is ontworpen voor 2kN/m2 dit is voldoende voor het dragen van de apparatuur

5.1.2.e	5.1.2.e	
		5.1.2.e

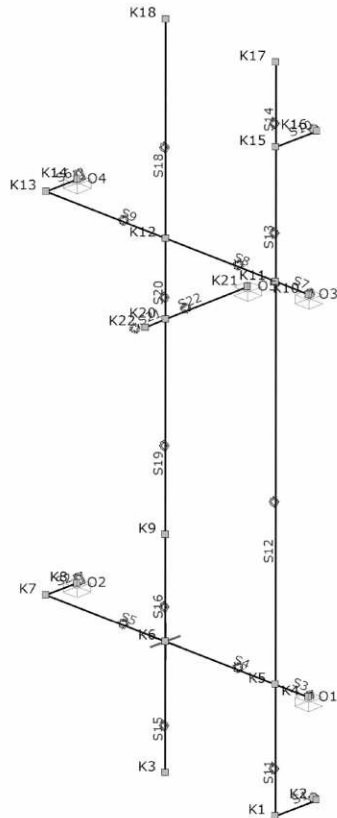
8.0 Bijlage

Bijlage 1

Computerberekeningen

Pole 1			
Project name	L025673	Project number	
Part description	Pole 1	Structural engineer	
Client		Units	m, kN, kNm

PIC. GEOMETRIE RAAMWERK



MEMBERS

Member	Node B	Node E	X-B	Y-B	Z-B	X-E	Y-E	Z-E	Length	Section	Pos
S1	K2	K1	0,200	0,535	0,490	0,000	0,535	0,490	0,200	P3	0,000 - L(0,
S2	K8	K7	0,150	-0,580	0,000	0,000	-0,580	0,000	0,150	P2	0,000 - L(0,
S3	K4	K5	0,000	0,695	0,000	0,000	0,535	0,000	0,160	P1	0,000 - L(0,
S4	K5	K6	0,000	0,535	0,000	0,000	0,000	0,000	0,535	P1	0,000 - L(0,
S5	K6	K7	0,000	0,000	0,000	0,000	-0,580	0,000	0,580	P1	0,000 - L(0,
S6	K14	K13	0,150	-0,580	-1,500	0,000	-0,580	-1,500	0,150	P2	0,000 - L(0,
S7	K10	K11	0,000	0,695	-1,500	0,000	0,535	-1,500	0,160	P1	0,000 - L(0,
S8	K11	K12	0,000	0,535	-1,500	0,000	0,000	-1,500	0,535	P1	0,000 - L(0,
S9	K12	K13	0,000	0,000	-1,500	0,000	-0,580	-1,500	0,580	P1	0,000 - L(0,
S10	K16	K15	0,200	0,535	-2,000	0,000	0,535	-2,000	0,200	P3	0,000 - L(0,
S11	K1	K5	0,000	0,535	0,490	0,000	0,535	0,000	0,490	P1	0,000 - L(0,
S12	K5	K11	0,000	0,535	0,000	0,000	0,535	-1,500	1,500	P1	0,000 - L(1,
S13	K11	K15	0,000	0,535	-1,500	0,000	0,535	-2,000	0,500	P1	0,000 - L(0,
S14	K15	K17	0,000	0,535	-2,000	0,000	0,535	-2,315	0,315	P1	0,000 - L(0,
S15	K3	K6	0,000	0,000	0,490	0,000	0,000	0,000	0,490	P1	0,000 - L(0,
S16	K6	K9	0,000	0,000	0,000	0,000	0,000	-0,400	0,400	P1	0,000 - L(0,
S18	K12	K18	0,000	0,000	-1,500	0,000	0,000	-2,315	0,535	P1	0,000 - L(0,
S19	K9	K20	0,000	0,000	-0,400	0,000	0,000	-1,200	0,800	P1	0,000 - L(0,
S20	K20	K12	0,000	0,000	-1,200	0,000	0,000	-1,500	0,300	P1	0,000 - L(0,
S21	K22	K20	-0,100	0,000	-1,200	0,000	0,000	-1,200	0,100	P1	0,000 - L(0,
S22	K20	K21	0,000	0,000	-1,200	0,400	0,000	-1,200	0,400	P1	0,000 - L(0,
-	-	-	m	m	m	m	m	m	m	-	

SECTIONS

Section	Section Name	Area	It	Iy	Iz	Material	Angle
P1	N60.3/4	7.0749e-04	5.6346e-07	2.8173e-07	2.8173e-07	S235H(EN10210-1)	0,0

	Pole 1	
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Section	Section Name	Area	It	Iy	Iz Material	Angle
P2	K60x60x4x4	8.9600e-04	7.0246e-07	4.7070e-07	4.7070e-07 S235	0,0
P3	C100	7.8540e-03	9.8175e-06	4.9087e-06	4.9087e-06 S235	0,0
-	-	m2	m4	m4	m4 -	°

SECTION SHAPES

Section	Tapered	hB	hE	tf	tw	tf2	B	b1	b2 Castellate	Height
P2	No	0,060	0,060	0,0040	0,0040	0,0000	0,060	0,000	0,000 No	0,000
P3	No	0,100	0,100	0,0000	0,0000	0,0000	0,100	0,000	0,000 No	0,000
-	-	m	m	m	m	m	m	m	m -	m

MATERIALS

Material Name	Poison	Density	Youngs mod.	Exp.
S235H(EN10210-1)	0.30	78.50	2.1000e+08	12.0000e-06
S235	0.30	78.50	2.1000e+08	12.0000e-06
-	-	kN/m3	kN/m2	C°m

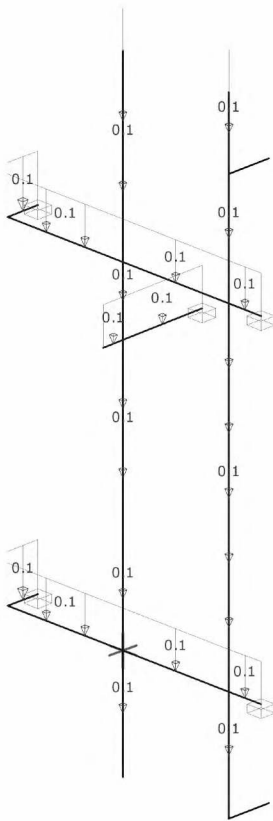
SUPPORTS

Support	Object	Position	X	Y	Z	Xr	Yr	Zr	AngleXr	AngleYr	AngleZr
O1	K4	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O2	K8	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O3	K10	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O4	K14	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O5	K21	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
-	-	m	kN/m	kN/m	kN/m	kNm/rad	kNm/rad	kNm/rad	°	°	°

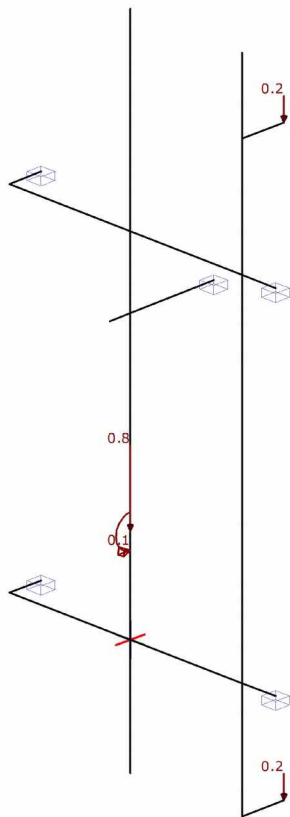
LOADS CASES TYPES

Label	Description	L.C. Type	Fav./Unfav.	Element	Level	Field	Psi0	Psi1	Psi2	Cprob ULS/SLS
B.G.1	Permanent actions	Permanent actions	-		None	None				
B.G.2	Permanent actions	Permanent actions	-		None	None				
B.G.3	wind x	Wind load	-		None	None	0.20			1,00/1,00

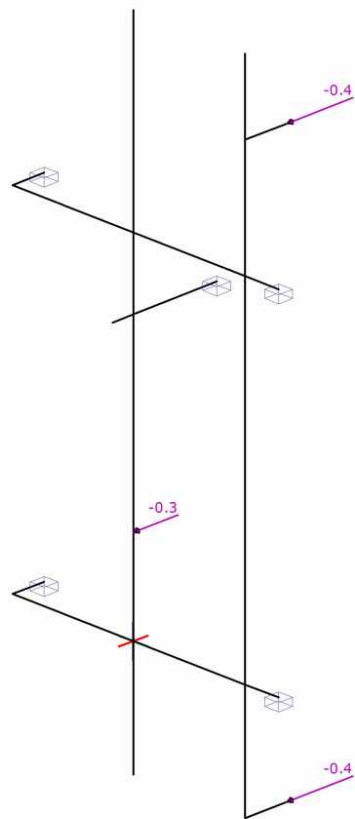
PIC. LASTEN B.G.1 PERMANENT ACTIONS



PIC. LASTEN B.G.2 PERMANENT ACTIONS



PIC. LASTEN B.G.3 WIND X



PERSISTENT LOADS COMBINATIONS (TABLE)

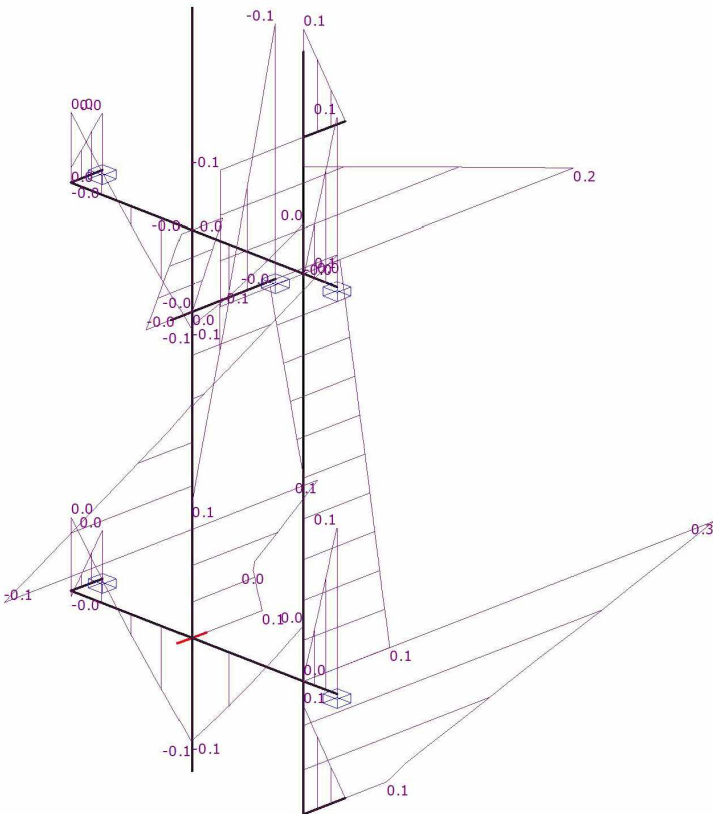
L.C.	Description	Pe.C.1	Pe.C.2
B.G.1	Permanent actions	1.35	1.20
B.G.2	Permanent actions	1.35	1.20
B.G.3	wind x	0.00	1.27

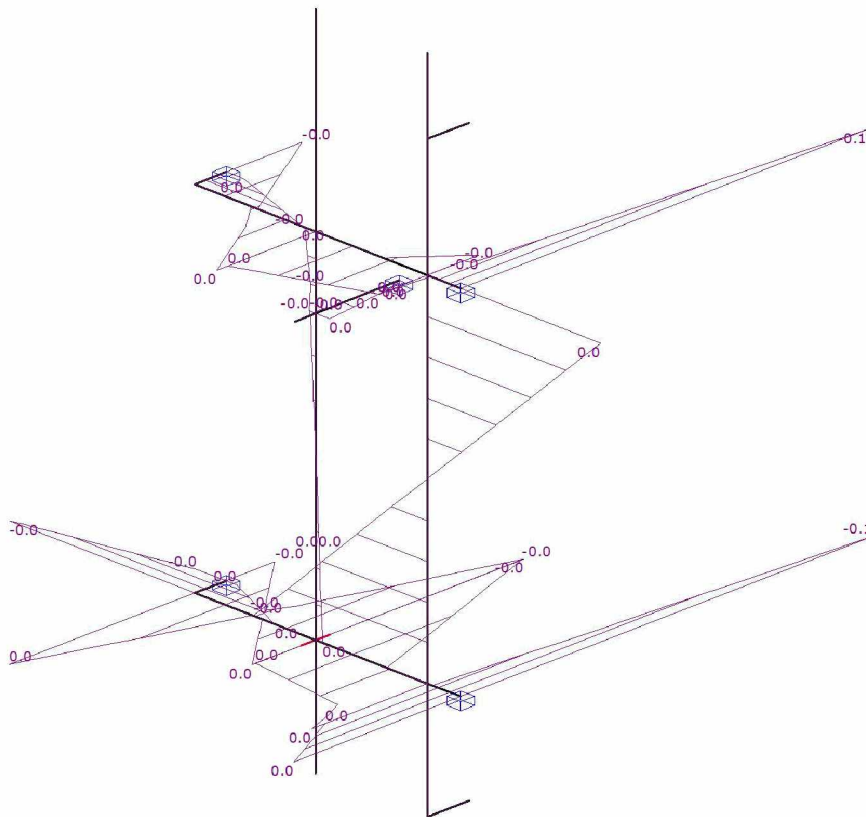
CHARACTERISTIC LOADS COMBINATIONS (TABLE)

L.C.	Description	Ch.C.(w1)	Ch.C.1
B.G.1	Permanent actions	1.00	1.00
B.G.2	Permanent actions	1.00	1.00
B.G.3	wind x	0.00	0.57

ANALYSIS ASSUMPTIONS

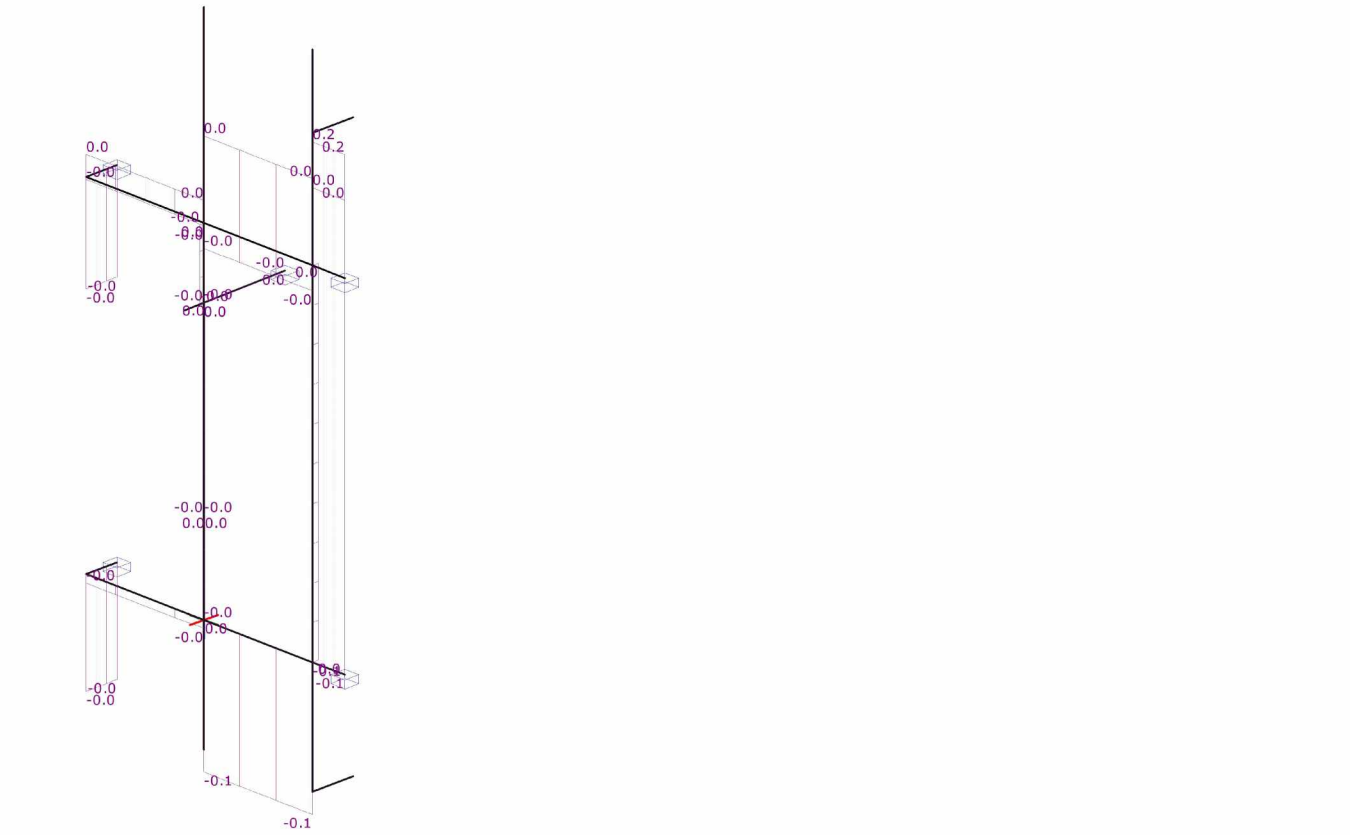
Linear Elastic Analysis performed

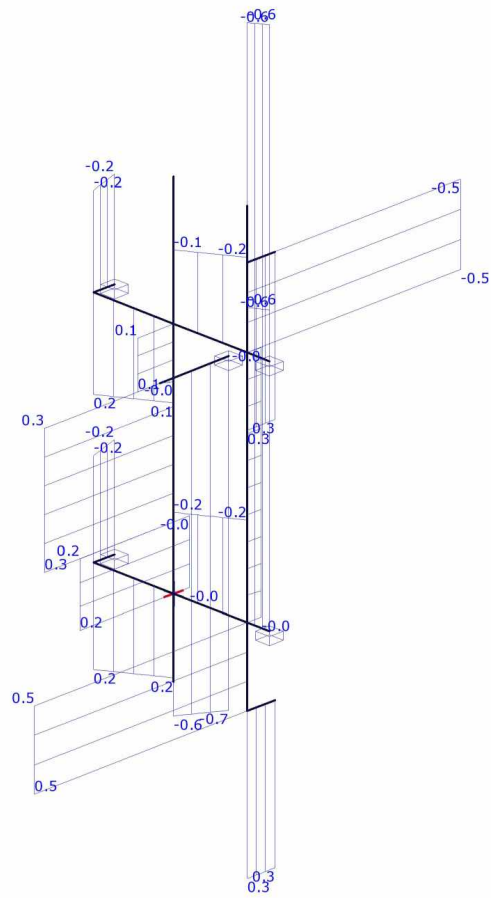


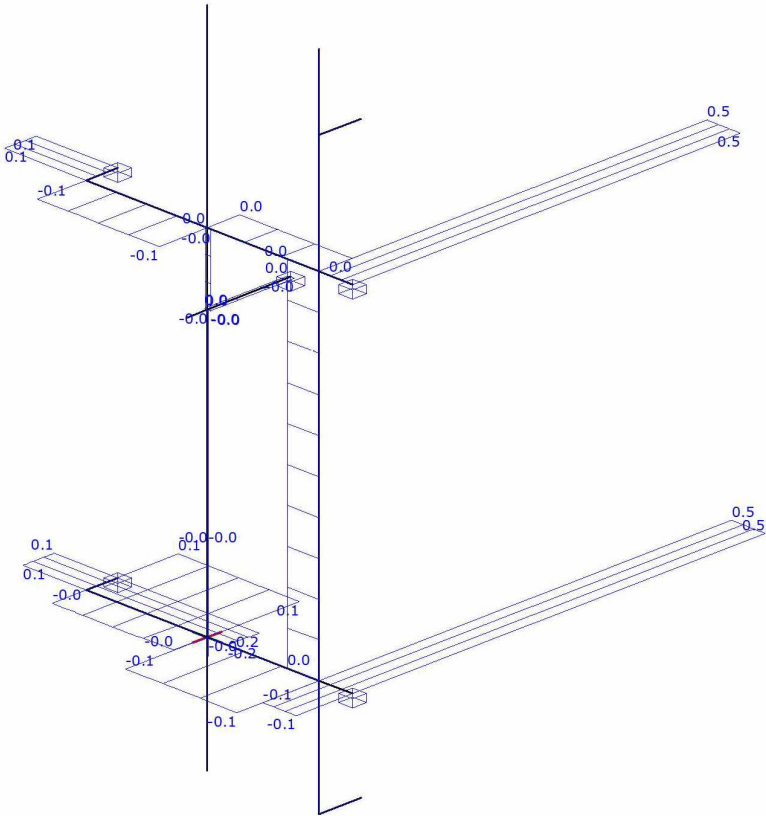


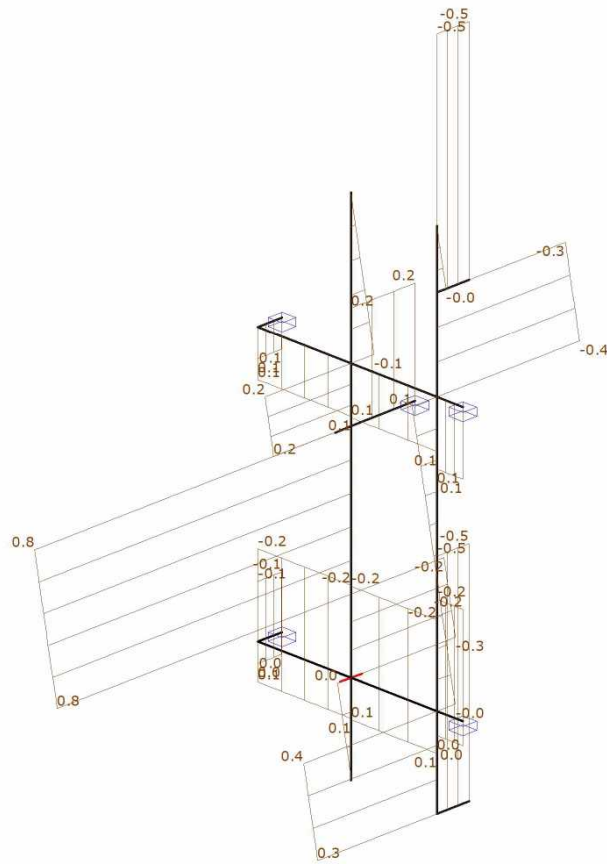
PIC. PE.C. MOMENT (MX) ENVELOPE Persistent Loads Combinations

PIC. PE.C. MOMENT (MX) ENVELOPE Persistent Loads Combinations









PE.C. MEMBER FORCES (MY, MZ)

Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S1	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
S2	Pe.C.1	My	0.03			0.00	0.134	0.000
		Mz	-0.01			-0.03	0.000	0.000
	Pe.C.2	My	0.02			0.00	0.136	0.000
		Mz	0.00			0.01	0.000	0.000
S3	Pe.C.1	My	0.10			0.00	0.000	0.000
		Mz	0.03			0.02	0.000	0.000
	Pe.C.2	My	0.08			0.00	0.000	0.000
		Mz	-0.07			0.01	0.136	0.000
S4	Pe.C.1	My	0.03			-0.06	0.176	0.000
		Mz	0.02			-0.04	0.192	0.000
	Pe.C.2	My	0.03			-0.05	0.174	0.000
		Mz	0.01			0.01	0.000	0.000
S5	Pe.C.1	My	-0.06			0.04	0.357	0.000
		Mz	-0.04			0.03	0.308	0.000
	Pe.C.2	My	-0.05			0.04	0.357	0.000
		Mz	0.01			-0.01	0.257	0.000
S6	Pe.C.1	My	0.03			0.00	0.145	0.000
		Mz	0.00			0.01	0.000	0.000
	Pe.C.2	My	0.03			0.01	0.000	0.000
		Mz	0.00			0.02	0.000	0.000
S7	Pe.C.1	My	0.10			0.00	0.157	0.000
	Mz	-0.01				-0.01	0.000	0.000
	Pe.C.2	My	0.09			0.00	0.156	0.000

	Pole 1					
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Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S8	Pe.C.1	Mz	-0.07			0.01	0.143	0.000
		My	0.03			-0.06	0.170	0.000
	Pe.C.2	Mz	-0.01			0.01	0.212	0.000
		My	0.02			-0.05	0.167	0.000
S9	Pe.C.1	Mz	0.01			0.02	0.000	0.000
		My	-0.06			0.04	0.357	0.000
	Pe.C.2	Mz	0.01			-0.01	0.310	0.000
		My	-0.05			0.03	0.357	0.000
S10	Pe.C.1	Mz	0.02			-0.02	0.279	0.000
		My	0.00			0.06	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.06	0.000	0.000
S11	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.06			0.06	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.06			0.31	0.000	0.000
S12	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.03			-0.03	0.757	0.000
	Pe.C.2	Mz	-0.03			0.03	0.753	0.000
		My	0.07			0.03	0.000	0.000
S13	Pe.C.1	Mz	-0.03			0.03	0.753	0.000
		My	-0.06			-0.06	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.21			-0.06	0.393	0.000
S14	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S15	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S16	Pe.C.1	Mz	0.00			0.10	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.05			0.04	0.000	0.000
		My	0.00			0.00	0.000	0.000
S18	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S19	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	-0.11			0.08	0.463	0.000
	Pe.C.2	Mz	0.00			0.00	0.395	0.000
		My	-0.14			0.11	0.450	0.000
S20	Pe.C.1	Mz	0.00			0.00	0.268	0.000
		My	-0.03			-0.01	0.000	0.000
	Pe.C.2	Mz	0.00			0.02	0.000	0.000
		My	0.00			0.00	0.000	0.000
S21	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S22	Pe.C.1	Mz	0.11			-0.15	0.174	0.000
		My	0.00			0.00	0.120	0.000
	Pe.C.2	Mz	0.11			-0.13	0.182	0.000
		My	0.00			0.00	0.000	0.000
-	-	-	kNm	kNm	m	kNm	m	m

PE.C. MEMBER FORCES (NX, VY, VZ, MX)

Member	LComb	TC	Nmax	Value	Vb	Vmax	Ve	Mxb	Mxe
S1	Pe.C.1	-	0.00	Vz	0.32	0.32	0.32	0.00	0.00
				Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.52	Vz	0.28	0.28	0.28	0.00	0.00

	Pole 1		
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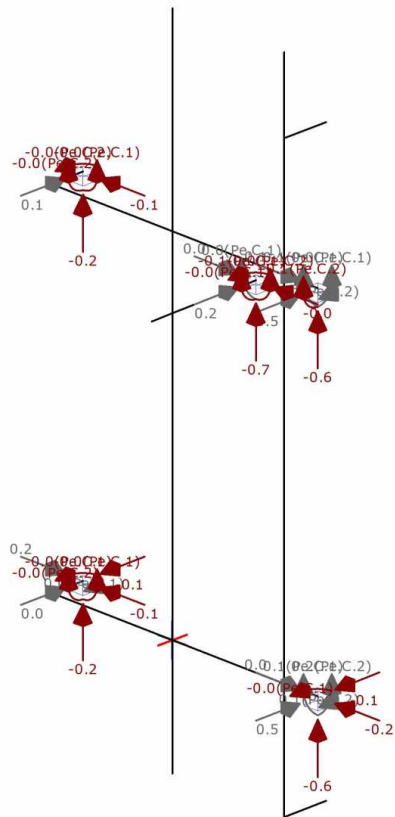
Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S2	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.12 Vz	-0.22	-0.22	-0.20	-0.04	-0.04
			Vy	-0.19	-0.19	-0.19		
	Pe.C.2	T	0.04 Vz	-0.19	-0.19	-0.17	-0.04	-0.04
			Vy	0.08	0.08	0.08		
			-0.23 Vz	-0.61	-0.61	-0.59	0.03	0.03
S3	Pe.C.1	C	Vy	-0.07	-0.07	-0.07		
			0.05 Vz	-0.53	-0.53	-0.52	0.19	0.19
			Vy	0.54	0.54	0.54		
S4	Pe.C.1	C	-0.19 Vz	-0.19	-0.19	-0.15	-0.01	-0.01
			Vy	-0.11	-0.11	-0.11		
			0.09 Vz	-0.16	-0.16	-0.13	-0.06	-0.06
	Pe.C.2	T	Vy	0.00	0.00	0.00		
			-0.19 Vz	0.16	0.20	0.20	0.00	0.00
			Vy	0.12	0.12	0.12		
S5	Pe.C.1	C	0.08 Vz	0.13	0.17	0.17	0.00	0.00
			Vy	-0.04	-0.04	-0.04		
			0.05 Vz	-0.21	-0.21	-0.19	-0.04	-0.04
S6	Pe.C.1	T	Vy	0.07	0.07	0.07		
			0.06 Vz	-0.17	-0.17	-0.16	-0.03	-0.03
			Vy	0.11	0.11	0.11		
S7	Pe.C.1	T	0.10 Vz	-0.63	-0.63	-0.62	0.03	0.03
			Vy	0.01	0.01	0.01		
			0.15 Vz	-0.56	-0.56	-0.54	-0.15	-0.15
	Pe.C.2	T	Vy	0.51	0.51	0.51		
			0.06 Vz	-0.18	-0.18	-0.14	-0.01	-0.01
			Vy	0.04	0.04	0.04		
S8	Pe.C.1	T	0.11 Vz	-0.15	-0.15	-0.12	0.03	0.03
			Vy	0.01	0.01	0.01		
			0.07 Vz	0.15	0.19	0.19	0.00	0.00
	Pe.C.2	T	Vy	-0.05	-0.05	-0.05		
			0.11 Vz	0.12	0.16	0.16	0.01	0.01
			Vy	-0.06	-0.06	-0.06		
S9	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
			-0.52 Vz	0.28	0.28	0.28	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			0.35 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
S10	Pe.C.1	T	0.31 Vz	0.52	0.52	0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
			0.06 Vz	-0.03	-0.03	-0.03	0.00	0.00
	Pe.C.2	T	Vy	0.04	0.04	0.04		
			0.06 Vz	-0.03	-0.03	-0.03	0.00	0.00
			Vy	0.04	0.04	0.04		
S11	Pe.C.1	C	-0.38 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
			-0.34 Vz	-0.52	-0.52	-0.52	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
S12	Pe.C.1	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
			-0.02 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			0.04 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
S13	Pe.C.1	T	0.03 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	T	Vy	0.00	0.00	0.00		
			-0.28 Vz	0.23	0.23	0.23	0.00	0.00
			Vy	0.00	0.00	0.00		
S14	Pe.C.1	C	-0.23 Vz	-0.04	-0.04	-0.04	0.00	0.00
			Vy	0.00	0.00	0.00		
			-0.06 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.05 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
S15	Pe.C.1	T	0.84 Vz	0.23	0.23	0.23	0.00	0.00
			Vy	0.00	0.00	0.00		

			Pole 1					
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Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S19	Pe.C.2	T	0.76 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
S20	Pe.C.1	T	0.23 Vz	0.09	0.09	0.09	0.00	0.00
			Vy	-0.01	-0.01	-0.01		
	Pe.C.2	T	0.19 Vz	0.08	0.08	0.08	0.00	0.00
			Vy	0.00	0.00	0.00		
S21	Pe.C.1	-	0.00 Vz	0.00	-0.01	-0.01	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	-	0.00 Vz	0.00	-0.01	-0.01	0.00	0.00
			Vy	0.00	0.00	0.00		
S22	Pe.C.1	T	0.14 Vz	-0.64	-0.67	-0.67	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	T	0.24 Vz	-0.60	-0.62	-0.62	0.00	0.00
			Vy	0.00	0.00	0.00		
-	-	-	kN -	kN	kN	kN	kNm	kNm

PIC. PE.C. OPLEGREACTIES ENVELOPE

Persistent Loads Combinations



PE.C. SUPPORT REACTIONS

LComb	Support	Node	X	Y	Z	Mx	My	Mz
Pe.C.1	O1	K4	-0.07	-0.23	-0.61	0.10	0.03	-0.03
Pe.C.1	O2	K8	-0.12	0.19	-0.22	-0.04	-0.03	0.01
Pe.C.1	O3	K10	0.01	0.10	-0.63	0.10	0.03	0.01
Pe.C.1	O4	K14	0.05	-0.07	-0.21	-0.04	-0.03	0.00
Pe.C.1	O5	K21	0.14	0.00	-0.67	0.00	-0.15	0.00
Sum Reactions			0.00	0.00	-2.33			
Sum Loads			0.00	0.00	2.33			
Pe.C.2	O1	K4	0.54	0.05	-0.53	0.08	0.19	0.07
Pe.C.2	O2	K8	0.04	-0.08	-0.19	-0.04	-0.02	0.00
Pe.C.2	O3	K10	0.51	0.15	-0.56	0.09	-0.15	0.07
Pe.C.2	O4	K14	0.06	-0.11	-0.17	-0.03	-0.03	0.00

			Pole 1					
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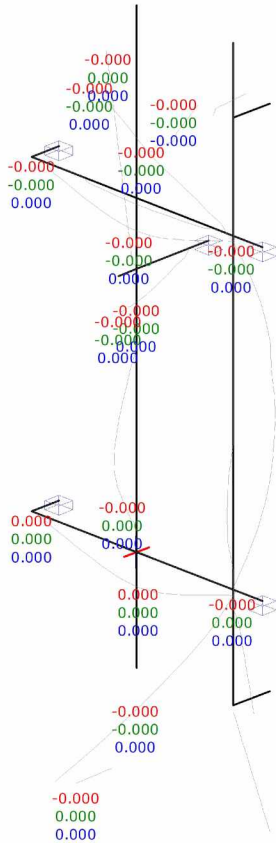
LComb	Support	Node	X	Y	Z	Mx	My	Mz
Pe.C.2	O5	K21	0.24	0.00	-0.62	0.00	-0.13	0.00
	Sum Reactions		1.40	0.00	-2.07			
	Sum Loads		-1.40	0.00	2.07			
-	-	-	kN	kN	kN	kNm	kNm	kNm

L.C. SUPPORT REACTIONS

L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O1	K4	0.00	-0.01	-0.13	0.02	0.00	0.00
B.G.1	O2	K8	0.00	0.00	-0.06	-0.01	-0.01	0.00
B.G.1	O3	K10	0.01	0.03	-0.14	0.02	0.00	0.00
B.G.1	O4	K14	0.02	-0.02	-0.06	-0.01	-0.01	0.00
B.G.1	O5	K21	-0.02	0.00	-0.12	0.00	-0.03	0.00
	Sum Reactions		0.00	0.00	-0.50			
	Sum Loads		0.00	0.00	0.50			
B.G.2	O1	K4	-0.05	-0.16	-0.32	0.05	0.02	-0.02
B.G.2	O2	K8	-0.09	0.13	-0.10	-0.02	-0.01	0.00
B.G.2	O3	K10	-0.01	0.05	-0.33	0.05	0.02	0.00
B.G.2	O4	K14	0.02	-0.03	-0.09	-0.02	-0.01	0.00
B.G.2	O5	K21	0.13	0.00	-0.38	0.00	-0.08	0.00
	Sum Reactions		0.00	0.00	-1.22			
	Sum Loads		0.00	0.00	1.22			
B.G.3	O1	K4	0.48	0.20	0.01	0.00	0.13	0.08
B.G.3	O2	K8	0.12	-0.20	0.01	0.00	0.00	-0.01
B.G.3	O3	K10	0.39	0.04	0.01	0.00	-0.13	0.05
B.G.3	O4	K14	0.02	-0.04	0.01	0.00	-0.01	0.00
B.G.3	O5	K21	0.09	-0.01	-0.02	0.00	0.00	0.00
	Sum Reactions		1.10	0.00	0.00			
	Sum Loads		-1.10	0.00	0.00			
-	-	-	kN	kN	kN	kNm	kNm	kNm

PIC. CH.C. VERPLAATSINGEN ENVELOPE

Characteristic Loads Combinations



CH.C. NODAL DISPLACEMENTS

Node	LComb	X	Y	Z	Xr	Yr	Zr
K1	Ch.C.(w1)	-0.0001	0.0000	0.0000	0.099e-03	-0.466e-03	-0.048e-03
	Ch.C.1	-0.0004	0.0000	0.0000	0.097e-03	-1.198e-03	0.014e-03
K2	Ch.C.(w1)	-0.0001	0.0001	0.0001	0.099e-03	-0.471e-03	-0.048e-03
	Ch.C.1	-0.0004	0.0000	0.0003	0.097e-03	-1.203e-03	0.014e-03
K3	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.004e-03	-0.017e-03	0.006e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.004e-03	-0.015e-03	-0.009e-03
K4	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K5	Ch.C.(w1)	0.0000	0.0000	0.0000	0.099e-03	-0.079e-03	-0.048e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.097e-03	-0.337e-03	0.014e-03
K6	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.004e-03	-0.017e-03	0.006e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.004e-03	-0.015e-03	-0.009e-03
K7	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.085e-03	0.014e-03	0.022e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.083e-03	0.014e-03	0.004e-03
K8	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K9	Ch.C.(w1)	0.0000	0.0000	0.0001	0.001e-03	0.233e-03	0.003e-03
	Ch.C.1	0.0000	0.0000	0.0001	0.001e-03	0.238e-03	-0.008e-03
K10	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K11	Ch.C.(w1)	0.0000	0.0000	0.0000	0.097e-03	-0.074e-03	0.015e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.095e-03	0.194e-03	0.046e-03
K12	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.004e-03	0.006e-03	-0.001e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.004e-03	0.064e-03	-0.011e-03
K13	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.080e-03	0.016e-03	-0.008e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.078e-03	0.021e-03	-0.011e-03
K14	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K15	Ch.C.(w1)	0.0001	0.0000	0.0000	0.097e-03	-0.470e-03	0.015e-03
	Ch.C.1	-0.0002	0.0000	0.0000	0.095e-03	0.293e-03	0.046e-03
K16	Ch.C.(w1)	0.0001	-0.0001	0.0001	0.097e-03	-0.474e-03	0.015e-03
	Ch.C.1	-0.0002	-0.0001	0.0000	0.095e-03	0.288e-03	0.046e-03
K17	Ch.C.(w1)	0.0003	-0.0001	0.0000	0.097e-03	-0.470e-03	0.015e-03
	Ch.C.1	-0.0003	-0.0001	0.0000	0.095e-03	0.293e-03	0.046e-03
K18	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.004e-03	0.006e-03	-0.001e-03
	Ch.C.1	-0.0001	0.0000	0.0001	-0.004e-03	0.064e-03	-0.011e-03
K20	Ch.C.(w1)	0.0000	0.0000	0.0001	0.001e-03	0.088e-03	-0.001e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.000e-03	0.074e-03	-0.006e-03
K21	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K22	Ch.C.(w1)	0.0000	0.0000	0.0001	0.001e-03	0.088e-03	-0.001e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.000e-03	0.075e-03	-0.006e-03
-	-	m	m	m	rad	rad	rad

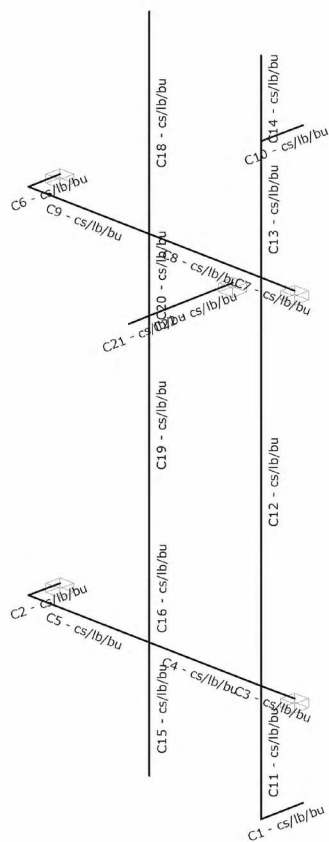
CH.C. EXTREME DEFLECTIONS

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S3	Ch.C. (w1)	0,000	0,000	0,000	0.068	0.0000	0.000	0.0000	0,000	0,000	0,000
S4	Ch.C. (w1)	0,000	0,000	0,000	0.355	0.0000	0.364	0.0000	0,000	0,000	0,000
S4	Ch.C.1	0,000	0,000	0,000	0.354	0.0000	0.166	0.0000	0,000	0,000	0,000
S5	Ch.C. (w1)	0,000	0,000	0,000	0.177	0.0000	0.139	0.0000	0,000	0,000	0,000
S7	Ch.C. (w1)	0,000	0,000	0,000	0.067	0.0000	0.000	0.0000	0,000	0,000	0,000
S8	Ch.C. (w1)	0,000	0,000	0,000	0.352	0.0000	0.376	0.0000	0,000	0,000	0,000
S8	Ch.C.1	0,000	0,000	0,000	0.351	0.0000	0.305	0.0000	0,000	0,000	0,000
S9	Ch.C. (w1)	0,000	0,000	0,000	0.177	0.0000	0.142	0.0000	0,000	0,000	0,000
S9	Ch.C.1	0,000	0,000	0,000	0.177	0.0000	0.457	0.0000	0,000	0,000	0,000

	Pole 1	
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Member	LComb	Node Begin				Member				Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z	
S11	Ch.C.1	0,000	0,000	0,000	0.267	0.0001	0.000	0.0000	0,000	0,000	0,000	
S12	Ch.C. (w1)	0,000	0,000	0,000	0.324	0.0000	0.320	0.0000	0,000	0,000	0,000	
S12	Ch.C.1	0,000	0,000	0,000	0.652	0.0001	0.320	0.0000	0,000	0,000	0,000	
S13	Ch.C. (w1)	0,000	0,000	0,000	0.250	0.0000	0.000	0.0000	0,000	0,000	0,000	
S13	Ch.C.1	0,000	0,000	0,000	0.147	0.0000	0.000	0.0000	0,000	0,000	0,000	
S16	Ch.C. (w1)	0,000	0,000	0,000	0.229	0.0000	0.000	0.0000	0,000	0,000	0,000	
S19	Ch.C.1	0,000	0,000	0,000	0.218	0.0000	0.000	0.0000	0,000	0,000	0,000	
S20	Ch.C. (w1)	0,000	0,000	0,000	0.135	0.0000	0.000	0.0000	0,000	0,000	0,000	
S22	Ch.C. (w1)	0,000	0,000	0,000	0.292	0.0000	0.000	0.0000	0,000	0,000	0,000	
-	-	m	m	m	m	m	m	m	m	m	m	

PIC. STAALCONTROLE



SM DEF.

Structural member	Member (s)
C1	S1
C2	S2
C3	S3
C4	S4
C5	S5
C6	S6
C7	S7
C8	S8
C9	S9
C10	S10
C11	S11

	Pole 1	
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C12	S12
C13	S13
C14	S14
C15	S15
C16	S16
C18	S18
C19	S19
C20	S20
C21	S21
C22	S22

BUCKLING DATA

Member	Section	Y-axis (local system)				Z-axis (local system)		
		Lsys	Method	Lbuc	Lbuc/Lsys	Method	Lbuc	Lbuc/Lsys
C1 - V1 (0.000-0.200)	P3	0.200	Cons.Br.	0.200	1.00	Cons.Br.	0.200	1.00
C2 - V1 (0.000-0.150)	P2	0.150	Cons.Br.	0.150	1.00	Cons.Br.	0.150	1.00
C3 - V1 (0.000-0.160)	P1	0.160	Cons.Br.	0.160	1.00	Cons.Br.	0.160	1.00
C4 - V1 (0.000-0.535)	P1	0.540	Cons.Br.	0.535	1.00	Cons.Br.	0.535	1.00
C5 - V1 (0.000-0.580)	P1	0.580	Cons.Br.	0.580	1.00	Cons.Br.	0.580	1.00
C10 - V1 (0.000-0.200)	P3	0.200	Cons.Br.	0.200	1.00	Cons.Br.	0.200	1.00
C12 - V1 (0.000-1.500)	P1	1.500	Cons.Br.	1.500	1.00	Cons.Br.	1.500	1.00
C13 - V1 (0.000-0.500)	P1	0.500	Cons.Br.	0.500	1.00	Cons.Br.	0.500	1.00
C14 - V1 (0.000-0.315)	P1	0.320	Cons.Br.	0.315	1.00	Cons.Br.	0.315	1.00
C16 - V1 (0.000-0.400)	P1	0.400	Cons.Br.	0.400	1.00	Cons.Br.	0.400	1.00
C18 - V1 (0.000-0.815)	P1	0.820	Cons.Br.	0.815	1.00	Cons.Br.	0.815	1.00
-	-	m	-	m	-	-	m	-

LATERAL BUCKLING DATA

Member	Section	Begin:	End:	Lat. Supp. upper	Lat. Supp. lower	Load attachment
C1 - V1 (0.000-0.200)	P3	Cantilever	Supported			Center
C2 - V1 (0.000-0.150)	P2	Supported	Supported			Center
C3 - V1 (0.000-0.160)	P1	Supported	Supported			Center
C4 - V1 (0.000-0.535)	P1	Supported	Supported			Center
C5 - V1 (0.000-0.580)	P1	Supported	Supported			Center
C6 - V1 (0.000-0.150)	P2	Supported	Supported			Center
C7 - V1 (0.000-0.160)	P1	Supported	Supported			Center
C8 - V1 (0.000-0.535)	P1	Supported	Supported			Center
C9 - V1 (0.000-0.580)	P1	Supported	Supported			Center
C10 - V1 (0.000-0.200)	P3	Cantilever	Supported			Center
C11 - V1 (0.000-0.490)	P1	Supported	Supported			Center
C12 - V1 (0.000-1.500)	P1	Supported	Supported			Center
C13 - V1 (0.000-0.500)	P1	Supported	Supported			Center
C14 - V1 (0.000-0.315)	P1	Supported	Cantilever			Center
C15 - V1 (0.000-0.490)	P1	Cantilever	Supported			Center
C16 - V1 (0.000-0.400)	P1	Supported	Supported			Center
C18 - V1 (0.000-0.815)	P1	Supported	Cantilever			Center
C19 - V1 (0.000-0.800)	P1	Supported	Supported			Center
C20 - V1 (0.000-0.300)	P1	Supported	Supported			Center
C21 - V1 (0.000-0.100)	P1	Cantilever	Supported			Center
C22 - V1 (0.000-0.400)	P1	Supported	Supported			Center
-	-	-	-	m	m	-

STEEL CODECHECK RESULTS WITH SECTION DATA NEN-EN1993-1-1:2016/NB:2016

Assumptions of Analysis for steel codecheck

5.1.2.e; $\alpha_{cr} = 531.44 > 10$;

Section data member C1-V1 (0.000-0.200)

C100	Analysis	Steel S235	$f_{yd}(\text{applied}) = 215 \text{ N/mm}^2$	
$h = 100,0 \text{ mm}$	$A = 7,85e-03 \text{ m}^2$	$W_{y,el} = 981.7e-07 \text{ m}^3$	$W_{y,pl} = 166.7e-06 \text{ m}^3$	
$b = 100,0 \text{ mm}$	$I_y = 490.9e-08 \text{ m}^4$	$W_{z,el} = 981.7e-07 \text{ m}^3$	$W_{z,pl} = 166.7e-06 \text{ m}^3$	
$t_f = 0,0 \text{ mm}$	$I_z = 490.9e-08 \text{ m}^4$	$A_{w,y,el} = 5.89e-03 \text{ m}^2$	$A_{w,y,pl} = 5.89e-03 \text{ m}^2$	

Pole 1			
tw = 0,0 mm	Mass/m = 61,7 kg/m	Aw;z;el = 5.89e-03 m2	Aw;z;pl = 5.89e-03 m2
r = 0,0 mm		It = 981.7e-08 m4	Iwa = 0.0e-02 m6
Crosscheck member C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.1 at 0,200 m		Section class = 1	
N;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,1 kNm	
	Vz;Ed = 0,3 kN	Mz;Ed = 0,0 kNm	
N;Rd = 1 688,6 kN	Vy;Rd = 731,2 kN	MyRd = 35,8 kNm	
	Vz;Rd = 731,2 kN	MzRd = 35,8 kNm	
NEN-EN1993-1-1(6.12): UC = 0,00 < 1			
Lateral Buckling member C1-V1 (0.000-0.200)			
Equi. section: C100			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Cantilever	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,200 m	lst = 0,400 m
Lsys = 0,200 m	Lg = 0,200 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,200 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section with h/b < 3			
Stability Check C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.2			
N;Ed = -0,5 kN	Nb;Rd;y = 1 688,6 kN	Nb;Rd;z = 1 688,6 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,200 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,200 m
Chi;y = 1,00		Buckl. curve: C	
Chi;z = 1,00		Buckl. curve: C	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Bending & Compression C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.2		Section class = 1	
N;Ed = -0,5 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,60	Cmz = 1,00	CmLT = 0,60	
Kyy = 0,600	Kyz = 0,600	Kzy = 0,681	Kzz = 1,000
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			
Section data member C2-V1 (0.000-0.150)			
K60x60x4x4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,0 mm	A = 0,90e-03 m2	Wy;el = 156.9e-07 m3	Wy;pl = 188.5e-07 m3
b = 60,0 mm	Iy = 470.7e-09 m4	Wz;el = 156.9e-07 m3	Wz;pl = 188.5e-07 m3
tf = 4,0 mm	Iz = 470.7e-09 m4	Aw;y;el = 4.48e-04 m2	Aw;y;pl = 4.48e-04 m2
tw = 4,0 mm	Mass/m = 7,0 kg/m	Aw;z;el = 4.48e-04 m2	Aw;z;pl = 4.48e-04 m2
r = 0,0 mm		It = 702.5e-09 m4	Iwa = 369.0e-12 m6
Crosscheck member C2-V1 (0.000-0.150)			
Decisive combination: Pe.C.1 at 0,150 m		Section class = 1	
Nx;Ed = -0,1 kN	Vy;Ed = -0,2 kN	My;Ed = 0,0 kNm	a3(y) = 0,464
	Vz;Ed = -0,2 kN	Mz;Ed = 0,0 kNm	a4(y) = 0,478
Nc;Rd = 210,6 kN	Vy;Rd = 60,8 kN	My;Rd = 4,4 kNm	a3(z) = 0,464

	Pole 1	
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NV _y ;R _d = 210,6 kN	V _z ;R _d = 60,8 kN	M _z ;R _d = 4,4 kNm	a ₄ (z) = 0,478
α ₁ = 1,000	NV _z ;R _d = 210,6 kN	MV _y ;R _d = 4,4 kNm	MV _z ;R _d = 4,4 kNm
α ₂ = 2,000	β ₁ = 1,000	MNV _y ;R _d = 4,4 kNm	q(y) = 1,030
NEN-EN1993-1-1(6.31): UC = 0,01 < 1	β ₂ = 0,750	MNV _z ;R _d = 4,4 kNm	q(z) = 1,030

Lateral Buckling member C2-V1 (0.000-0.150)

Equi. section: K60x60x4x4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	X _b ;l _{st} = 0,000 m	X _e ;l _{st} = 0,150 m	l _{st} = 0,150 m
L _{sys} = 0,150 m	L _g = 0,150 m	S = 0,037 m	I _{wa} = 3.6903e-10 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M _i ;E _d = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	l _{kip} = 0,150 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C2-V1 (0.000-0.150)

Decisive combination: Pe.C.1			
N _i ;E _d = -0,1 kN	N _b ;R _d ;y = 210,6 kN	N _b ;R _d ;z = 210,6 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	L _{buc} Y = 0,150 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	L _{buc} Z = 0,150 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C2-V1 (0.000-0.150)

Decisive combination: Pe.C.1		Section class = 1	
N _i ;E _d = -0,1 kN	My;E _d = 0,0 kNm	M _z ;E _d = 0,0 kNm	
	Delta;My;E _d = 0,0 kNm	Delta;M _z ;E _d = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
C _{my} = 0,55	C _{mz} = 0,66	C _m LT = 0,55	
K _{yy} = 0,546	K _{yz} = 0,397	K _{zy} = 0,328	K _{zz} = 0,661
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1			

Section data member C3-V1 (0.000-0.160)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	I _y = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
t _f = 4,0 mm	I _z = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
t _w = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		I _t = 563.5e-09 m ⁴	I _{wa} = 0.0e-02 m ⁶

Crosscheck member C3-V1 (0.000-0.160)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
N _x ;E _d = 0,0 kN	V _y ;E _d = 0,5 kN	My;E _d = 0,1 kNm	
	V _z ;E _d = -0,5 kN	Mz;E _d = -0,1 kNm	
N _c ;R _d = 166,3 kN	V _y ;R _d = 61,1 kN	My;R _d = 3,0 kNm	
	V _z ;R _d = 61,1 kN	Mz;R _d = 3,0 kNm	
NV _y ;R _d = 166,3 kN	NV _z ;R _d = 166,3 kN	MV _y ;R _d = 3,0 kNm	MV _z ;R _d = 3,0 kNm
α ₁ = 2,000	β ₁ = 1,000	MNV _y ;R _d = 3,0 kNm	q(y) = 1,030
α ₂ = 2,000	β ₂ = 1,000	MNV _z ;R _d = 3,0 kNm	q(z) = 1,030
NEN-EN1993-1-1(6.1): UC = 0,08 < 1			

	Pole 1	
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Lateral Buckling member C3-V1 (0.000-0.160)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,160 m	lst = 0,160 m
Lsys = 0,160 m	Lg = 0,160 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,160 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C3-V1 (0.000-0.160)

Decisive combination: Pe.C.1			
N;Ed = -0,2 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,160 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,160 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C3-V1 (0.000-0.160)

Decisive combination: Pe.C.1		Section class = 1	
N;Ed = -0,2 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,60	Cmz = 0,84	CmLT = 0,60	
Kyy = 0,605	Kyz = 0,505	Kzy = 0,363	Kzz = 0,842
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,03 < 1			

Section data member C4-V1 (0.000-0.535)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	Iy = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	Iz = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		It = 563.5e-09 m ⁴	Iwa = 0.0e-02 m ⁶

Crosscheck member C4-V1 (0.000-0.535)

Decisive combination: Pe.C.1 at 0,535 m		Section class = 1	
Nx;Ed = -0,2 kN	Vy;Ed = -0,1 kN	My;Ed = -0,1 kNm	
	Vz;Ed = -0,2 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV;z;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,02 < 1			

Lateral Buckling member C4-V1 (0.000-0.535)

Equi. section: N60.3/4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			

	Pole 1	
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Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,535 m	lst = 0,535 m
Lsys = 0,535 m	Lg = 0,535 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,535 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = -0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C4-V1 (0.000-0.535)

Decisive combination: Pe.C.1			
N;Ed = -0,2 kN	Nb;Rd;y = 163,1 kN	Nb;Rd;z = 163,1 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,535 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,535 m
Chi;y = 0,98		Buckl. curve: A	
Chi;z = 0,98		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C4-V1 (0.000-0.535)

Decisive combination: Pe.C.1		Section class = 1	
N;Ed = -0,2 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = -0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,42	Cmz = 0,40	CmLT = 0,42	
Kyy = 0,416	Kyz = 0,240	Kzy = 0,250	Kzz = 0,400
Chi;y = 0,98	Chi;z = 0,98	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1			

Section data member C5-V1 (0.000-0.580)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C5-V1 (0.000-0.580)

Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
Nx;Ed = -0,2 kN	Vy;Ed = 0,1 kN	My;Ed = -0,1 kNm	
	Vz;Ed = 0,2 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV;z;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,02 < 1			

Lateral Buckling member C5-V1 (0.000-0.580)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,580 m	lst = 0,580 m

Pole 1			
Lsys = 0,580 m	Lg = 0,580 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,580 m		UC(z) = 0,00
My;begin = -0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			
Stability Check C5-V1 (0.000-0.580)			
Decisive combination: Pe.C.1			
N;Ed = -0,2 kN	Nb;Rd;y = 162,1 kN	Nb;Rd;z = 162,1 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,580 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,580 m
Chi;y = 0,98		Buckl. curve: A	
Chi;z = 0,98		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Bending & Compression C5-V1 (0.000-0.580)			
Decisive combination: Pe.C.1		Section class = 1	
N;Ed = -0,2 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = -0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,40	Cmz = 0,40	CmLT = 0,40	
Kyy = 0,400	Kyz = 0,240	Kzy = 0,240	Kzz = 0,400
Chi;y = 0,98	Chi;z = 0,98	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1			
Section data member C6-V1 (0.000-0.150)			
K60x60x4x4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,0 mm	A = 0,90e-03 m2	Wy;el = 156.9e-07 m3	Wy;pl = 188.5e-07 m3
b = 60,0 mm	Iy = 470.7e-09 m4	Wz;el = 156.9e-07 m3	Wz;pl = 188.5e-07 m3
tf = 4,0 mm	Iz = 470.7e-09 m4	Aw;y;el = 4.48e-04 m2	Aw;y;pl = 4.48e-04 m2
tw = 4,0 mm	Mass/m = 7,0 kg/m	Aw;z;el = 4.48e-04 m2	Aw;z;pl = 4.48e-04 m2
r = 0,0 mm		It = 702.5e-09 m4	Iwa = 369.0e-12 m6
Crosscheck member C6-V1 (0.000-0.150)			
Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
Nx;Ed = 0,1 kN	Vy;Ed = 0,1 kN	My;Ed = 0,0 kNm	a3(y) = 0,464
	Vz;Ed = -0,2 kN	Mz;Ed = 0,0 kNm	a4(y) = 0,478
Nc;Rd = 210,6 kN	Vy;Rd = 60,8 kN	My;Rd = 4,4 kNm	a3(z) = 0,464
	Vz;Rd = 60,8 kN	Mz;Rd = 4,4 kNm	a4(z) = 0,478
NVy;Rd = 210,6 kN	NVz;Rd = 210,6 kN	MV;y;Rd = 4,4 kNm	MV;z;Rd = 4,4 kNm
$\alpha_1 = 1,000$	$\beta_1 = 1,000$	MNV;y;Rd = 4,4 kNm	q(y) = 1,030
$\alpha_2 = 2,000$	$\beta_2 = 0,750$	MNV;z;Rd = 4,4kNm	q(z) = 1,030
NEN-EN1993-1-1(6.31): UC = 0,01 < 1			
Lateral Buckling member C6-V1 (0.000-0.150)			
Equi. section: K60x60x4x4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,150 m	lst = 0,150 m
Lsys = 0,150 m	Lg = 0,150 m	S = 0,037 m	Iwa = 3.6903e-10 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,150 m		UC(z) = 0,00

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My;begin = 0,0 kNm My;end = 0,0 kNm
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)

Section data member C7-V1 (0.000-0.160)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	W _{y;el} = 934.4e-08 m ³	W _{y;pl} = 127.0e-07 m ³
b = 60,3 mm	I _y = 281.7e-09 m ⁴	W _{z;el} = 934.4e-08 m ³	W _{z;pl} = 127.0e-07 m ³
t _f = 4,0 mm	I _z = 281.7e-09 m ⁴	A _{w;y;el} = 4.50e-04 m ²	A _{w;y;pl} = 4.50e-04 m ²
t _w = 4,0 mm	Mass/m = 5,6 kg/m	A _{w;z;el} = 4.50e-04 m ²	A _{w;z;pl} = 4.50e-04 m ²
r = 26,1 mm		I _t = 563.5e-09 m ⁴	I _{wa} = 0.0e-02 m ⁶

Crosscheck member C7-V1 (0.000-0.160)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
N _{x;Ed} = 0,1 kN	V _{y;Ed} = 0,5 kN	M _{y;Ed} = 0,1 kNm	
	V _{z;Ed} = -0,6 kN	M _{z;Ed} = -0,1 kNm	
N _{c;Rd} = 166,3 kN	V _{y;Rd} = 61,1 kN	M _{y;Rd} = 3,0 kNm	
	V _{z;Rd} = 61,1 kN	M _{z;Rd} = 3,0 kNm	
NV _{y;Rd} = 166,3 kN	NV _{z;Rd} = 166,3 kN	MV _{y;Rd} = 3,0 kNm	MV _{z;Rd} = 3,0 kNm
α ₁ = 2,000	β ₁ = 1,000	MNV _{y;Rd} = 3,0 kNm	q(y) = 1,030
α ₂ = 2,000	β ₂ = 1,000	MNV _{z;Rd} = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(6.1): UC = 0,08 < 1			

Lateral Buckling member C7-V1 (0.000-0.160)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	X _{b;l} = 0,000 m	X _{e;l} = 0,160 m	l _{st} = 0,160 m
L _{sys} = 0,160 m	L _g = 0,160 m	S = 0,000 m	I _{wa} = 0.0000e+00 m ⁶
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
5.1.2.a , LT(Pe.C.2) = 1,00	M _{i;Ed} = 0,1 kNm		UC(y) = 0,00
5.1.2.a , LT,Z = 1,00	l _{kip} = 0,160 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C8-V1 (0.000-0.535)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	W _{y;el} = 934.4e-08 m ³	W _{y;pl} = 127.0e-07 m ³
b = 60,3 mm	I _y = 281.7e-09 m ⁴	W _{z;el} = 934.4e-08 m ³	W _{z;pl} = 127.0e-07 m ³
t _f = 4,0 mm	I _z = 281.7e-09 m ⁴	A _{w;y;el} = 4.50e-04 m ²	A _{w;y;pl} = 4.50e-04 m ²
t _w = 4,0 mm	Mass/m = 5,6 kg/m	A _{w;z;el} = 4.50e-04 m ²	A _{w;z;pl} = 4.50e-04 m ²
r = 26,1 mm		I _t = 563.5e-09 m ⁴	I _{wa} = 0.0e-02 m ⁶

Crosscheck member C8-V1 (0.000-0.535)

Decisive combination: Pe.C.1 at 0,535 m		Section class = 1	
N _{x;Ed} = 0,1 kN	V _{y;Ed} = 0,0 kN	M _{y;Ed} = -0,1 kNm	
	V _{z;Ed} = -0,1 kN	M _{z;Ed} = 0,0 kNm	
N _{c;Rd} = 166,3 kN	V _{y;Rd} = 61,1 kN	M _{y;Rd} = 3,0 kNm	
	V _{z;Rd} = 61,1 kN	M _{z;Rd} = 3,0 kNm	
NV _{y;Rd} = 166,3 kN	NV _{z;Rd} = 166,3 kN	MV _{y;Rd} = 3,0 kNm	MV _{z;Rd} = 3,0 kNm
α ₁ = 2,000	β ₁ = 1,000	MNV _{y;Rd} = 3,0 kNm	q(y) = 1,030
α ₂ = 2,000	β ₂ = 1,000	MNV _{z;Rd} = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,02 < 1			

Lateral Buckling member C8-V1 (0.000-0.535)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			

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Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,535 m	lst = 0,535 m
Lsys = 0,535 m	Lg = 0,535 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,535 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C9-V1 (0.000-0.580)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	W _y ;el = 934.4e-08 m3	W _y ;pl = 127.0e-07 m3
b = 60,3 mm	I _y = 281.7e-09 m4	W _z ;el = 934.4e-08 m3	W _z ;pl = 127.0e-07 m3
t _f = 4,0 mm	I _z = 281.7e-09 m4	A _w ;y;el = 4.50e-04 m2	A _w ;y;pl = 4.50e-04 m2
t _w = 4,0 mm	Mass/m = 5,6 kg/m	A _w ;z;el = 4.50e-04 m2	A _w ;z;pl = 4.50e-04 m2
r = 26,1 mm		I _t = 563.5e-09 m4	I _{wa} = 0.0e-02 m6

Crosscheck member C9-V1 (0.000-0.580)

Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
N _x ;Ed = 0,1 kN	V _y ;Ed = 0,0 kN	M _y ;Ed = -0,1 kNm	
	V _z ;Ed = 0,1 kN	M _z ;Ed = 0,0 kNm	
N _c ;Rd = 166,3 kN	V _y ;Rd = 61,1 kN	M _y ;Rd = 3,0 kNm	
	V _z ;Rd = 61,1 kN	M _z ;Rd = 3,0 kNm	
NV _y ;Rd = 166,3 kN	NV _z ;Rd = 166,3 kN	MV _y ;Rd = 3,0 kNm	MV _z ;Rd = 3,0 kNm
α ₁ = 2,000	β ₁ = 1,000	MNV _y ;Rd = 3,0 kNm	q(y) = 1,030
α ₂ = 2,000	β ₂ = 1,000	MNV _z ;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,02 < 1			

Lateral Buckling member C9-V1 (0.000-0.580)

Equi. section: N60.3/4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,580 m	lst = 0,580 m
Lsys = 0,580 m	Lg = 0,580 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,580 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C10-V1 (0.000-0.200)

C100	Analysis	Steel S235	f _{yd} (applied) = 215 N/mm2
h = 100,0 mm	A = 7,85e-03 m2	W _y ;el = 981.7e-07 m3	W _y ;pl = 166.7e-06 m3
b = 100,0 mm	I _y = 490.9e-08 m4	W _z ;el = 981.7e-07 m3	W _z ;pl = 166.7e-06 m3
t _f = 0,0 mm	I _z = 490.9e-08 m4	A _w ;y;el = 5.89e-03 m2	A _w ;y;pl = 5.89e-03 m2
t _w = 0,0 mm	Mass/m = 61,7 kg/m	A _w ;z;el = 5.89e-03 m2	A _w ;z;pl = 5.89e-03 m2
r = 0,0 mm		I _t = 981.7e-08 m4	I _{wa} = 0.0e-02 m6

Crosscheck member C10-V1 (0.000-0.200)

Decisive combination: Pe.C.1 at 0,200 m		Section class = 1	
N _x ;Ed = 0,0 kN	V _y ;Ed = 0,0 kN	M _y ;Ed = 0,1 kNm	a1 = 0,500
	V _z ;Ed = 0,3 kN	M _z ;Ed = 0,0 kNm	a2 = 0,001
N _c ;Rd = 1 688,6 kN	V _y ;Rd = 731,2 kN	M _y ;Rd = 35,8 kNm	p = 0,998
	V _z ;Rd = 731,2 kN	M _z ;Rd = 35,8 kNm	q = 1,030
NV _y ;Rd = 0,0 kN	NV _z ;Rd = 0,0 kN	MV _y ;Rd = 35,8 kNm	MV _z ;Rd = 35,8 kNm

	Pole 1	
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NEN-EN1993-1-1(6.12): $UC = 0,00 < 1$

Lateral Buckling member C10-V1 (0.000-0.200)

Equi. section: C100			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Cantilever	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,200 m	lst = 0,400 m
Lsys = 0,200 m	Lg = 0,200 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,200 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section with h/b < 3			

Stability Check C10-V1 (0.000-0.200)

Decisive combination: Pe.C.2			
N;Ed = -0,5 kN	Nb;Rd;y = 1 688,6 kN	Nb;Rd;z = 1 688,6 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,200 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,200 m
Chi;y = 1,00		Buckl. curve: C	
Chi;z = 1,00		Buckl. curve: C	
NEN-EN1993-1-1(6.46): $UC = 0,00 < 1$			

Bending & Compression C10-V1 (0.000-0.200)

Decisive combination: Pe.C.2	Lat-tors. Sensitive Yes	Section class = 1	
N;Ed = -0,5 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,60	Cmz = 1,00	CmLT = 0,60	
Kyy = 0,600	Kyz = 0,600	Kzy = 0,681	Kzz = 1,000
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): $UC = 0,00 < 1$			

Section data member C11-V1 (0.000-0.490)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	Iy = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	Iz = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		It = 563.5e-09 m ⁴	Iwa = 0.0e-02 m ⁶

Crosscheck member C11-V1 (0.000-0.490)

Decisive combination: Pe.C.2 at 0,490 m		Section class = 1	
Nx;Ed = 0,3 kN	Vy;Ed = 0,0 kN	My;Ed = 0,3 kNm	
	Vz;Ed = 0,5 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
NEN-EN1993-1-1(NB.33): $UC = 0,10 < 1$			

Lateral Buckling member C11-V1 (0.000-0.490)

Equi. section: N60.3/4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			

	Pole 1	
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Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.7	M = 0,3kN/m	MBeta = 0,1	F = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,490 m	lst = 0,490 m
Lsys = 0,490 m	Lg = 0,490 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,63	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,3 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,490 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,3 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C12-V1 (0.000-1.500)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C12-V1 (0.000-1.500)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,1 kNm	
	Vz;Ed = 0,0 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV;z;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,02 < 1			

Lateral Buckling member C12-V1 (0.000-1.500)

Equi. section: N60.3/4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (1)	M = 0,1kN/m	MBeta = 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 1,500 m	lst = 1,500 m
Lsys = 1,500 m	Lg = 1,500 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,36	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 1,500 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C12-V1 (0.000-1.500)

Decisive combination: Pe.C.2			
N;Ed = 0,0 kN	Nb;Rd;y = 132,3 kN	Nb;Rd;z = 132,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 1,500 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 1,500 m
Chi;y = 0,80		Buckl. curve: A	
Chi;z = 0,80		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C12-V1 (0.000-1.500)

Decisive combination: Pe.C.2		Section class = 1	
N;Ed = 0,0 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	

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$M_z = 0,0 \text{ kNm}$ $M_z; \Psi_i = 0,0 \text{ kNm}$ $M_z; s = 0,0 \text{ kNm}$
 $C_{my} = 0,77$ $C_{mz} = 0,40$ $C_{mLT} = 0,77$
 $K_{yy} = 0,769$ $K_{yz} = 0,240$ $K_{zy} = 0,462$ $K_{zz} = 0,400$
 $\chi_i; y = 0,80$ $\chi_i; z = 0,80$ $\chi_i; LT = 1,00$
 NEN-EN1993-1-1(6.61&6.62): $UC = 0,02 < 1$

Section data member C13-V1 (0.000-0.500)

N60.3/4	Analysis	Steel S235H(EN10210-1)	$f_{yd}(\text{applied}) = 235 \text{ N/mm}^2$
$h = 60,3 \text{ mm}$	$A = 0,71e-03 \text{ m}^2$	$W_{y;el} = 934.4e-08 \text{ m}^3$	$W_{y;pl} = 127.0e-07 \text{ m}^3$
$b = 60,3 \text{ mm}$	$I_y = 281.7e-09 \text{ m}^4$	$W_{z;el} = 934.4e-08 \text{ m}^3$	$W_{z;pl} = 127.0e-07 \text{ m}^3$
$t_f = 4,0 \text{ mm}$	$I_z = 281.7e-09 \text{ m}^4$	$A_{w;y;el} = 4.50e-04 \text{ m}^2$	$A_{w;y;pl} = 4.50e-04 \text{ m}^2$
$t_w = 4,0 \text{ mm}$	$\text{Mass}/m = 5,6 \text{ kg/m}$	$A_{w;z;el} = 4.50e-04 \text{ m}^2$	$A_{w;z;pl} = 4.50e-04 \text{ m}^2$
$r = 26,1 \text{ mm}$		$I_t = 563.5e-09 \text{ m}^4$	$I_{wa} = 0.0e-02 \text{ m}^6$

Crosscheck member C13-V1 (0.000-0.500)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
$N_x; Ed = -0,3 \text{ kN}$	$V_y; Ed = 0,0 \text{ kN}$	$M_y; Ed = 0,2 \text{ kNm}$	
	$V_z; Ed = -0,5 \text{ kN}$	$M_z; Ed = 0,0 \text{ kNm}$	
$N_c; Rd = 166,3 \text{ kN}$	$V_y; Rd = 61,1 \text{ kN}$	$M_y; Rd = 3,0 \text{ kNm}$	
	$V_z; Rd = 61,1 \text{ kN}$	$M_z; Rd = 3,0 \text{ kNm}$	
$NV_y; Rd = 166,3 \text{ kN}$	$NV_z; Rd = 166,3 \text{ kN}$	$MV_y; Rd = 3,0 \text{ kNm}$	$MV_z; Rd = 3,0 \text{ kNm}$
NEN-EN1993-1-1(NB.33): $UC = 0,07 < 1$			

Lateral Buckling member C13-V1 (0.000-0.500)

Equi. section: N60.3/4		Instab. curve Lat. Buck.: d	
Decisive combination: Pe.C.2			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	$b\text{-eff}(\text{Begin}) = 0,000$	$b\text{-eff}(\text{End}) = 0,000$
Table used: NB.NB.1 (1)	$M = 0,2 \text{ kNm/m}$	$MBeta = -0,1$	
Upper flange decisive	$X_b; l_{st} = 0,000 \text{ m}$	$X_e; l_{st} = 0,500 \text{ m}$	$l_{st} = 0,500 \text{ m}$
$L_{sys} = 0,500 \text{ m}$	$L_g = 0,500 \text{ m}$	$S = 0,000 \text{ m}$	$I_{wa} = 0.0000e+00 \text{ m}^6$
$C_1 = 2,06$	$C_2 = 0,00 \text{ (table)}$	$C_2(\text{applied}) = 0,00$	$C = 0,00$
$M_{cr} = 0,0 \text{ kNm}$	$k_{red} = 1,0$	$\text{Lam-rel} = 0,00$	Section class 1
$\chi_i; LT(\text{Pe.C.2}) = 1,00$	$M; Ed = 0,2 \text{ kNm}$		$UC(y) = 0,00$
$\chi_i; LT, Z = 1,00$	$I_{kip} = 0,500 \text{ m}$		$UC(z) = 0,00$
$M_y; \text{begin} = 0,2 \text{ kNm}$	$M_y; \text{end} = -0,1 \text{ kNm}$		
NEN-EN1993-1-1(6.54): $UC = 0,00 < 1$ Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C13-V1 (0.000-0.500)

Decisive combination: Pe.C.2			
$N; Ed = -0,3 \text{ kN}$	$N_b; Rd; y = 163,8 \text{ kN}$	$N_b; Rd; z = 163,8 \text{ kN}$	
Method Y = Cons.Br.	$Ca(y) = 0,000$	$Cb(y) = 0,000$	$L_{buc} Y = 0,500 \text{ m}$
Method Z = Cons.Br.	$Ca(z) = N/A$	$Cb(z) = N/A$	$L_{buc} Z = 0,500 \text{ m}$
$\chi_i; y = 0,99$		Buckl. curve: A	
$\chi_i; z = 0,99$		Buckl. curve: A	
NEN-EN1993-1-1(6.46): $UC = 0,00 < 1$			

Bending & Compression C13-V1 (0.000-0.500)

Decisive combination: Pe.C.2		Section class = 1	
$N; Ed = -0,3 \text{ kN}$	$M_y; Ed = 0,2 \text{ kNm}$	$M_z; Ed = 0,0 \text{ kNm}$	
	$\Delta; M_y; Ed = 0,0 \text{ kNm}$	$\Delta; M_z; Ed = 0,0 \text{ kNm}$	
$M_y = 0,2 \text{ kNm}$	$M_y; \Psi_i = -0,1 \text{ kNm}$	$M_y; s = 0,1 \text{ kNm}$	
$M_z = 0,0 \text{ kNm}$	$M_z; \Psi_i = 0,0 \text{ kNm}$	$M_z; s = 0,0 \text{ kNm}$	
$C_{my} = 0,49$	$C_{mz} = 1,00$	$C_{mLT} = 0,49$	
$K_{yy} = 0,491$	$K_{yz} = 0,600$	$K_{zy} = 0,294$	$K_{zz} = 1,000$
$\chi_i; y = 0,99$	$\chi_i; z = 0,99$	$\chi_i; LT = 1,00$	
NEN-EN1993-1-1(6.61&6.62): $UC = 0,04 < 1$			

Section data member C14-V1 (0.000-0.315)

N60.3/4	Analysis	Steel S235H(EN10210-1)	$f_{yd}(\text{applied}) = 235 \text{ N/mm}^2$
$h = 60,3 \text{ mm}$	$A = 0,71\text{e-}03 \text{ m}^2$	$W_{y,el} = 934.4\text{e-}08 \text{ m}^3$	$W_{y,pl} = 127.0\text{e-}07 \text{ m}^3$
$b = 60,3 \text{ mm}$	$I_y = 281.7\text{e-}09 \text{ m}^4$	$W_{z,el} = 934.4\text{e-}08 \text{ m}^3$	$W_{z,pl} = 127.0\text{e-}07 \text{ m}^3$
$t_f = 4,0 \text{ mm}$	$I_z = 281.7\text{e-}09 \text{ m}^4$	$A_{w,y,el} = 4.50\text{e-}04 \text{ m}^2$	$A_{w,y,pl} = 4.50\text{e-}04 \text{ m}^2$
$t_w = 4,0 \text{ mm}$	$\text{Mass}/m = 5,6 \text{ kg/m}$	$A_{w,z,el} = 4.50\text{e-}04 \text{ m}^2$	$A_{w,z,pl} = 4.50\text{e-}04 \text{ m}^2$
$r = 26,1 \text{ mm}$		$I_t = 563.5\text{e-}09 \text{ m}^4$	$I_{wa} = 0.0\text{e-}02 \text{ m}^6$

Crosscheck member C14-V1 (0.000-0.315)

Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
$N_x;Ed = 0,0 \text{ kN}$	$V_y;Ed = 0,0 \text{ kN}$	$M_y;Ed = 0,0 \text{ kNm}$	
	$V_z;Ed = 0,0 \text{ kN}$	$M_z;Ed = 0,0 \text{ kNm}$	
$N_c;Rd = 166,3 \text{ kN}$	$V_y;Rd = 61,1 \text{ kN}$	$M_y;Rd = 3,0 \text{ kNm}$	
	$V_z;Rd = 61,1 \text{ kN}$	$M_z;Rd = 3,0 \text{ kNm}$	
$NV_y;Rd = 0,0 \text{ kN}$	$NV_z;Rd = 0,0 \text{ kN}$	$MV_y;Rd = 0,0 \text{ kNm}$	$MV_z;Rd = 0,0 \text{ kNm}$
NEN-EN1993-1-1(6.9): UC = 0,00 < 1			

Lateral Buckling member C14-V1 (0.000-0.315)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Cantilever	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;l _{st} = 0,000 m	Xe;l _{st} = 0,315 m	l _{st} = 0,630 m
L _{sys} = 0,315 m	L _g = 0,315 m	S = 0,000 m	I _{wa} = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	I _{kip} = 0,315 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: no bending			

Stability Check C14-V1 (0.000-0.315)

Decisive combination: Pe.C.1			
$N_i;Ed = 0,0 \text{ kN}$	$N_b;Rd;y = 166,3 \text{ kN}$	$N_b;Rd;z = 166,3 \text{ kN}$	
Method Y = Cons.Br.	$Ca(y) = 0,000$	$Cb(y) = 0,000$	$L_{buc} Y = 0,315 \text{ m}$
Method Z = Cons.Br.	$Ca(z) = N/A$	$Cb(z) = N/A$	$L_{buc} Z = 0,315 \text{ m}$
$\chi_i;y = 1,00$		Buckl. curve: A	
$\chi_i;z = 1,00$		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C14-V1 (0.000-0.315)

Decisive combination: Pe.C.1		Section class = 1	
$N_i;Ed = 0,0 \text{ kN}$	$M_y;Ed = 0,0 \text{ kNm}$	$M_z;Ed = 0,0 \text{ kNm}$	
	$\Delta;M_y;Ed = 0,0 \text{ kNm}$	$\Delta;M_z;Ed = 0,0 \text{ kNm}$	
$M_y = 0,0 \text{ kNm}$	$M_y;\Psi_i = 0,0 \text{ kNm}$	$M_y;s = 0,0 \text{ kNm}$	
$M_z = 0,0 \text{ kNm}$	$M_z;\Psi_i = 0,0 \text{ kNm}$	$M_z;s = 0,0 \text{ kNm}$	
$C_{my} = 1,00$	$C_{mz} = 1,00$	$C_{mLT} = 1,00$	
$K_{yy} = 1,000$	$K_{yz} = 0,600$	$K_{zy} = 0,600$	$K_{zz} = 1,000$
$\chi_i;y = 1,00$	$\chi_i;z = 1,00$	$\chi_i;LT = 1,00$	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			

Section data member C15-V1 (0.000-0.490)

N60.3/4	Analysis	Steel S235H(EN10210-1)	$f_{yd}(\text{applied}) = 235 \text{ N/mm}^2$
$h = 60,3 \text{ mm}$	$A = 0,71\text{e-}03 \text{ m}^2$	$W_{y,el} = 934.4\text{e-}08 \text{ m}^3$	$W_{y,pl} = 127.0\text{e-}07 \text{ m}^3$
$b = 60,3 \text{ mm}$	$I_y = 281.7\text{e-}09 \text{ m}^4$	$W_{z,el} = 934.4\text{e-}08 \text{ m}^3$	$W_{z,pl} = 127.0\text{e-}07 \text{ m}^3$
$t_f = 4,0 \text{ mm}$	$I_z = 281.7\text{e-}09 \text{ m}^4$	$A_{w,y,el} = 4.50\text{e-}04 \text{ m}^2$	$A_{w,y,pl} = 4.50\text{e-}04 \text{ m}^2$
$t_w = 4,0 \text{ mm}$	$\text{Mass}/m = 5,6 \text{ kg/m}$	$A_{w,z,el} = 4.50\text{e-}04 \text{ m}^2$	$A_{w,z,pl} = 4.50\text{e-}04 \text{ m}^2$
$r = 26,1 \text{ mm}$		$I_t = 563.5\text{e-}09 \text{ m}^4$	$I_{wa} = 0.0\text{e-}02 \text{ m}^6$

	Pole 1	
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Crosscheck member C15-V1 (0.000-0.490)

Decisive combination: Pe.C.1 at 0,490 m		Section class = 1
N _i Ed = 0,0 kN	V _y Ed = 0,0 kN	M _y Ed = 0,0 kNm
	V _z Ed = 0,0 kN	M _z Ed = 0,0 kNm
N _i Rd = 166,3 kN	V _y Rd = 61,1 kN	M _y Rd = 3,0 kNm
	V _z Rd = 61,1 kN	M _z Rd = 3,0 kNm
NEN-EN1993-1-1(6.5): UC = 0,00 < 1		

Lateral Buckling member C15-V1 (0.000-0.490)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Cantilever	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,490 m	lst = 0,980 m
Lsys = 0,490 m	Lg = 0,490 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,490 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: no bending			

Section data member C16-V1 (0.000-0.400)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	W _y el = 934.4e-08 m3	W _y pl = 127.0e-07 m3
b = 60,3 mm	I _y = 281.7e-09 m4	W _z el = 934.4e-08 m3	W _z pl = 127.0e-07 m3
t _f = 4,0 mm	I _z = 281.7e-09 m4	A _w y _{el} = 4.50e-04 m2	A _w y _{pl} = 4.50e-04 m2
t _w = 4,0 mm	Mass/m = 5,6 kg/m	A _w z _{el} = 4.50e-04 m2	A _w z _{pl} = 4.50e-04 m2
r = 26,1 mm		I _t = 563.5e-09 m4	I _{wa} = 0.0e-02 m6

Crosscheck member C16-V1 (0.000-0.400)

Decisive combination: Pe.C.1 at 0,400 m		Section class = 1	
N _x Ed = -0,2 kN	V _y Ed = 0,0 kN	M _y Ed = 0,1 kNm	
	V _z Ed = 0,2 kN	M _z Ed = 0,0 kNm	
N _c Rd = 166,3 kN	V _y Rd = 61,1 kN	M _y Rd = 3,0 kNm	
	V _z Rd = 61,1 kN	M _z Rd = 3,0 kNm	
NV _y Rd = 166,3 kN	NV _z Rd = 166,3 kN	MV _y Rd = 3,0 kNm	MV _z Rd = 3,0 kNm
α ₁ = 2,000	β ₁ = 1,000	MNV _y Rd = 3,0 kNm	q(y) = 1,030
α ₂ = 2,000	β ₂ = 1,000	MNV _z Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,03 < 1			

Lateral Buckling member C16-V1 (0.000-0.400)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (1)	M = 0,1kN/m	MBeta = 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,400 m	lst = 0,400 m
Lsys = 0,400 m	Lg = 0,400 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,16	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm	UC(y) = 0,00	
Chi;LT,Z = 1,00	lkip = 0,400 m	UC(z) = 0,00	
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

	Pole 1	
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Stability Check C16-V1 (0.000-0.400)

Decisive combination: Pe.C.1

N _i Ed = -0,3 kN	Nb;Rd;y = 165,8 kN	Nb;Rd;z = 165,8 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,400 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,400 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C16-V1 (0.000-0.400)

Decisive combination: Pe.C.1

N _i Ed = -0,3 kN	My;Ed = 0,1 kNm	Section class = 1	
	Delta;My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
		Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,62	Cmz = 0,80	CmLT = 0,62	
Kyy = 0,616	Kyz = 0,479	Kzy = 0,370	Kzz = 0,799
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,02 < 1			

Section data member C18-V1 (0.000-0.815)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C18-V1 (0.000-0.815)

Decisive combination: Pe.C.1 at 0,000 m

N _x Ed = -0,1 kN	Vy;Ed = 0,0 kN	Section class = 1	
	Vz;Ed = 0,0 kN	My;Ed = 0,0 kNm	
		Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NV _y ;Rd = 0,0 kN	NV _z ;Rd = 0,0 kN	MV _y ;Rd = 0,0 kNm	MV _z ;Rd = 0,0 kNm
NEN-EN1993-1-1(6.9): UC = 0,00 < 1			

Lateral Buckling member C18-V1 (0.000-0.815)

Equi. section: N60.3/4

Decisive combination: Pe.C.2

Instab. curve Lat. Buck.:d

Load Attachment Position: 0,000 m from section center

Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Cantilever	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;l _{st} = 0,000 m	Xe;l _{st} = 0,815 m	l _{st} = 1,630 m
L _{sys} = 0,815 m	L _g = 0,815 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M _i Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	I _{kip} = 0,815 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: no bending			

Stability Check C18-V1 (0.000-0.815)

Decisive combination: Pe.C.1

N _i Ed = -0,1 kN	Nb;Rd;y = 156,8 kN	Nb;Rd;z = 156,8 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,815 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,815 m
Chi;y = 0,94		Buckl. curve: A	
Chi;z = 0,94		Buckl. curve: A	

	Pole 1	
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NEN-EN1993-1-1(6.46): $UC = 0,00 < 1$

Bending & Compression C18-V1 (0.000-0.815)

Decisive combination: Pe.C.1		Section class = 1	
N _i Ed = -0,1 kN	My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 1,00	Cmz = 1,00	CmLT = 1,00	
Kyy = 1,000	Kyz = 0,600	Kzy = 0,600	Kzz = 1,000
Chi;y = 0,94	Chi;z = 0,94	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): $UC = 0,00 < 1$			

Section data member C19-V1 (0.000-0.800)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	Iy = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	Iz = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		It = 563.5e-09 m ⁴	Iwa = 0.0e-02 m ⁶

Crosscheck member C19-V1 (0.000-0.800)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
N _i Ed = 0,7 kN	Vy;Ed = 0,0 kN	My;Ed = -0,1 kNm	
	Vz;Ed = 0,3 kN	Mz;Ed = 0,0 kNm	
N _i Rd = 166,3 kN	Vy;Rd = 61,1 kN	MyRd = 3,0 kNm	
	Vz;Rd = 61,1 kN	MzRd = 3,0 kNm	
NEN-EN1993-1-1(NB.33): $UC = 0,05 < 1$			

Lateral Buckling member C19-V1 (0.000-0.800)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.4	M = -0,1kN/m	MBeta = 0,1	q = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,800 m	lst = 0,800 m
Lsys = 0,800 m	Lg = 0,800 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,800 m		UC(z) = 0,00
My;begin = -0,1 kNm	My;end = 0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C20-V1 (0.000-0.300)

N60.3/4	Analysis	Steel S235H(EN10210-1)	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	Iy = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	Iz = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		It = 563.5e-09 m ⁴	Iwa = 0.0e-02 m ⁶

Crosscheck member C20-V1 (0.000-0.300)

Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
N _x Ed = 0,2 kN	Vy;Ed = 0,0 kN	My;Ed = 0,0 kNm	
	Vz;Ed = 0,1 kN	Mz;Ed = 0,0 kNm	
N _c Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NV _y ;Rd = 166,3 kN	NV _z ;Rd = 166,3 kN	MV _y ;Rd = 3,0 kNm	MV _z ;Rd = 3,0 kNm

	Pole 1	
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$\alpha_1 = 2,000$ $\beta_1 = 1,000$ MNV;y;Rd = 3,0 kNm q(y) = 1,030
 $\alpha_2 = 2,000$ $\beta_2 = 1,000$ MNV;z;Rd = 3,0kNm q(z) = 1,030
 NEN-EN1993-1-1(NB.33): UC = 0,01 < 1

Lateral Buckling member C20-V1 (0.000-0.300)

Equi. section: N60.3/4
 Decisive combination: Pe.C.2 Instab. curve Lat. Buck.:d
 Load Attachment Position: 0,000 m from section center
 Lateral buckling supports upper flange: None
 Lateral buckling supports lower flange: None
 Restr. begin: Supported Restr. end: Supported b-eff(Begin) = 0,000 b-eff(End) = 0,000
 Table used: NB.NB.3 (1) = 0,0kN/m = 0,0
 Upper flange decisive Xb;lst = 0,000 m Xe;lst = 0,300 m lst = 0,300 m
 Lsys = 0,300 m Lg = 0,300 m S = 0,000 m Iwa = 0.0000e+00 m6
 C1 = 2,30 C2 = 1,55 (table) C2(applied) = 0,00 C = 0,00
 Mcr = 0,0 kNm kred = 1.0 Lam-rel = 0,00 Section class 1
 Chi;LT(Pe.C.2) = 1,00 M;Ed = 0,0 kNm UC(y) = 0,00
 Chi;LT,Z = 1,00 lkip = 0,300 m UC(z) = 0,00
 My;begin = 0,0 kNm My;end = 0,0 kNm
 NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)

Section data member C21-V1 (0.000-0.100)

N60.3/4	Analysis	Steel S235H(EN10210-1)	fyd(applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C21-V1 (0.000-0.100)

Decisive combination: Pe.C.2 at 0,100 m Section class = 1
 Nx;Ed = 0,0 kN Vy;Ed = 0,0 kN My;Ed = 0,0 kNm
 Vz;Ed = 0,0 kN Mz;Ed = 0,0 kNm
 Nc;Rd = 166,3 kN Vy;Rd = 61,1 kN My;Rd = 3,0 kNm
 Vz;Rd = 61,1 kN Mz;Rd = 3,0 kNm
 NVy;Rd = 166,3 kN NVz;Rd = 166,3 kN MVy;Rd = 3,0 kNm MVz;Rd = 3,0 kNm
 NEN-EN1993-1-1(6.12): UC = 0,00 < 1

Lateral Buckling member C21-V1 (0.000-0.100)

Equi. section: N60.3/4
 Decisive combination: Pe.C.2 Instab. curve Lat. Buck.:d
 Load Attachment Position: 0,000 m from section center
 Lateral buckling supports upper flange: None
 Lateral buckling supports lower flange: None
 Restr. begin: Cantilever Restr. end: Supported b-eff(Begin) = 0,000 b-eff(End) = 0,000
 Table used: NB.NB.1 (6) q = 0,1kN/m = 0,0
 Upper flange decisive Xb;lst = 0,000 m Xe;lst = 0,100 m lst = 0,082 m
 Lsys = 0,100 m Lg = 0,100 m S = 0,000 m Iwa = 0.0000e+00 m6
 C1 = 1,68 C2 = 0,78 (table) C2(applied) = 0,00 C = 0,00
 Mcr = 0,0 kNm kred = 1.0 Lam-rel = 0,00 Section class 1
 Chi;LT(Pe.C.2) = 1,00 M;Ed = 0,0 kNm UC(y) = 0,00
 Chi;LT,Z = 1,00 lkip = 0,100 m UC(z) = 0,00
 My;begin = 0,0 kNm My;end = 0,0 kNm
 NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: Flange is under tension

Section data member C22-V1 (0.000-0.400)

N60.3/4	Analysis	Steel S235H(EN10210-1)	fyd(applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

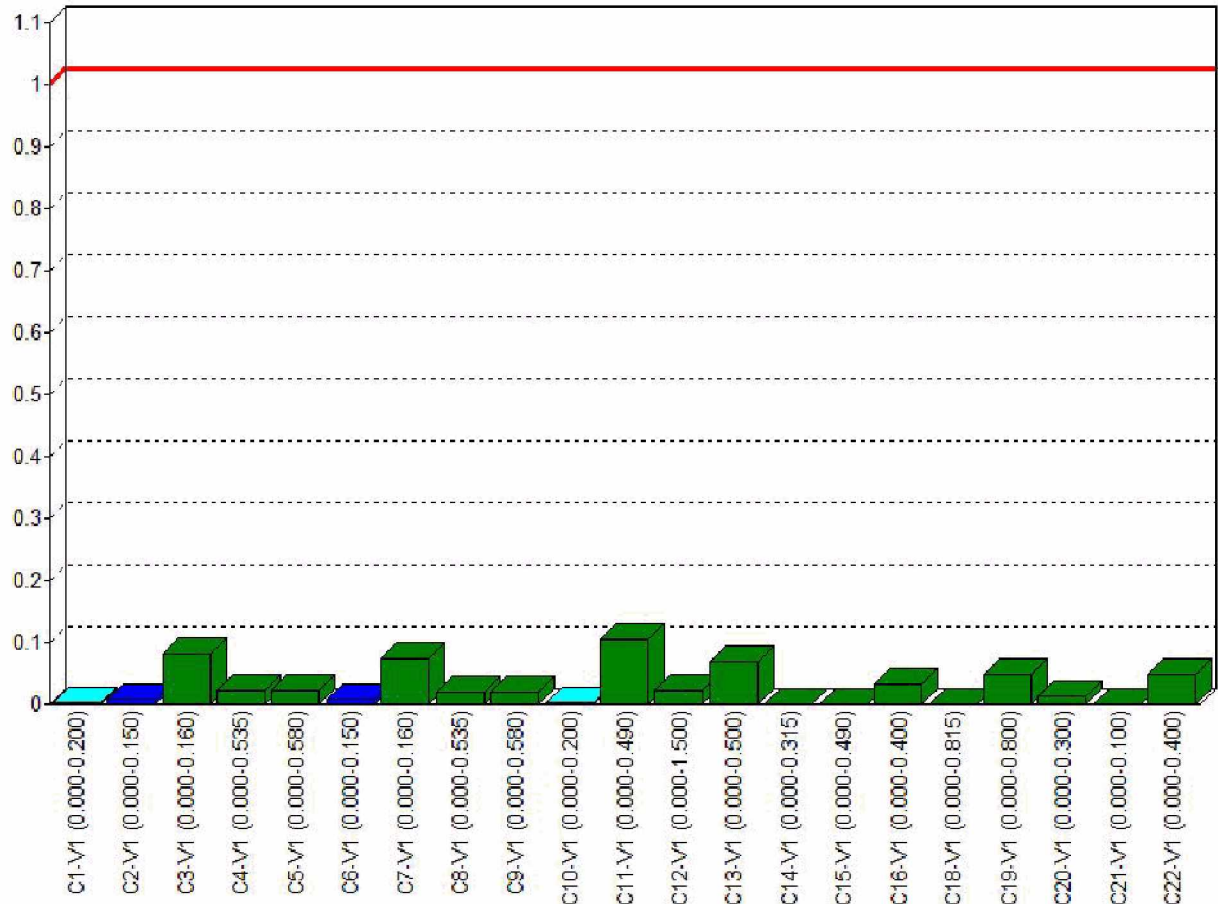
Crosscheck member C22-V1 (0.000-0.400)

Decisive combination: Pe.C.1 at 0,400 m		Section class = 1	
Nx;Ed = 0,1 kN	Vy;Ed = 0,0 kN	My;Ed = -0,1 kNm	
	Vz;Ed = -0,7 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV;z;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,05 < 1			

Lateral Buckling member C22-V1 (0.000-0.400)

Equi. section: N60.3/4			
Decisive combination: Pe.C.2		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.4	M = -0,1kN/m	MBeta = 0,1	q = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,400 m	lst = 0,400 m
Lsys = 0,400 m	Lg = 0,400 m	S = 0,000 m	lwa = 0.0000e+00 m6
C1 = 2,30	C2 = 0,01 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.2) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,400 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = -0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

PIC. STEEL UC CHART



UNITY CHECK NEN-EN1993-1-1:2016/NB:2016

Field	Check Type	Combination	Formula	UC max
C1-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C1-V1 (0.000-0.200)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C2-V1 (0.000-0.150)	Cross	Pe.C.1	NEN-EN1993-1-1(6.31)	0,01
C2-V1 (0.000-0.150)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C2-V1 (0.000-0.150)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C2-V1 (0.000-0.150)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,01
C2-V1 (0.000-0.150)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C3-V1 (0.000-0.160)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,08
C3-V1 (0.000-0.160)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C3-V1 (0.000-0.160)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C3-V1 (0.000-0.160)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,03
C3-V1 (0.000-0.160)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C4-V1 (0.000-0.535)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C4-V1 (0.000-0.535)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C4-V1 (0.000-0.535)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C4-V1 (0.000-0.535)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,01
C4-V1 (0.000-0.535)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C5-V1 (0.000-0.580)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C5-V1 (0.000-0.580)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.580)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.580)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,01
C5-V1 (0.000-0.580)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C6-V1 (0.000-0.150)	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C6-V1 (0.000-0.150)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C7-V1 (0.000-0.160)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,08
C7-V1 (0.000-0.160)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C8-V1 (0.000-0.535)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C8-V1 (0.000-0.535)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C9-V1 (0.000-0.580)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C9-V1 (0.000-0.580)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C10-V1 (0.000-0.200)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C11-V1 (0.000-0.490)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C11-V1 (0.000-0.490)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C12-V1 (0.000-1.500)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,02
C12-V1 (0.000-1.500)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C13-V1 (0.000-0.500)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,07
C13-V1 (0.000-0.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,04
C13-V1 (0.000-0.500)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C14-V1 (0.000-0.315)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C14-V1 (0.000-0.315)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.315)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.315)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00

	Pole 1	
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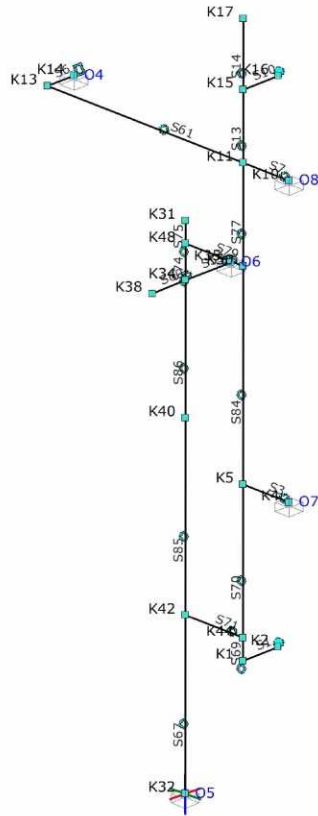
Field	Check Type	Combination	Formula	UC max
C14-V1 (0.000-0.315)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C15-V1 (0.000-0.490)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C15-V1 (0.000-0.490)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C16-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C16-V1 (0.000-0.400)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C16-V1 (0.000-0.400)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C16-V1 (0.000-0.400)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,02
C16-V1 (0.000-0.400)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C18-V1 (0.000-0.815)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C18-V1 (0.000-0.815)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C18-V1 (0.000-0.815)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C18-V1 (0.000-0.815)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C18-V1 (0.000-0.815)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C19-V1 (0.000-0.800)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,05
C19-V1 (0.000-0.800)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C20-V1 (0.000-0.300)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,01
C20-V1 (0.000-0.300)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C21-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(6.12)	0,00
C21-V1 (0.000-0.100)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00
C22-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,05
C22-V1 (0.000-0.400)	Lat. Buck.	Pe.C.2	NEN-EN1993-1-1(6.54)	0,00

STEEL CONSTRUCTION WEIGHT

Member	Section	Lsys	Mass
C10-V1 (0.000-0.200)	C100	0,200	12,331
C1-V1 (0.000-0.200)	C100	0,200	12,331
SubTotal:	C100	0,400	24,662
C2-V1 (0.000-0.150)	K60x60x4x4	0,150	1,055
C6-V1 (0.000-0.150)	K60x60x4x4	0,150	1,055
SubTotal:	K60x60x4x4	0,300	2,110
C11-V1 (0.000-0.490)	N60.3/4	0,490	2,721
C12-V1 (0.000-1.500)	N60.3/4	1,500	8,331
C13-V1 (0.000-0.500)	N60.3/4	0,500	2,777
C14-V1 (0.000-0.315)	N60.3/4	0,315	1,749
C15-V1 (0.000-0.490)	N60.3/4	0,490	2,721
C16-V1 (0.000-0.400)	N60.3/4	0,400	2,222
C18-V1 (0.000-0.815)	N60.3/4	0,815	4,526
C19-V1 (0.000-0.800)	N60.3/4	0,800	4,443
C20-V1 (0.000-0.300)	N60.3/4	0,300	1,666
C21-V1 (0.000-0.100)	N60.3/4	0,100	0,555
C22-V1 (0.000-0.400)	N60.3/4	0,400	2,222
C3-V1 (0.000-0.160)	N60.3/4	0,160	0,889
C4-V1 (0.000-0.535)	N60.3/4	0,535	2,971
C5-V1 (0.000-0.580)	N60.3/4	0,580	3,221
C7-V1 (0.000-0.160)	N60.3/4	0,160	0,889
C8-V1 (0.000-0.535)	N60.3/4	0,535	2,971
C9-V1 (0.000-0.580)	N60.3/4	0,580	3,221
SubTotal:	N60.3/4	8,660	48,096
Total:		9,360 m	74,867 kg

Pole 2			
Project name	L025673	Project number	
Part description	Pole 2	Structural engineer	5.1.2.e
Client		Units	m, kN, kNm

PIC. GEOMETRIE RAAMWERK



MEMBERS

Member	Node B	Node E	X-B	Y-B	Z-B	X-E	Y-E	Z-E	Length	Section	Pos
S1	K2	K1	0,200	0,330	-0,680	0,000	0,330	-0,680	0,200	P3	0,000 - L(0,
S3	K4	K5	0,000	0,590	-1,450	0,000	0,330	-1,450	0,260	P1	0,000 - L(0,
S6	K14	K13	0,150	-0,785	-2,850	0,000	-0,785	-2,850	0,150	P2	0,000 - L(0,
S7	K10	K11	0,000	0,590	-2,850	0,000	0,330	-2,850	0,260	P1	0,000 - L(0,
S10	K16	K15	0,200	0,330	-3,170	0,000	0,330	-3,170	0,200	P3	0,000 - L(0,
S13	K11	K15	0,000	0,330	-2,850	0,000	0,330	-3,170	0,320	P1	0,000 - L(0,
S14	K15	K17	0,000	0,330	-3,170	0,000	0,330	-3,480	0,310	P1	0,000 - L(0,
S53	K35	K34	0,260	0,000	-2,240	0,000	0,000	-2,240	0,260	P1	0,000 - L(0,
S60	K34	K38	0,000	0,000	-2,240	-0,190	0,000	-2,240	0,190	P1	0,000 - L(0,
S61	K11	K13	0,000	0,330	-2,850	0,000	-0,785	-2,850	1,115	P1	0,000 - L(1,
S67	K32	K42	0,000	0,000	0,000	0,000	0,000	-0,780	0,780	P1	0,000 - L(0,
S69	K1	K44	0,000	0,330	-0,680	0,000	0,330	-0,780	0,100	P1	0,000 - L(0,
S70	K44	K5	0,000	0,330	-0,780	0,000	0,330	-1,450	0,670	P1	0,000 - L(0,
S71	K44	K42	0,000	0,330	-0,780	0,000	0,000	-0,780	0,330	P4	0,000 - L(0,
S74	K34	K48	0,000	0,000	-2,240	0,000	0,000	-2,400	0,160	P1	0,000 - L(0,
S75	K48	K31	0,000	0,000	-2,400	0,000	0,000	-2,500	0,100	P1	0,000 - L(0,
S77	K50	K11	0,000	0,330	-2,400	0,000	0,330	-2,850	0,450	P1	0,000 - L(0,
S79	K50	K48	0,000	0,330	-2,400	0,000	0,000	-2,400	0,330	P4	0,000 - L(0,
S84	K5	K50	0,000	0,330	-1,450	0,000	0,330	-2,400	0,950	P1	0,000 - L(0,
S85	K42	K40	0,000	0,000	-0,780	0,000	0,000	-1,640	0,860	P1	0,000 - L(0,
S86	K40	K34	0,000	0,000	-1,640	0,000	0,000	-2,240	0,600	P1	0,000 - L(0,
-	-	-	m	m	m	m	m	m	m	-	

SECTIONS

Section	Section Name	Area	It	Iy	Iz	Material	Angle
P1	N60.3/4	7.0749e-04	5.6346e-07	2.8173e-07	2.8173e-07	S235	0,0

	Pole 2	
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Section	Section Name	Area	It	Iy	Iz Material	Angle
P2	K60x60x4x4	8.9600e-04	7.0246e-07	4.7070e-07	4.7070e-07 S235	0,0
P3	C100	7.8540e-03	9.8175e-06	4.9087e-06	4.9087e-06 S235	0,0
P4	B48.3x3.2	4.5339e-04	2.3171e-07	1.1586e-07	1.1586e-07 S235	0,0
-	-	m2	m4	m4	m4 -	°

SECTION SHAPES

Section	Tapered	hB	hE	tf	tw	tf2	B	b1	b2 Castellate	Height
P2	No	0,060	0,060	0,0040	0,0040	0,0000	0,060	0,000	0,000 No	0,000
P3	No	0,100	0,100	0,0000	0,0000	0,0000	0,100	0,000	0,000 No	0,000
P4	No	0,048	0,048	0,0032	0,0032	0,0000	0,048	0,000	0,000 No	0,000
-	-	m	m	m	m	m	m	m	m -	m

MATERIALS

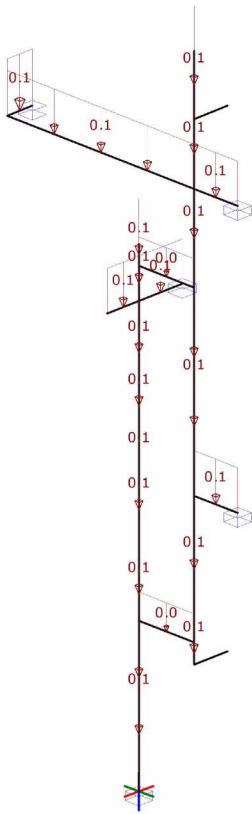
Material Name	Poison	Density	Youngs mod.	Exp.
S235	0.30	78.50	2.1000e+08	12.0000e-06
-	-	kN/m3	kN/m2	C°m

SUPPORTS

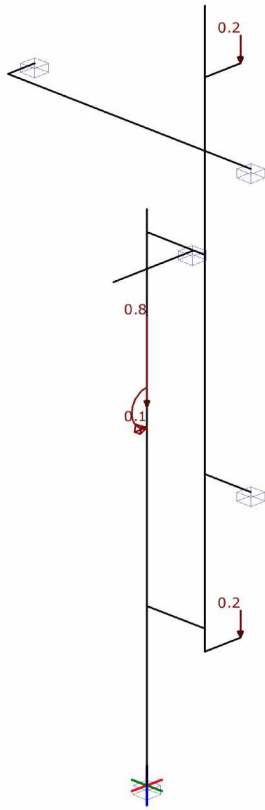
Support	Object	Position	X	Y	Z	Xr	Yr	Zr	AngleXr	AngleYr	AngleZr
O4	K14	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O5	K32	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O6	K35	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O7	K4	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O8	K10	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
-	-	m	kN/m	kN/m	kN/m	kNm/rad	kNm/rad	kNm/rad	°	°	°

LOADS CASES TYPES

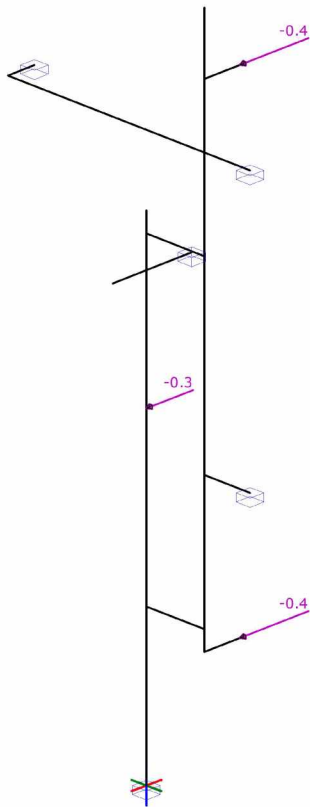
Label	Description	L.C. Type	Fav./Unfav.	Element	Level	Field	Psi0	Psi1	Psi2	Cprob ULS/SLS
B.G.1	Permanent actions	Permanent actions	-		None	None				
B.G.2	Permanent actions	Permanent actions	-		None	None				
B.G.3	wind x	Wind load	-		None	None	0.20			1,00/1,00



PIC. LASTEN B.G.2 PERMANENT ACTIONS



PIC. LASTEN B.G.3 WIND X



PERSISTENT LOADS COMBINATIONS (TABLE)

L.C.	Description	Pe.C.1	Pe.C.2	Pe.C.3
B.G.1	Permanent actions	1.35	1.20	0.90
B.G.2	Permanent actions	1.35	1.20	0.90
B.G.3	wind x	0.00	1.27	-1.27

CHARACTERISTIC LOADS COMBINATIONS (TABLE)

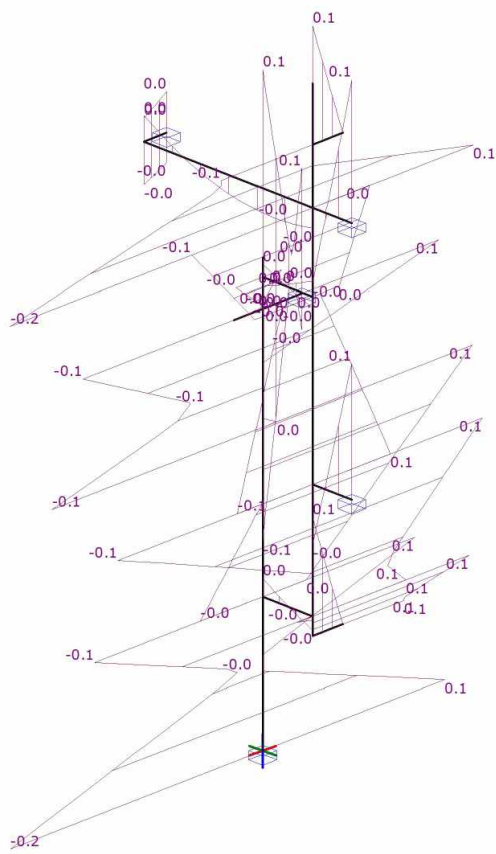
L.C.	Description	Ch.C.(w1)	Ch.C.1
B.G.1	Permanent actions	1.00	1.00
B.G.2	Permanent actions	1.00	1.00
B.G.3	wind x	0.00	0.57

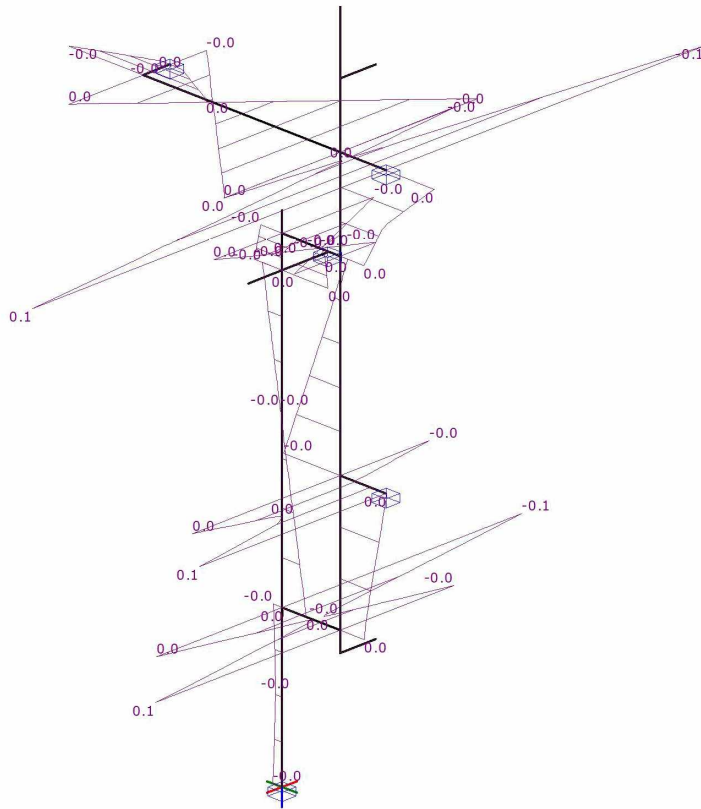
ANALYSIS ASSUMPTIONS

Linear Elastic Analysis performed

PIC. PE.C. MOMENT (MY) ENVELOPE

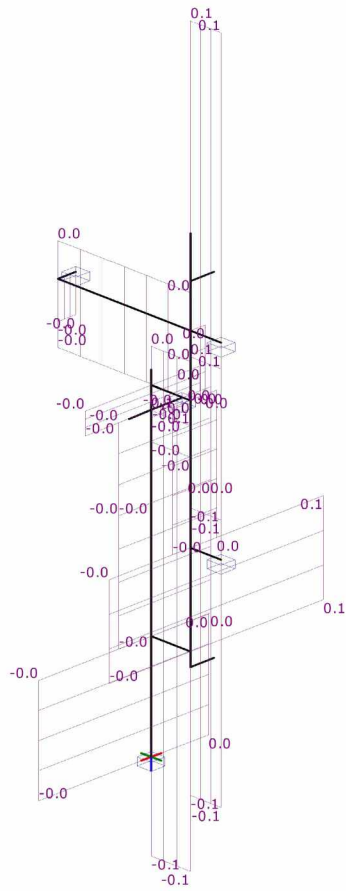
Persistent Loads Combinations

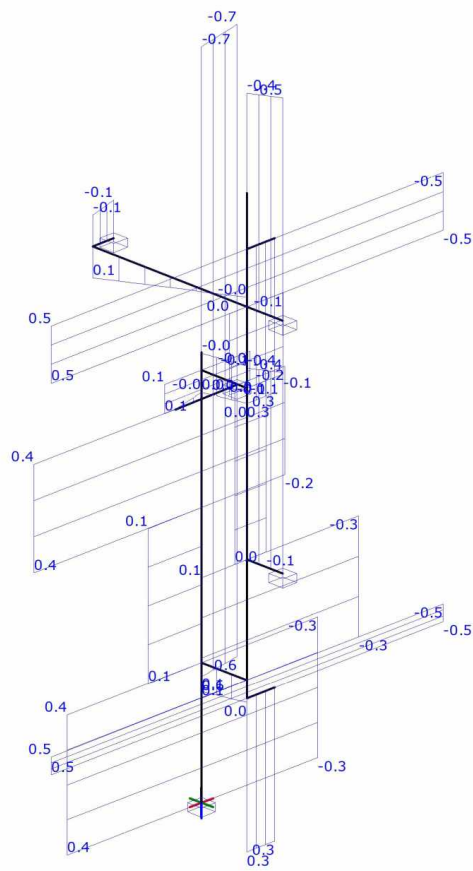


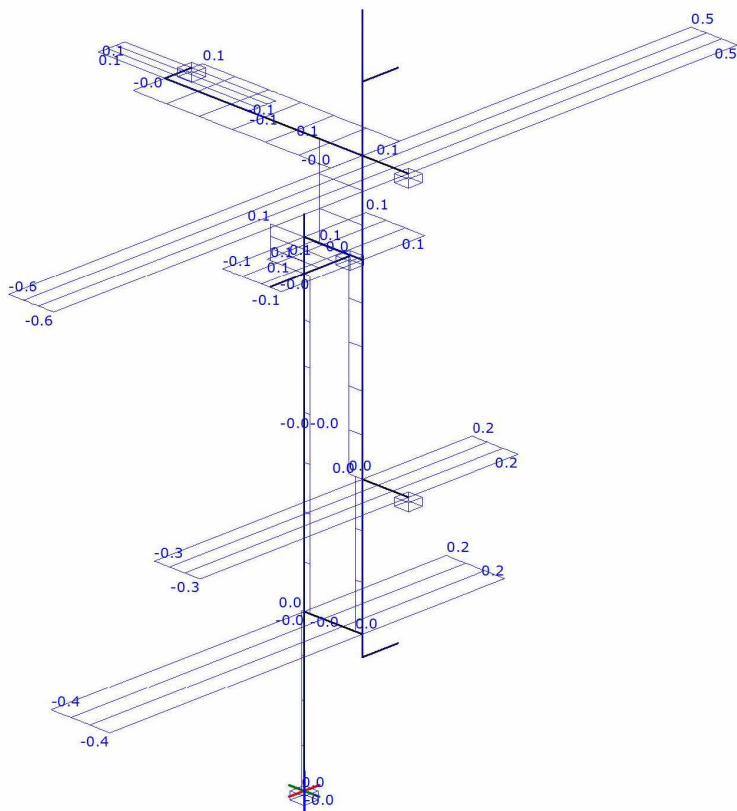


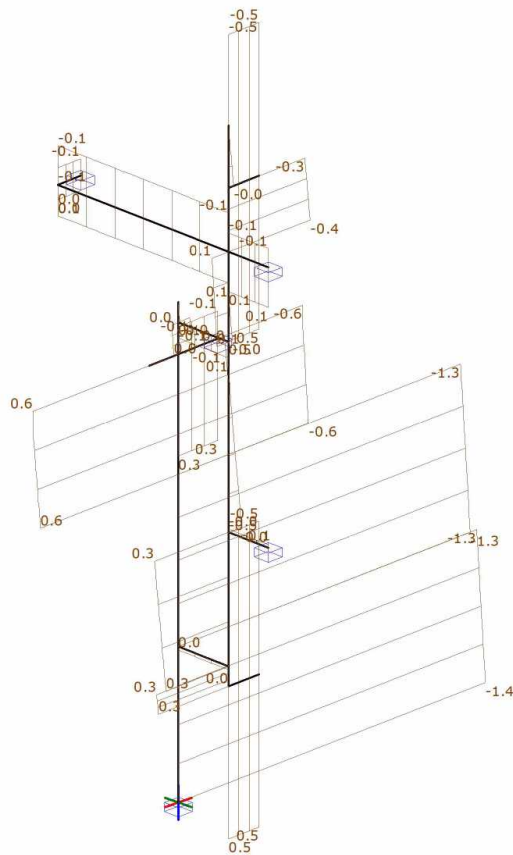
PIC. PE.C. MOMENT (MX) ENVELOPE Persistent Loads Combinations

PIC. PE.C. MOMENT (MX) ENVELOPE Persistent Loads Combinations









PE.C. MEMBER FORCES (MY, MZ)

Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S1	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.04	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
S3	Pe.C.1	My	0.07			-0.03	0.179	0.000
		Mz	0.04			0.01	0.000	0.000
	Pe.C.2	My	0.06			-0.03	0.179	0.000
		Mz	0.00			0.05	0.000	0.000
	Pe.C.3	My	0.05			-0.02	0.179	0.000
		Mz	0.06			-0.03	0.176	0.000
S6	Pe.C.1	My	0.00			-0.01	0.055	0.000
		Mz	0.00			0.00	0.024	0.000
	Pe.C.2	My	0.02			0.01	0.000	0.000
		Mz	0.00			0.02	0.000	0.000
	Pe.C.3	My	-0.02			-0.02	0.000	0.000
		Mz	0.00			-0.02	0.000	0.000
S7	Pe.C.1	My	0.08			-0.04	0.169	0.000
		Mz	0.01			0.00	0.000	0.000
	Pe.C.2	My	0.06			-0.03	0.175	0.000
		Mz	-0.10			0.04	0.190	0.000
	Pe.C.3	My	0.05			-0.03	0.163	0.000
		Mz	0.11			-0.04	0.196	0.000
S10	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000

			Pole 2					
Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S77	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	-0.01			-0.03	0.000	0.000
		Mz	0.00			0.03	0.000	0.000
	Pe.C.2	My	0.02			0.04	0.000	0.000
S79	Pe.C.3	Mz	0.01			0.02	0.000	0.000
		My	-0.04			-0.08	0.000	0.000
		Mz	-0.01			0.02	0.092	0.000
	Pe.C.1	My	0.00	0.00	0.188	0.00	0.000	0.000
		Mz	0.00			0.00	0.096	0.000
		My	-0.01			0.01	0.169	0.000
	Pe.C.2	Mz	-0.01			0.02	0.114	0.000
		My	0.01			0.00	0.198	0.000
		Mz	0.01			-0.03	0.110	0.000
	Pe.C.3	My	0.00			-0.01	0.104	0.000
S84	Pe.C.1	Mz	-0.02			0.00	0.840	0.000
		My	0.05			0.01	0.000	0.000
		Mz	-0.02			0.00	0.863	0.000
	Pe.C.2	My	-0.05			-0.02	0.000	0.000
		Mz	-0.01			0.00	0.812	0.000
		My	-0.01			0.11	0.058	0.000
	Pe.C.3	Mz	0.01			0.00	0.805	0.000
		My	-0.04			0.03	0.482	0.000
		Mz	0.01			0.00	0.782	0.000
	Pe.C.1	My	0.03			0.14	0.000	0.000
S85	Pe.C.2	Mz	0.00			0.00	0.842	0.000
		My	-0.09			0.00	0.000	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.3	My	-0.15			0.12	0.327	0.000
		Mz	0.00			-0.01	0.000	0.000
		My	0.01			-0.12	0.044	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	-0.09			0.00	0.000	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.2	My	-0.15			0.12	0.327	0.000
		Mz	0.00			-0.01	0.000	0.000
S86	Pe.C.3	My	0.01			-0.12	0.044	0.000
		Mz	0.00			0.00	0.000	0.000
		My	-0.09			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			-0.01	0.000	0.000
		My	-0.15			0.12	0.327	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.2	My	0.01			-0.12	0.044	0.000
		Mz	0.00			0.00	0.000	0.000
		My	-0.09			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			-0.01	0.000	0.000
		My	-0.15			0.12	0.327	0.000
		Mz	0.00			-0.01	0.000	0.000
-	-	-	kNm	kNm	m	kNm	m	m

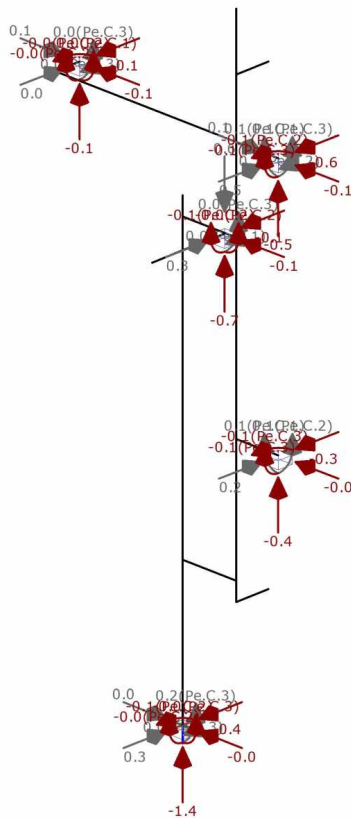
PE.C. MEMBER FORCES (NX, VY, VZ, MX)

Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S1	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.52 Vz	0.28	0.28	0.28	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.52 Vz	0.21	0.21	0.21	0.00	0.00
			Vy	0.00	0.00	0.00		
S3	Pe.C.1	C	-0.01 Vz	-0.41	-0.41	-0.39	-0.01	-0.01
			Vy	-0.10	-0.10	-0.10		
	Pe.C.2	C	-0.01 Vz	-0.35	-0.35	-0.34	0.06	0.06
			Vy	0.17	0.17	0.17		
	Pe.C.3	C	0.00 Vz	-0.29	-0.29	-0.27	-0.08	-0.08
			Vy	-0.33	-0.33	-0.33		
S6	Pe.C.1	C	-0.01 Vz	-0.08	-0.08	-0.06	-0.01	-0.01
			Vy	-0.02	-0.02	-0.02		
	Pe.C.2	T	0.05 Vz	-0.07	-0.07	-0.06	-0.01	-0.01
			Vy	0.11	0.11	0.11		
	Pe.C.3	C	-0.06 Vz	-0.05	-0.05	-0.04	-0.01	-0.01
			Vy	-0.13	-0.13	-0.13		
S7	Pe.C.1	T	0.05 Vz	-0.46	-0.46	-0.44	0.03	0.03
			Vy	-0.03	-0.03	-0.03		
	Pe.C.2	T	0.14 Vz	-0.37	-0.37	-0.35	-0.06	-0.06
			Vy	0.52	0.52	0.52		
	Pe.C.3	C	-0.06 Vz	-0.34	-0.34	-0.33	0.10	0.10
			Vy	-0.56	-0.56	-0.56		
S10	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		

	Pole 2	
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Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S10	Pe.C.2	C	-0.52 Vz	0.28	0.28	0.28	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.52 Vz	0.21	0.21	0.21	0.00	0.00
S13	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.36 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.32 Vz	-0.52	-0.52	-0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
S14	Pe.C.1	C	-0.24 Vz	0.52	0.52	0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
S53	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.16 Vz	-0.12	-0.12	-0.10	0.00	0.00
	Pe.C.2	T	Vy	0.05	0.05	0.05		
			0.35 Vz	-0.74	-0.74	-0.72	0.00	0.00
	Pe.C.3	C	Vy	0.02	0.02	0.02		
S60	Pe.C.1	-	-0.09 Vz	0.56	0.57	0.57	0.00	0.00
			Vy	0.06	0.06	0.06		
	Pe.C.2	-	0.00 Vz	-0.01	-0.01	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	-	0.00 Vz	-0.01	-0.01	0.00	0.00	0.00
S61	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.02 Vz	-0.02	0.06	0.06	-0.01	-0.01
	Pe.C.2	T	Vy	0.01	0.01	0.01		
			0.11 Vz	-0.02	0.06	0.06	0.01	0.01
	Pe.C.3	C	Vy	-0.05	-0.05	-0.05		
S67	Pe.C.1	C	-0.13 Vz	-0.01	0.04	0.04	-0.02	-0.02
			Vy	0.06	0.06	0.06		
	Pe.C.2	C	-1.17 Vz	0.03	0.03	0.03	-0.02	-0.02
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.46 Vz	-0.31	-0.31	-0.31	-0.05	-0.05
S69	Pe.C.1	T	Vy	0.00	0.00	0.00		
			-1.37 Vz	0.36	0.36	0.36	0.02	0.02
	Pe.C.2	T	Vy	0.00	0.00	0.00		
			0.32 Vz	0.00	0.00	0.00	0.00	0.00
	S70	Pe.C.1	T	Vy	0.00	0.00	0.00	
0.29 Vz				0.52	0.52	0.52	0.00	0.00
Pe.C.2		T	Vy	0.00	0.00	0.00		
			0.22 Vz	-0.52	-0.52	-0.52	0.00	0.00
S71		Pe.C.1	T	Vy	0.00	0.00	0.00	
	0.33 Vz			-0.11	-0.11	-0.11	0.01	0.01
	Pe.C.2	T	Vy	0.01	0.01	0.01		
			0.29 Vz	0.12	0.12	0.12	0.06	0.06
	S74	Pe.C.1	C	Vy	0.01	0.01	0.01	
0.23 Vz				-0.30	-0.30	-0.30	-0.04	-0.04
Pe.C.2		T	Vy	0.01	0.01	0.01		
			0.01 Vz	0.04	0.06	0.06	0.00	0.00
S75		Pe.C.1	C	Vy	-0.11	-0.11	-0.11	
	0.01 Vz			0.05	0.06	0.06	-0.07	-0.07
	Pe.C.2	T	Vy	-0.40	-0.40	-0.40		
			0.01 Vz	0.02	0.03	0.03	0.08	0.08
	S77	Pe.C.1	C	Vy	0.23	0.23	0.23	
-0.03 Vz				-0.02	-0.02	-0.02	0.00	0.00
Pe.C.2		C	Vy	0.04	0.04	0.04		
			-0.06 Vz	0.10	0.10	0.10	0.02	0.02
S78		Pe.C.1	C	Vy	0.01	0.01	0.01	
	0.03 Vz			-0.13	-0.13	-0.13	-0.03	-0.03
	Pe.C.2	C	Vy	0.05	0.05	0.05		
			-0.01 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.3	T	Vy	0.00	0.00	0.00		
-0.01 Vz			0.00	0.00	0.00	0.00	0.00	

			Pole 2					
Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S77	Pe.C.3	C	Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.06 Vz	-0.03	-0.03	-0.03	0.00	0.00
	Pe.C.2	C	Vy	0.06	0.06	0.06		
			-0.02 Vz	0.05	0.05	0.05	0.00	0.00
S79	Pe.C.3	T	Vy	0.03	0.03	0.03		
			0.07 Vz	-0.10	-0.10	-0.10	0.01	0.01
	Pe.C.1	T	Vy	0.07	0.07	0.07		
			0.04 Vz	-0.01	-0.01	0.01	0.00	0.00
	Pe.C.2	T	Vy	-0.02	-0.02	-0.02		
			0.01 Vz	0.03	0.05	0.05	0.01	0.01
S84	Pe.C.3	T	Vy	0.10	0.10	0.10		
			0.05 Vz	-0.05	-0.05	-0.03	-0.02	-0.02
	Pe.C.1	C	Vy	-0.13	-0.13	-0.13		
			-0.06 Vz	-0.01	-0.01	-0.01	0.00	0.00
	Pe.C.2	C	Vy	0.02	0.02	0.02		
			-0.05 Vz	-0.05	-0.05	-0.05	0.01	0.01
S85	Pe.C.3	C	Vy	0.02	0.02	0.02		
			-0.04 Vz	0.03	0.03	0.03	-0.01	-0.01
	Pe.C.1	C	Vy	0.01	0.01	0.01		
			-1.06 Vz	0.14	0.14	0.14	0.01	0.01
	Pe.C.2	C	Vy	-0.01	-0.01	-0.01		
			-0.34 Vz	0.09	0.09	0.09	0.03	0.03
S86	Pe.C.3	C	Vy	-0.01	-0.01	-0.01		
			-1.30 Vz	0.13	0.13	0.13	-0.01	-0.01
	Pe.C.1	T	Vy	-0.01	-0.01	-0.01		
			0.06 Vz	0.14	0.14	0.14	0.01	0.01
	Pe.C.2	T	Vy	-0.01	-0.01	-0.01		
			0.65 Vz	0.45	0.45	0.45	0.03	0.03
	Pe.C.3	C	Vy	-0.01	-0.01	-0.01		
			-0.58 Vz	-0.22	-0.22	-0.22	-0.01	-0.01
-	-	-	Vy	-0.01	-0.01	-0.01		
			kN -	kN	kN	kN	kNm	kNm



PE.C. SUPPORT REACTIONS

LComb	Support	Node	X	Y	Z	Mx	My	Mz
Pe.C.1	O4	K14	-0.01	0.02	-0.08	-0.01	0.00	0.00
Pe.C.1	O5	K32	-0.03	0.00	-1.17	0.00	0.03	-0.02
Pe.C.1	O6	K35	0.16	-0.05	-0.12	0.00	-0.03	0.00
Pe.C.1	O7	K4	-0.10	-0.01	-0.41	0.07	-0.01	-0.04
Pe.C.1	O8	K10	-0.03	0.05	-0.46	0.08	0.03	-0.01
Sum Reactions			0.00	0.00	-2.24			
Sum Loads			0.00	0.00	2.24			
Pe.C.2	O4	K14	0.05	-0.11	-0.07	-0.01	-0.02	0.00
Pe.C.2	O5	K32	0.31	0.00	-0.46	0.00	-0.13	-0.05
Pe.C.2	O6	K35	0.35	-0.02	-0.74	0.00	-0.07	0.00
Pe.C.2	O7	K4	0.17	-0.01	-0.35	0.06	0.06	0.00
Pe.C.2	O8	K10	0.52	0.14	-0.37	0.06	-0.06	0.10
Sum Reactions			1.40	0.00	-1.99			
Sum Loads			-1.40	0.00	1.99			
Pe.C.3	O4	K14	-0.06	0.13	-0.05	-0.01	0.02	0.00
Pe.C.3	O5	K32	-0.36	0.00	-1.37	0.00	0.17	0.02
Pe.C.3	O6	K35	-0.09	-0.06	0.56	0.00	0.02	0.00
Pe.C.3	O7	K4	-0.33	0.00	-0.29	0.05	-0.08	-0.06
Pe.C.3	O8	K10	-0.56	-0.06	-0.34	0.05	0.10	-0.11
Sum Reactions			-1.40	0.00	-1.49			
Sum Loads			1.40	0.00	1.49			
-	-	-	kN	kN	kN	kNm	kNm	kNm

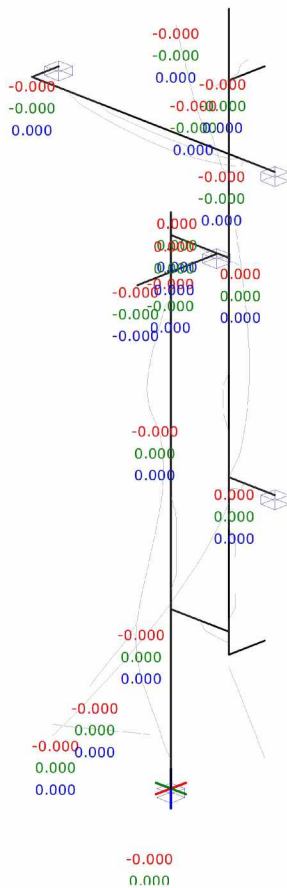
L.C. SUPPORT REACTIONS

L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O4	K14	0.00	0.00	-0.05	-0.01	-0.01	0.00
B.G.1	O5	K32	0.00	0.00	-0.16	0.00	0.00	0.00
B.G.1	O6	K35	0.00	-0.01	-0.02	0.00	0.00	0.00

Pole 2								
L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O7	K4	0.00	0.00	-0.11	0.02	0.00	0.00
B.G.1	O8	K10	0.00	0.01	-0.10	0.02	0.00	0.00
	Sum Reactions		0.00	0.00	-0.43			
	Sum Loads		0.00	0.00	0.43			
B.G.2	O4	K14	0.00	0.01	-0.01	0.00	0.00	0.00
B.G.2	O5	K32	-0.02	0.00	-0.71	0.00	0.02	-0.01
B.G.2	O6	K35	0.12	-0.03	-0.06	0.00	-0.02	0.00
B.G.2	O7	K4	-0.07	0.00	-0.20	0.04	-0.01	-0.03
B.G.2	O8	K10	-0.02	0.02	-0.24	0.04	0.02	-0.01
	Sum Reactions		0.00	0.00	-1.22			
	Sum Loads		0.00	0.00	1.22			
B.G.3	O4	K14	0.04	-0.10	0.00	0.00	-0.01	0.00
B.G.3	O5	K32	0.27	0.00	0.46	0.00	-0.12	-0.03
B.G.3	O6	K35	0.16	0.02	-0.50	0.00	-0.03	0.00
B.G.3	O7	K4	0.21	0.00	0.01	0.00	0.06	0.03
B.G.3	O8	K10	0.43	0.07	0.03	0.00	-0.07	0.08
	Sum Reactions		1.10	0.00	0.00			
	Sum Loads		-1.10	0.00	0.00			
-	-	-	kN	kN	kN	kNm	kNm	kNm

PIC. CH.C. VERPLAATSINGEN ENVELOPE

Characteristic Loads Combinations



CH.C. NODAL DISPLACEMENTS

Node	LComb	X	Y	Z	Xr	Yr	Zr
K1	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.027e-03	-0.252e-03	-0.237e-03
	Ch.C.1	-0.0003	0.0000	0.0000	-0.029e-03	-0.728e-03	-0.535e-03
K2	Ch.C.(w1)	0.0000	0.0001	0.0001	-0.027e-03	-0.256e-03	-0.237e-03
	Ch.C.1	-0.0003	0.0001	0.0002	-0.029e-03	-0.732e-03	-0.535e-03
K4	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03

		Pole 2					
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Node	LComb	X	Y	Z	Xr	Yr	Zr
K5	Ch.C.(w1)	0.0000	0.0000	0.0000	0.064e-03	0.052e-03	-0.081e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.063e-03	-0.138e-03	-0.084e-03
K10	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K11	Ch.C.(w1)	0.0000	0.0000	0.0000	0.058e-03	-0.120e-03	-0.012e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.059e-03	0.095e-03	0.056e-03
K13	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.028e-03	-0.002e-03	0.001e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.027e-03	0.011e-03	-0.007e-03
K14	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K15	Ch.C.(w1)	0.0001	0.0000	0.0000	0.058e-03	-0.373e-03	-0.012e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.059e-03	0.044e-03	0.056e-03
K16	Ch.C.(w1)	0.0001	0.0000	0.0001	0.058e-03	-0.378e-03	-0.012e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.059e-03	0.039e-03	0.056e-03
K17	Ch.C.(w1)	0.0002	0.0000	0.0000	0.058e-03	-0.373e-03	-0.012e-03
	Ch.C.1	-0.0001	0.0000	0.0000	0.059e-03	0.044e-03	0.056e-03
K31	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.017e-03	0.047e-03	-0.030e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.024e-03	-0.029e-03	0.016e-03
K32	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K34	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.008e-03	0.052e-03	-0.018e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.010e-03	-0.033e-03	-0.012e-03
K35	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K38	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.008e-03	0.053e-03	-0.018e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.010e-03	-0.032e-03	-0.012e-03
K40	Ch.C.(w1)	0.0001	0.0000	0.0000	0.018e-03	0.386e-03	-0.090e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.017e-03	0.177e-03	-0.197e-03
K42	Ch.C.(w1)	0.0001	0.0000	0.0000	-0.021e-03	-0.188e-03	-0.195e-03
	Ch.C.1	-0.0001	0.0000	0.0000	-0.021e-03	-0.059e-03	-0.463e-03
K44	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.027e-03	-0.172e-03	-0.237e-03
	Ch.C.1	-0.0003	0.0000	0.0000	-0.029e-03	-0.629e-03	-0.535e-03
K48	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.017e-03	0.047e-03	-0.030e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.024e-03	-0.029e-03	0.016e-03
K50	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.027e-03	-0.001e-03	-0.044e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.029e-03	0.051e-03	0.040e-03
-	-	m	m	m	rad	rad	rad

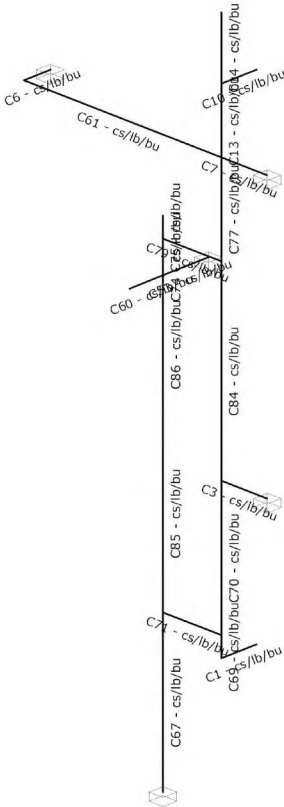
CH.C. EXTREME DEFLECTIONS

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S3	Ch.C. (w1)	0,000	0,000	0,000	0.089	0.0000	0.119	0.0000	0,000	0,000	0,000
S3	Ch.C.1	0,000	0,000	0,000	0.089	0.0000	0.136	0.0000	0,000	0,000	0,000
S7	Ch.C. (w1)	0,000	0,000	0,000	0.085	0.0000	0.000	0.0000	0,000	0,000	0,000
S7	Ch.C.1	0,000	0,000	0,000	0.086	0.0000	0.093	0.0000	0,000	0,000	0,000
S13	Ch.C. (w1)	0,000	0,000	0,000	0.160	0.0000	0.000	0.0000	0,000	0,000	0,000
S53	Ch.C. (w1)	0,000	0,000	0,000	0.112	0.0000	0.000	0.0000	0,000	0,000	0,000
S53	Ch.C.1	0,000	0,000	0,000	0.187	0.0000	0.000	0.0000	0,000	0,000	0,000
S61	Ch.C. (w1)	0,000	0,000	0,000	0.452	0.0000	0.361	0.0000	0,000	0,000	0,000
S61	Ch.C.1	0,000	0,000	0,000	0.454	0.0000	0.361	0.0000	0,000	0,000	0,000
S67	Ch.C. (w1)	0,000	0,000	0,000	0.350	0.0000	0.369	0.0000	0,000	0,000	0,000
S67	Ch.C.1	0,000	0,000	0,000	0.583	0.0000	0.394	0.0000	0,000	0,000	0,000
S69	Ch.C.1	0,000	0,000	0,000	0.052	0.0000	0.000	0.0000	0,000	0,000	0,000
S70	Ch.C.1	0,000	0,000	0,000	0.342	0.0000	0.351	0.0000	0,000	0,000	0,000
S71	Ch.C.1	0,000	0,000	0,000	0.000	0.0000	0.239	0.0000	0,000	0,000	0,000
S77	Ch.C. (w1)	0,000	0,000	0,000	0.238	0.0000	0.258	0.0000	0,000	0,000	0,000
S77	Ch.C.1	0,000	0,000	0,000	0.240	0.0000	0.250	0.0000	0,000	0,000	0,000
S79	Ch.C.1	0,000	0,000	0,000	0.000	0.0000	0.224	0.0000	0,000	0,000	0,000

	Pole 2	
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Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S84	Ch.C. (w1)	0,000	0,000	0,000	0.565	0.0000	0.383	0.0000	0,000	0,000	0,000
S84	Ch.C.1	0,000	0,000	0,000	0.396	0.0000	0.386	0.0000	0,000	0,000	0,000
S85	Ch.C. (w1)	0,000	0,000	0,000	0.505	0.0001	0.355	0.0000	0,000	0,000	0,000
S85	Ch.C.1	0,000	0,000	0,000	0.556	0.0000	0.353	0.0000	0,000	0,000	0,000
S86	Ch.C. (w1)	0,000	0,000	0,000	0.255	0.0000	0.340	0.0000	0,000	0,000	0,000
S86	Ch.C.1	0,000	0,000	0,000	0.191	0.0000	0.339	0.0000	0,000	0,000	0,000
-	-	m	m	m	m	m	m	m	m	m	m

PIC. STAALCONTROLE



SM DEF.

Structural member	Member (s)
C1	S1
C3	S3
C6	S6
C7	S7
C10	S10
C13	S13
C14	S14
C53	S53
C60	S60
C61	S61
C67	S67
C69	S69
C70	S70
C71	S71
C74	S74

	Pole 2	
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C75	S75
C77	S77
C79	S79
C84	S84
C85	S85
C86	S86

BUCKLING DATA

Member	Section	Y-axis (local system)				Z-axis (local system)		
		Lsys	Method	Lbuc	Lbuc/Lsys	Method	Lbuc	Lbuc/Lsys
C1 - V1 (0.000-0.200)	P3	0.200	Cons.Br.	0.200	1.00	Cons.Br.	0.200	1.00
C3 - V1 (0.000-0.260)	P1	0.260	Cons.Br.	0.260	1.00	Cons.Br.	0.260	1.00
C6 - V1 (0.000-0.150)	P2	0.150	Cons.Br.	0.150	1.00	Cons.Br.	0.150	1.00
C7 - V1 (0.000-0.260)	P1	0.260	Cons.Br.	0.260	1.00	Cons.Br.	0.260	1.00
C10 - V1 (0.000-0.200)	P3	0.200	Cons.Br.	0.200	1.00	Cons.Br.	0.200	1.00
C13 - V1 (0.000-0.320)	P1	0.320	Cons.Br.	0.320	1.00	Cons.Br.	0.320	1.00
C14 - V1 (0.000-0.310)	P1	0.310	Cons.Br.	0.310	1.00	Cons.Br.	0.310	1.00
C53 - V1 (0.000-0.260)	P1	0.260	Cons.Br.	0.260	1.00	Cons.Br.	0.260	1.00
C61 - V1 (0.000-1.115)	P1	1.120	Cons.Br.	1.115	1.00	Cons.Br.	1.115	1.00
C67 - V1 (0.000-0.780)	P1	0.780	Cons.Br.	0.780	1.00	Cons.Br.	0.780	1.00
C74 - V1 (0.000-0.160)	P1	0.160	Cons.Br.	0.160	1.00	Cons.Br.	0.160	1.00
C75 - V1 (0.000-0.100)	P1	0.100	Cons.Br.	0.100	1.00	Cons.Br.	0.100	1.00
C77 - V1 (0.000-0.450)	P1	0.450	Cons.Br.	0.450	1.00	Cons.Br.	0.450	1.00
C84 - V1 (0.000-0.950)	P1	0.950	Cons.Br.	0.950	1.00	Cons.Br.	0.950	1.00
C85 - V1 (0.000-0.860)	P1	0.860	Cons.Br.	0.860	1.00	Cons.Br.	0.860	1.00
C86 - V1 (0.000-0.600)	P1	0.600	Cons.Br.	0.600	1.00	Cons.Br.	0.600	1.00
-	-	m	-	m	-	-	m	-

LATERAL BUCKLING DATA

Member	Section	Begin:	End:	Lat. Supp. upper	Lat. Supp. lower	Load attachment
C1 - V1 (0.000-0.200)	P3	Cantilever	Supported			Center
C3 - V1 (0.000-0.260)	P1	Supported	Supported			Center
C6 - V1 (0.000-0.150)	P2	Supported	Supported			Center
C7 - V1 (0.000-0.260)	P1	Supported	Supported			Center
C10 - V1 (0.000-0.200)	P3	Cantilever	Supported			Center
C13 - V1 (0.000-0.320)	P1	Supported	Supported			Center
C14 - V1 (0.000-0.310)	P1	Supported	Cantilever			Center
C53 - V1 (0.000-0.260)	P1	Supported	Supported			Center
C60 - V1 (0.000-0.190)	P1	Supported	Cantilever			Center
C61 - V1 (0.000-1.115)	P1	Supported	Supported			Center
C67 - V1 (0.000-0.780)	P1	Supported	Supported			Center
C69 - V1 (0.000-0.100)	P1	Supported	Supported			Center
C70 - V1 (0.000-0.670)	P1	Supported	Supported			Center
C71 - V1 (0.000-0.330)	P4	Supported	Supported			Center
C74 - V1 (0.000-0.160)	P1	Supported	Supported			Center
C75 - V1 (0.000-0.100)	P1	Supported	Cantilever			Center
C77 - V1 (0.000-0.450)	P1	Supported	Supported			Center
C79 - V1 (0.000-0.330)	P4	Supported	Supported			Center
C84 - V1 (0.000-0.950)	P1	Supported	Supported			Center
C85 - V1 (0.000-0.860)	P1	Supported	Supported			Center
C86 - V1 (0.000-0.600)	P1	Supported	Supported			Center
-	-	-	-	m	m	-

STEEL CODECHECK RESULTS WITH SECTION DATA NEN-EN1993-1-1:2016/NB:2016

Assumptions of Analysis for steel codecheck

5.1.2.e; $\text{cr} = 572.23 > 10$;

Section data member C1-V1 (0.000-0.200)

C100	Analysis	Steel S235	$f_{yd}(\text{applied}) = 215 \text{ N/mm}^2$
$h = 100,0 \text{ mm}$	$A = 7,85\text{e-}03 \text{ m}^2$	$W_{y,eI} = 981.7\text{e-}07 \text{ m}^3$	$W_{y,pl} = 166.7\text{e-}06 \text{ m}^3$

Pole 2			
b = 100,0 mm	ly = 490.9e-08 m4	Wz;el = 981.7e-07 m3	Wz;pl = 166.7e-06 m3
tf = 0,0 mm	lz = 490.9e-08 m4	Aw;y;el = 5.89e-03 m2	Aw;y;pl = 5.89e-03 m2
tw = 0,0 mm	Mass/m = 61,7 kg/m	Aw;z;el = 5.89e-03 m2	Aw;z;pl = 5.89e-03 m2
r = 0,0 mm		It = 981.7e-08 m4	Iwa = 0.0e-02 m6
Crosscheck member C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.1 at 0,200 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,1 kNm	a1 = 0,500
	Vz;Ed = 0,3 kN	Mz;Ed = 0,0 kNm	a2 = 0,001
Nc;Rd = 1 688,6 kN	Vy;Rd = 731,2 kN	My;Rd = 35,8 kNm	p = 0,998
	Vz;Rd = 731,2 kN	Mz;Rd = 35,8 kNm	q = 1,030
NVy;Rd = 0,0 kN	NVz;Rd = 0,0 kN	MV;y;Rd = 35,8 kNm	MV;z;Rd = 35,8 kNm
NEN-EN1993-1-1(6.12): UC = 0,00 < 1			
Lateral Buckling member C1-V1 (0.000-0.200)			
Equi. section: C100		Instab. curve Lat. Buck.: d	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Cantilever	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,200 m	lst = 0,400 m
Lsys = 0,200 m	Lg = 0,200 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,200 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section with h/b < 3			
Stability Check C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.2			
N;Ed = -0,5 kN	Nb;Rd;y = 1 688,6 kN	Nb;Rd;z = 1 688,6 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,200 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,200 m
Chi;y = 1,00		Buckl. curve: C	
Chi;z = 1,00		Buckl. curve: C	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Bending & Compression C1-V1 (0.000-0.200)			
Decisive combination: Pe.C.2		Section class = 1	
N;Ed = -0,5 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,60	Cmz = 1,00	CmLT = 0,60	
Kyy = 0,600	Kyz = 0,600	Kzy = 0,681	Kzz = 1,000
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			
Section data member C3-V1 (0.000-0.260)			
N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	ly = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	lz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6
Crosscheck member C3-V1 (0.000-0.260)			
Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	

	Pole 2	
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N _x ;E _d = 0,0 kN	V _y ;E _d = -0,1 kN	My;E _d = 0,1 kNm	
	V _z ;E _d = -0,4 kN	Mz;E _d = 0,0 kNm	
N _c ;R _d = 166,3 kN	V _y ;R _d = 61,1 kN	My;R _d = 3,0 kNm	
	V _z ;R _d = 61,1 kN	Mz;R _d = 3,0 kNm	
NV _y ;R _d = 166,3 kN	NV _z ;R _d = 166,3 kN	MV _y ;R _d = 3,0 kNm	MV _z ;R _d = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV _y ;R _d = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV _z ;R _d = 3,0 kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,03 < 1			

Lateral Buckling member C3-V1 (0.000-0.260)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (2)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;l _{st} = 0,000 m	Xe;l _{st} = 0,260 m	l _{st} = 0,260 m
L _{sys} = 0,260 m	L _g = 0,260 m	S = 0,000 m	I _{wa} = 0.0000e+00 m6
C1 = 1,70	C2 = 1,42 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
5.12.e;LT(Pe.C.3) = 1,00	M;E _d = 0,1 kNm		UC(y) = 0,00
5.12.e;LT,Z = 1,00	I _{kip} = 0,260 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C3-V1 (0.000-0.260)

Decisive combination: Pe.C.1			
N;E _d = 0,0 kN	N _b ;R _d ;y = 166,3 kN	N _b ;R _d ;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	L _{buc} Y = 0,260 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	L _{buc} Z = 0,260 m
5.12.e , y = 1,00		Buckl. curve: A	
5.12.e , z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C3-V1 (0.000-0.260)

Decisive combination: Pe.C.1		Section class = 1	
N;E _d = 0,0 kN	My;E _d = 0,1 kNm	Mz;E _d = 0,0 kNm	
	Delta;My;E _d = 0,0 kNm	Delta;Mz;E _d = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
C _{my} = 0,42	C _{mz} = 0,73	C _{mLT} = 0,42	
K _{yy} = 0,415	K _{yz} = 0,435	K _{zy} = 0,249	K _{zz} = 0,725
5.12.e , y = 1,00	5.12.e , z = 1,00	5.12.e , LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,02 < 1			

Section data member C6-V1 (0.000-0.150)

K60x60x4x4	Analysis	Steel S235	f _{yd} (applied) = 235 N/mm2
h = 60,0 mm	A = 0,90e-03 m2	W _y ;e _l = 156.9e-07 m3	W _y ;p _l = 188.5e-07 m3
b = 60,0 mm	I _y = 470.7e-09 m4	W _z ;e _l = 156.9e-07 m3	W _z ;p _l = 188.5e-07 m3
t _f = 4,0 mm	I _z = 470.7e-09 m4	A _w ;y;e _l = 4.48e-04 m2	A _w ;y;p _l = 4.48e-04 m2
tw = 4,0 mm	Mass/m = 7,0 kg/m	A _w ;z;e _l = 4.48e-04 m2	A _w ;z;p _l = 4.48e-04 m2
r = 0,0 mm		I _t = 702.5e-09 m4	I _{wa} = 369.0e-12 m6

Crosscheck member C6-V1 (0.000-0.150)

Decisive combination: Pe.C.3 at 0,150 m		Section class = 1	
N _x ;E _d = -0,1 kN	V _y ;E _d = -0,1 kN	My;E _d = 0,0 kNm	a3(y) = 0,464
	V _z ;E _d = 0,0 kN	Mz;E _d = 0,0 kNm	a4(y) = 0,478
N _c ;R _d = 210,6 kN	V _y ;R _d = 60,8 kN	My;R _d = 4,4 kNm	a3(z) = 0,464
	V _z ;R _d = 60,8 kN	Mz;R _d = 4,4 kNm	a4(z) = 0,478
NV _y ;R _d = 210,6 kN	NV _z ;R _d = 210,6 kN	MV _y ;R _d = 4,4 kNm	MV _z ;R _d = 4,4 kNm

Pole 2				
$\alpha_1 = 1,000$		$\beta_1 = 1,000$	MNV;y;Rd = 4,4 kNm	q(y) = 1,030
$\alpha_2 = 2,000$		$\beta_2 = 0,750$	MNV;z;Rd = 4,4kNm	q(z) = 1,030
NEN-EN1993-1-1(6.31): UC = 0,01 < 1				
Lateral Buckling member C6-V1 (0.000-0.150)				
Equi. section: K60x60x4x4				
Decisive combination: Pe.C.3			Instab. curve Lat. 5.1.2.e	
Load Attachment Position: 0,000 m from section center				
Lateral buckling supports upper flange: None				
Lateral buckling supports lower flange: None				
Restr. begin: Supported		Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)		= 0,0kN/m	= 0,0	
Upper flange decisive		Xb;lst = 0,000 m	Xe;lst = 0,150 m	lst = 0,150 m
Lsys = 0,150 m		Lg = 0,150 m	S = 0,037 m	Iwa = 3.6903e-10 m6
C1 = 2,30		C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm		kred = 1.0	Lam-rel = 0,00	Section class 1
5.12a, LT(Pe.C.3) = 1,00		M;Ed = 0,0 kNm		UC(y) = 0,00
5.12a, LT,Z = 1,00		lkip = 0,150 m		UC(z) = 0,00
My;begin = 0,0 kNm		My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: Flange is under tension				
Stability Check C6-V1 (0.000-0.150)				
Decisive combination: Pe.C.3				
N;Ed = -0,1 kN		Nb;Rd;y = 210,6 kN	Nb;Rd;z = 210,6 kN	
Method Y = Cons.Br.		Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,150 m
Method Z = Cons.Br.		Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,150 m
5.12a, y = 1,00			Buckl. curve: A	
5.12a, z = 1,00			Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1				
Bending & Compression C6-V1 (0.000-0.150)				
Decisive combination: Pe.C.3				
N;Ed = -0,1 kN		My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
		Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm		My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm		Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,88		Cmz = 0,65	CmLT = 0,88	
Kyy = 0,880		Kyz = 0,392	Kzy = 0,528	Kzz = 0,654
5.12a, y = 1,00		5.12a, z = 1,00	5.12a, LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1				
Section data member C7-V1 (0.000-0.260)				
N60.3/4	Analysis	Steel S235	fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3		Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3		Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2		Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2		Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4		Iwa = 0.0e-02 m6
Crosscheck member C7-V1 (0.000-0.260)				
Decisive combination: Pe.C.3 at 0,000 m				
N;Ed = -0,1 kN		Vy;Ed = -0,6 kN	My;Ed = 0,1 kNm	Mx;Ed = 0,1 kNm
		Vz;Ed = -0,3 kN	Mz;Ed = 0,1 kNm	
N;Rd = 166,3 kN		Vy;Rd = 61,1 kN	MyRd = 3,0 kNm	$\sigma_{HH} = 15,2 \text{ kN/m2}$
		Vz;Rd = 61,1 kN	MzRd = 3,0 kNm	
NEN-EN1993-1-1(6.1): UC = 0,06 < 1				
Lateral Buckling member C7-V1 (0.000-0.260)				
Equi. section: N60.3/4				
Decisive combination: Pe.C.3			Instab. curve Lat. 5.1.2.e	
Load Attachment Position: 0.000 m from section center				

	Pole 2	
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Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,260 m	lst = 0,260 m
Lsys = 0,260 m	Lg = 0,260 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,260 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C7-V1 (0.000-0.260)

Decisive combination: Pe.C.3			
N;Ed = -0,1 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,260 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,260 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C7-V1 (0.000-0.260)

Decisive combination: Pe.C.3		Section class = 1	
N;Ed = -0,1 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,1 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,1 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,40	Cmz = 0,47	CmLT = 0,40	
Kyy = 0,400	Kyz = 0,282	Kzy = 0,240	Kzz = 0,469
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,02 < 1			

Section data member C10-V1 (0.000-0.200)

C100	Analysis	Steel S235	f _{yd} (applied) = 215 N/mm2
h = 100,0 mm	A = 7,85e-03 m2	Wy;el = 981.7e-07 m3	Wy;pl = 166.7e-06 m3
b = 100,0 mm	Iy = 490.9e-08 m4	Wz;el = 981.7e-07 m3	Wz;pl = 166.7e-06 m3
tf = 0,0 mm	Iz = 490.9e-08 m4	Aw;y;el = 5.89e-03 m2	Aw;y;pl = 5.89e-03 m2
tw = 0,0 mm	Mass/m = 61,7 kg/m	Aw;z;el = 5.89e-03 m2	Aw;z;pl = 5.89e-03 m2
r = 0,0 mm		It = 981.7e-08 m4	Iwa = 0.0e-02 m6

Crosscheck member C10-V1 (0.000-0.200)

Decisive combination: Pe.C.1 at 0,200 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,1 kNm	a1 = 0,500
	Vz;Ed = 0,3 kN	Mz;Ed = 0,0 kNm	a2 = 0,001
Nc;Rd = 1 688,6 kN	Vy;Rd = 731,2 kN	My;Rd = 35,8 kNm	p = 0,998
	Vz;Rd = 731,2 kN	Mz;Rd = 35,8 kNm	q = 1,030
NVy;Rd = 0,0 kN	NVz;Rd = 0,0 kN	MV;y;Rd = 35,8 kNm	MV;z;Rd = 35,8 kNm
NEN-EN1993-1-1(6.12): UC = 0,00 < 1			

Lateral Buckling member C10-V1 (0.000-0.200)

Equi. section: C100			
Decisive combination: Pe.C.3		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Cantilever	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,200 m	lst = 0,400 m
Lsys = 0,200 m	Lg = 0,200 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00

Pole 2			
Mcr = 0,0 kNm			
Ch _i ;LT(Pe.C.3) = 1,00			
Ch _i ;LT,Z = 1,00			
My _{begin} = 0,0 kNm			
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section with h/b < 3			
k _{red} = 1.0			
M _i ;Ed = 0,0 kNm			
l _{kip} = 0,200 m			
My _{end} = 0,0 kNm			
Lam-rel = 0,00			
Section class 1			
UC(y) = 0,00			
UC(z) = 0,00			
Stability Check C10-V1 (0.000-0.200)			
Decisive combination: Pe.C.2			
N _i ;Ed = -0,5 kN			
Method Y = Cons.Br.			
Method Z = Cons.Br.			
Ch _i ;y = 1,00			
Ch _i ;z = 1,00			
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Nb;Rd;y = 1 688,6 kN			
Ca(y) = 0,000			
Cb(y) = 0,000			
Lbuc Y = 0,200 m			
Ca(z) = N/A			
Cb(z) = N/A			
Lbuc Z = 0,200 m			
Buckl. curve: C			
Buckl. curve: C			
Bending & Compression C10-V1 (0.000-0.200)			
Decisive combination: Pe.C.2			
N _i ;Ed = -0,5 kN			
My = 0,1 kNm			
Mz = 0,0 kNm			
C _{my} = 0,60			
K _{yy} = 0,600			
Ch _i ;y = 1,00			
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			
Lat-tors. Sensitive Yes			
My;Ed = 0,1 kNm			
Delta;My;Ed = 0,0 kNm			
My;Psi = 0,0 kNm			
Mz;Psi = 0,0 kNm			
C _{mz} = 1,00			
K _{yz} = 0,600			
Ch _i ;z = 1,00			
Section class = 1			
Mz;Ed = 0,0 kNm			
Delta;Mz;Ed = 0,0 kNm			
My;s = 0,0 kNm			
Mz;s = 0,0 kNm			
C _{mLT} = 0,60			
K _{zy} = 0,681			
Ch _i ;LT = 1,00			
K _{zz} = 1,000			
Section data member C13-V1 (0.000-0.320)			
N60.3/4			
h = 60,3 mm			
b = 60,3 mm			
t _f = 4,0 mm			
t _w = 4,0 mm			
r = 26,1 mm			
Analysis			
A = 0,71e-03 m ²			
I _y = 281.7e-09 m ⁴			
I _z = 281.7e-09 m ⁴			
Mass/m = 5,6 kg/m			
Steel S235 f _{yd} (applied) = 235 N/mm ²			
W _y ;el = 934.4e-08 m ³			
W _z ;el = 934.4e-08 m ³			
A _w ;y;el = 4.50e-04 m ²			
A _w ;z;el = 4.50e-04 m ²			
I _t = 563.5e-09 m ⁴			
W _y ;pl = 127.0e-07 m ³			
W _z ;pl = 127.0e-07 m ³			
A _w ;y;pl = 4.50e-04 m ²			
A _w ;z;pl = 4.50e-04 m ²			
I _{wa} = 0.0e-02 m ⁶			
Crosscheck member C13-V1 (0.000-0.320)			
Decisive combination: Pe.C.3 at 0,000 m			
N _i ;Ed = -0,2 kN			
N _i ;Rd = 166,3 kN			
NEN-EN1993-1-1(NB.33): UC = 0,07 < 1			
V _y ;Ed = 0,0 kN			
V _y ;Rd = 61,1 kN			
V _z ;Ed = 0,5 kN			
V _z ;Rd = 61,1 kN			
Section class = 1			
My;Ed = -0,2 kNm			
Mz;Ed = 0,0 kNm			
MyRd = 3,0 kNm			
MzRd = 3,0 kNm			
Lateral Buckling member C13-V1 (0.000-0.320)			
Equi. section: N60.3/4			
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported			
Table used: NB.NB.1 (1)			
Upper flange decisive			
L _{sys} = 0,320 m			
C1 = 1,55			
Mcr = 0,0 kNm			
Ch _i ;LT(Pe.C.3) = 1,00			
Ch _i ;LT,Z = 1,00			
My _{begin} = -0,2 kNm			
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: Flange is under tension			
Restr. end: Supported			
M = -0,2kN/m			
X _b ;l _{st} = 0,000 m			
L _g = 0,320 m			
C2 = 0,00 (table)			
k _{red} = 1.0			
M _i ;Ed = 0,0 kNm			
l _{kip} = 0,320 m			
My _{end} = 0,0 kNm			
b-eff(Begin) = 0,000			
MBeta = 0,0			
X _e ;l _{st} = 0,320 m			
S = 0,000 m			
C2(applied) = 0,00			
Lam-rel = 0,00			
b-eff(End) = 0,000			
l _{st} = 0,320 m			
I _{wa} = 0.0000e+00 m ⁶			
C = 0,00			
Section class 1			
UC(y) = 0,00			
UC(z) = 0,00			
Stability Check C13-V1 (0.000-0.320)			
Decisive combination: Pe.C.3			

Pole 2			
N;Ed = -0,2 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,320 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,320 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Bending & Compression C13-V1 (0.000-0.320)			
Decisive combination: Pe.C.3		Section class = 1	
N;Ed = -0,2 kN	My;Ed = 0,2 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = -0,2 kNm	My;Psi = 0,0 kNm	My;s = -0,1 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,68	Cmz = 1,00	CmLT = 0,68	
Kyy = 0,680	Kyz = 0,600	Kzy = 0,408	Kzz = 1,000
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,05 < 1			
Section data member C14-V1 (0.000-0.310)			
N60.3/4	Analysis	Steel S235	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	Iy = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	Iz = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		It = 563.5e-09 m ⁴	Iwa = 0.0e-02 m ⁶
Crosscheck member C14-V1 (0.000-0.310)			
Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,0 kNm	
	Vz;Ed = 0,0 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 0,0 kN	NVz;Rd = 0,0 kN	MV;y;Rd = 0,0 kNm	MV;z;Rd = 0,0 kNm
NEN-EN1993-1-1(6.9): UC = 0,00 < 1			
Lateral Buckling member C14-V1 (0.000-0.310)			
Equi. section: N60.3/4		Instab. curve Lat. Buck.: d	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Cantilever	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,310 m	lst = 0,620 m
Lsys = 0,310 m	Lg = 0,310 m	S = 0,000 m	Iwa = 0.0000e+00 m ⁶
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 0,310 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: no bending			
Stability Check C14-V1 (0.000-0.310)			
Decisive combination: Pe.C.1			
N;Ed = 0,0 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,310 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,310 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

	Pole 2	
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Bending & Compression C14-V1 (0.000-0.310)

Decisive combination: Pe.C.1		Section class = 1	
N _i Ed = 0,0 kN	My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 1,00	Cmz = 1,00	CmLT = 1,00	
Kyy = 1,000	Kyz = 0,600	Kzy = 0,600	Kzz = 1,000
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			

Section data member C53-V1 (0.000-0.260)

N60.3/4	Analysis	Steel S235	f _{yd} (applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C53-V1 (0.000-0.260)

Decisive combination: Pe.C.3 at 0,260 m		Section class = 1	
N _x Ed = -0,1 kN	Vy;Ed = 0,1 kN	My;Ed = 0,1 kNm	
	Vz;Ed = 0,6 kN	Mz;Ed = 0,0 kNm	
N _c Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NV _y ;Rd = 166,3 kN	NV _z ;Rd = 166,3 kN	MV _y ;Rd = 3,0 kNm	MV _z ;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV _y ;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV _z ;Rd = 3,0 kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,04 < 1			

Lateral Buckling member C53-V1 (0.000-0.260)

Equi. section: N60.3/4		Instab. curve Lat. Buck.:d	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.4	M = 0,1kN/m	MBeta = 0,0	q = 0,4
Upper flange decisive	Xb;l _{st} = 0,000 m	Xe;l _{st} = 0,260 m	l _{st} = 0,260 m
L _{sys} = 0,260 m	L _g = 0,260 m	S = 0,000 m	I _{wa} = 0.0000e+00 m6
C1 = 1,91	C2 = 0,02 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	l _{kip} = 0,260 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0.00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C53-V1 (0.000-0.260)

Decisive combination: Pe.C.3			
N _i Ed = -0,1 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,260 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,260 m
Chi;y = 1,00		Buckl. curve: A	
Chi;z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C53-V1 (0.000-0.260)

Decisive combination: Pe.C.3		Section class = 1	
N _i Ed = -0,1 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,1 kNm	

	Pole 2	
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Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,54	Cmz = 0,57	CmLT = 0,54	
Kyy = 0,537	Kyz = 0,343	Kzy = 0,322	Kzz = 0,571
Chi;y = 1,00	Chi;z = 1,00	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,03 < 1			

Section data member C60-V1 (0.000-0.190)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C60-V1 (0.000-0.190)

Decisive combination: Pe.C.1 at 0,000 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,0 kNm	
	Vz;Ed = 0,0 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
NEN-EN1993-1-1(6.12): UC = 0,00 < 1			

Lateral Buckling member C60-V1 (0.000-0.190)

Equi. section: N60.3/4			
Decisive combination: Pe.C.3		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Cantilever	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,190 m	lst = 0,380 m
Lsys = 0,190 m	Lg = 0,190 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,190 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0.00 < 1 Lat.buc.N/A: Flange is under tension			

Section data member C61-V1 (0.000-1.115)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C61-V1 (0.000-1.115)

Decisive combination: Pe.C.3 at 0,000 m		Section class = 1	
N;Ed = -0,1 kN	Vy;Ed = 0,1 kN	My;Ed = 0,0 kNm	
	Vz;Ed = 0,0 kN	Mz;Ed = 0,0 kNm	
N;Rd = 166,3 kN	Vy;Rd = 61,1 kN	MyRd = 3,0 kNm	
	Vz;Rd = 61,1 kN	MzRd = 3,0 kNm	
NEN-EN1993-1-1(NB.33): UC = 0,01 < 1			

Lateral Buckling member C61-V1 (0.000-1.115)

Equi. section: N60.3/4			
Decisive combination: Pe.C.3		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			

	Pole 2	
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Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 1,115 m	lst = 1,115 m
Lsys = 1,115 m	Lg = 1,115 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	Ikip = 1,115 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C61-V1 (0.000-1.115)

Decisive combination: Pe.C.3			
N;Ed = -0,1 kN	Nb;Rd;y = 148,3 kN	Nb;Rd;z = 148,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 1,115 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 1,115 m
Chi;y = 0,89		Buckl. curve: A	
Chi;z = 0,89		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C61-V1 (0.000-1.115)

Decisive combination: Pe.C.3		Section class = 1	
N;Ed = -0,1 kN	My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,72	Cmz = 0,40	CmLT = 0,72	
Kyy = 0,716	Kyz = 0,240	Kzy = 0,430	Kzz = 0,400
Chi;y = 0,89	Chi;z = 0,89	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1			

Section data member C67-V1 (0.000-0.780)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C67-V1 (0.000-0.780)

Decisive combination: Pe.C.3 at 0,000 m		Section class = 1	
Nx;Ed = -1,4 kN	Vy;Ed = 0,0 kN	My;Ed = -0,2 kNm	
	Vz;Ed = 0,4 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
α 1 = 2,000	β 1 = 1,000	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
α 2 = 2,000	β 2 = 1,000	MNV;z;Rd = 3,0kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,06 < 1			

Lateral Buckling member C67-V1 (0.000-0.780)

Equi. section: N60.3/4			
Decisive combination: Pe.C.3		Instab. curve Lat. Buck.:d	
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.7	M = -0,2kN/m	MBeta = 0,1	F = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,780 m	lst = 0,780 m
Lsys = 0,780 m	Lg = 0,780 m	S = 0,000 m	Iwa = 0.0000e+00 m6

	Pole 2	
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C1 = 2,30	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,780 m		UC(z) = 0,00
My;begin = -0,2 kNm	My;end = 0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C67-V1 (0.000-0.780)

Decisive combination: Pe.C.3

N;Ed = -1,4 kN	Nb;Rd;y = 157,7 kN	Nb;Rd;z = 157,7 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,780 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,780 m
Chi;y = 0,95		Buckl. curve: A	
Chi;z = 0,95		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,01 < 1			

Bending & Compression C67-V1 (0.000-0.780)

Decisive combination: Pe.C.3

N;Ed = -1,4 kN	My;Ed = 0,2 kNm	Mz;Ed = 0,0 kNm	Section class = 1
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = -0,2 kNm	My;Psi = 0,1 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,40	Cmz = 0,53	CmLT = 0,40	
Kyy = 0,401	Kyz = 0,318	Kzy = 0,240	Kzz = 0,530
Chi;y = 0,95	Chi;z = 0,95	Chi;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,03 < 1			

Section data member C69-V1 (0.000-0.100)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C69-V1 (0.000-0.100)

Decisive combination: Pe.C.2 at 0,100 m

Nx;Ed = 0,3 kN	Vy;Ed = 0,0 kN	My;Ed = 0,1 kNm	
	Vz;Ed = 0,5 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NV;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
NEN-EN1993-1-1(NB.33): UC = 0,04 < 1			

Lateral Buckling member C69-V1 (0.000-0.100)

Equi. section: N60.3/4

Decisive combination: Pe.C.3

Load Attachment Position: 0,000 m from section center

Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,100 m	lst = 0,100 m
Lsys = 0,100 m	Lg = 0,100 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
Chi;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
Chi;LT,Z = 1,00	lkip = 0,100 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

	Pole 2	
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Section data member C70-V1 (0.000-0.670)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C70-V1 (0.000-0.670)

Decisive combination: Pe.C.3 at 0,670 m		Section class = 1	
Nx;Ed = 0,2 kN	Vy;Ed = 0,0 kN	My;Ed = -0,1 kNm	
	Vz;Ed = -0,3 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
$\alpha_1 = 2,000$	$\beta_1 = 1,000$	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
$\alpha_2 = 2,000$	$\beta_2 = 1,000$	MNV;z;Rd = 3,0 kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,05 < 1			

Lateral Buckling member C70-V1 (0.000-0.670)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.4	M = -0,1 kN/m	MBeta = 0,1	q = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,670 m	lst = 0,670 m
Lsys = 0,670 m	Lg = 0,670 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,28	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1,0	Lam-rel = 0,00	Section class 1
5.12.a , LT(Pe.C.3) = 1,00	M;Ed = 0,1 kNm		UC(y) = 0,00
5.12.a , LT,Z = 1,00	lkip = 0,670 m		UC(z) = 0,00
My;begin = 0,1 kNm	My;end = -0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C71-V1 (0.000-0.330)

B48.3x3.2	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 48,3 mm	A = 0,45e-03 m2	Wy;el = 479.7e-08 m3	Wy;pl = 652.0e-08 m3
b = 48,3 mm	Iy = 115.9e-09 m4	Wz;el = 479.7e-08 m3	Wz;pl = 652.0e-08 m3
tf = 3,2 mm	Iz = 115.9e-09 m4	Aw;y;el = 2.89e-04 m2	Aw;y;pl = 2.89e-04 m2
tw = 3,2 mm	Mass/m = 3,6 kg/m	Aw;z;el = 2.89e-04 m2	Aw;z;pl = 2.89e-04 m2
r = 0,0 mm		It = 231.7e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C71-V1 (0.000-0.330)

Decisive combination: Pe.C.2 at 0,330 m		Section class = 1	
N;Ed = 0,0 kN	Vy;Ed = -0,4 kN	My;Ed = 0,0 kNm	
	Vz;Ed = 0,1 kN	Mz;Ed = -0,1 kNm	
N;Rd = 106,5 kN	Vy;Rd = 39,2 kN	MyRd = 1,5 kNm	
	Vz;Rd = 39,2 kN	MzRd = 1,5 kNm	
NEN-EN1993-1-1(NB.33): UC = 0,05 < 1			

Lateral Buckling member C71-V1 (0.000-0.330)

Equi. section: B48.3x3.2		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0 kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,330 m	lst = 0,330 m
Lsys = 0,330 m	Lg = 0,330 m	S = 0,000 m	Iwa = 0.0000e+00 m6

C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
§12a LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
§12a LT,Z = 1,00	lkip = 0,330 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Section data member C74-V1 (0.000-0.160)

N60.3/4	Analysis	Steel S235	fyd(applied) = 235 N/mm2
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C74-V1 (0.000-0.160)

Decisive combination: Pe.C.3 at 0,160 m		Section class = 1	
Nx;Ed = 0,0 kN	Vy;Ed = 0,1 kN	My;Ed = 0,0 kNm	
	Vz;Ed = -0,1 kN	Mz;Ed = 0,0 kNm	
Nc;Rd = 166,3 kN	Vy;Rd = 61,1 kN	My;Rd = 3,0 kNm	
	Vz;Rd = 61,1 kN	Mz;Rd = 3,0 kNm	
NVy;Rd = 166,3 kN	NVz;Rd = 166,3 kN	MV;y;Rd = 3,0 kNm	MV;z;Rd = 3,0 kNm
$\alpha_1 = 2,000$	$\beta_1 = 1,000$	MNV;y;Rd = 3,0 kNm	q(y) = 1,030
$\alpha_2 = 2,000$	$\beta_2 = 1,000$	MNV;z;Rd = 3,0 kNm	q(z) = 1,030
NEN-EN1993-1-1(NB.33): UC = 0,01 < 1			

Lateral Buckling member C74-V1 (0.000-0.160)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (1)	= 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,160 m	lst = 0,160 m
Lsys = 0,160 m	Lg = 0,160 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 2,30	C2 = 1,55 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
§12.a ;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
§12.a ;LT,Z = 1,00	lkip = 0,160 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

Stability Check C74-V1 (0.000-0.160)

Decisive combination: Pe.C.2			
N;Ed = -0,1 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,160 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,160 m
§12a y = 1,00		Buckl. curve: A	
§12a z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C74-V1 (0.000-0.160)

Decisive combination: Pe.C.2		Section class = 1	
N;Ed = -0,1 kN	My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,51	Cmz = 0,91	CmLT = 0,51	
Kyy = 0,515	Kyz = 0,545	Kzy = 0,309	Kzz = 0,909
§12a y = 1,00	§12a z = 1,00	§12a LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			

Section data member C75-V1 (0.000-0.100)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6

Crosscheck member C75-V1 (0.000-0.100)

Decisive combination: Pe.C.1 at 0,100 m		Section class = 1
N;Ed = 0,0 kN	Vy;Ed = 0,0 kN	My;Ed = 0,0 kNm
	Vz;Ed = 0,0 kN	Mz;Ed = 0,0 kNm
N;Rd = 166,3 kN	Vy;Rd = 61,1 kN	MyRd = 3,0 kNm
	Vz;Rd = 61,1 kN	MzRd = 3,0 kNm
NEN-EN1993-1-1(6.5): UC = 0,00 < 1		

Lateral Buckling member C75-V1 (0.000-0.100)

Equi. section: N60.3/4			
Decisive combination: Pe.C.3		Instab. curve Lat	5.1.2.e
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Cantilever	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.1 (5)	M = 0,0kN/m	= 0,0	
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,100 m	lst = 0,200 m
Lsys = 0,100 m	Lg = 0,100 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,00	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
5.12a: LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
5.12a: LT,Z = 1,00	Ikip = 0,100 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = 0,0 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: no bending			

Stability Check C75-V1 (0.000-0.100)

Decisive combination: Pe.C.1			
N;Ed = 0,0 kN	Nb;Rd;y = 166,3 kN	Nb;Rd;z = 166,3 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	Lbuc Y = 0,100 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	Lbuc Z = 0,100 m
5.12a: y = 1,00		Buckl. curve: A	
5.12a: Z = 1,00		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C75-V1 (0.000-0.100)

Decisive combination: Pe.C.1			
N;Ed = 0,0 kN	My;Ed = 0,0 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,0 kNm	My;Psi = 0,0 kNm	My;s = 0,0 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 1,00	Cmz = 1,00	CmLT = 1,00	
Kyy = 1,000	Kyz = 0,600	Kzy = 0,600	Kzz = 1,000
5.12a: y = 1,00	5.12a: z = 1,00	5.12a: LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,00 < 1			

Section data member C77-V1 (0.000-0.450)

N60.3/4	Analysis	Steel S235 fyd(applied) = 235 N/mm2	
h = 60,3 mm	A = 0,71e-03 m2	Wy;el = 934.4e-08 m3	Wy;pl = 127.0e-07 m3
b = 60,3 mm	Iy = 281.7e-09 m4	Wz;el = 934.4e-08 m3	Wz;pl = 127.0e-07 m3
tf = 4,0 mm	Iz = 281.7e-09 m4	Aw;y;el = 4.50e-04 m2	Aw;y;pl = 4.50e-04 m2
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m2	Aw;z;pl = 4.50e-04 m2

Pole 2			
r = 26,1 mm		It = 563.5e-09 m4	Iwa = 0.0e-02 m6
Crosscheck member C77-V1 (0.000-0.450)			
Decisive combination: Pe.C.3 at 0,450 m		Section class = 1	
N _i Ed = 0,1 kN	V _y Ed = 0,1 kN	M _y Ed = -0,1 kNm	
	V _z Ed = -0,1 kN	M _z Ed = 0,0 kNm	
N _i Rd = 166,3 kN	V _y Rd = 61,1 kN	M _y Rd = 3,0 kNm	
	V _z Rd = 61,1 kN	M _z Rd = 3,0 kNm	
NEN-EN1993-1-1(NB.33): UC = 0,03 < 1			
Lateral Buckling member C77-V1 (0.000-0.450)			
Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.3 (2)	= 0,0kN/m	= 0,0	
Upper flange decisive	X _b l _{st} = 0,000 m	X _e l _{st} = 0,450 m	l _{st} = 0,450 m
L _{sys} = 0,450 m	L _g = 0,450 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,70	C2 = 1,42 (table)	C2(applied) = 0,00	C = 0,00
M _{cr} = 0,0 kNm	k _{red} = 1.0	Lam-rel = 0,00	Section class 1
5.12.e, LT(Pe.C.3) = 1,00	M _i Ed = 0,0 kNm		UC(y) = 0,00
5.12.e, LT,Z = 1,00	I _{kip} = 0,450 m		UC(z) = 0,00
M _y begin = 0,0 kNm	M _y end = -0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: Flange is under tension			
Stability Check C77-V1 (0.000-0.450)			
Decisive combination: Pe.C.2			
N _i Ed = 0,0 kN	N _b Rd _y = 164,8 kN	N _b Rd _z = 164,8 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	L _{buc} Y = 0,450 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	L _{buc} Z = 0,450 m
5.12.e, y = 0,99		Buckl. curve: A	
5.12.e, z = 0,99		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			
Bending & Compression C77-V1 (0.000-0.450)			
Decisive combination: Pe.C.2		Section class = 1	
N _i Ed = 0,0 kN	M _y Ed = 0,0 kNm	M _z Ed = 0,0 kNm	
	Delta;M _y Ed = 0,0 kNm	Delta;M _z Ed = 0,0 kNm	
M _y = 0,0 kNm	M _y ;Psi = 0,0 kNm	M _y ;s = 0,0 kNm	
M _z = 0,0 kNm	M _z ;Psi = 0,0 kNm	M _z ;s = 0,0 kNm	
C _{my} = 0,79	C _{mz} = 0,74	C _m LT = 0,79	
K _{yy} = 0,786	K _{yz} = 0,445	K _{zy} = 0,471	K _{zz} = 0,741
5.12.e, y = 0,99	5.12.e, z = 0,99	5.12.e, LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1			
Section data member C79-V1 (0.000-0.330)			
B48.3x3.2	Analysis	Steel S235	f _{yd} (applied) = 235 N/mm2
h = 48,3 mm	A = 0,45e-03 m2	W _y ;el = 479.7e-08 m3	W _y ;pl = 652.0e-08 m3
b = 48,3 mm	I _y = 115.9e-09 m4	W _z ;el = 479.7e-08 m3	W _z ;pl = 652.0e-08 m3
t _f = 3,2 mm	I _z = 115.9e-09 m4	A _w ;y;el = 2.89e-04 m2	A _w ;y;pl = 2.89e-04 m2
t _w = 3,2 mm	Mass/m = 3,6 kg/m	A _w ;z;el = 2.89e-04 m2	A _w ;z;pl = 2.89e-04 m2
r = 0,0 mm		It = 231.7e-09 m4	Iwa = 0.0e-02 m6
Crosscheck member C79-V1 (0.000-0.330)			
Decisive combination: Pe.C.3 at 0,330 m		Section class = 1	
N _i Ed = 0,1 kN	V _y Ed = -0,1 kN	M _y Ed = 0,0 kNm	
	V _z Ed = 0,0 kN	M _z Ed = 0,0 kNm	
N _i Rd = 106,5 kN	V _y Rd = 39,2 kN	M _y Rd = 1,5 kNm	

	Pole 2	
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Vz;Rd = 39,2 kN

MzRd = 1,5 kNm

NEN-EN1993-1-1(NB.33): UC = 0,02 < 1

Lateral Buckling member C79-V1 (0.000-0.330)

Equi. section: B48.3x3.2

Decisive combination: Pe.C.3

Instab. curve Lat. **5.1.2.e**

Load Attachment Position: 0,000 m from section center

Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported

Restr. end: Supported

b-eff(Begin) = 0,000

b-eff(End) = 0,000

Table used: NB.NB.3 (1)

= 0,0kN/m

= 0,0

Upper flange decisive

Xb;lst = 0,000 m

Xe;lst = 0,330 m

lst = 0,330 m

Lsys = 0,330 m

Lg = 0,330 m

S = 0,000 m

lwa = 0.0000e+00 m6

C1 = 2,30

C2 = 1,55 (table)

C2(applied) = 0,00

C = 0,00

Mcr = 0,0 kNm

kred = 1.0

Lam-rel = 0,00

Section class 1

5.12.e, LT(Pe.C.3) = 1,00

M;Ed = 0,0 kNm

UC(y) = 0,00

5.12.e, LT,Z = 1,00

lkip = 0,330 m

UC(z) = 0,00

My;begin = 0,0 kNm

My;end = 0,0 kNm

NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)

Section data member C84-V1 (0.000-0.950)

N60.3/4

Analysis

Steel S235 fyd(applied) = 235 N/mm2

h = 60,3 mm

A = 0,71e-03 m2

Wy;el = 934.4e-08 m3

Wy;pl = 127.0e-07 m3

b = 60,3 mm

Iy = 281.7e-09 m4

Wz;el = 934.4e-08 m3

Wz;pl = 127.0e-07 m3

tf = 4,0 mm

Iz = 281.7e-09 m4

Aw;y;el = 4.50e-04 m2

Aw;y;pl = 4.50e-04 m2

tw = 4,0 mm

Mass/m = 5,6 kg/m

Aw;z;el = 4.50e-04 m2

Aw;z;pl = 4.50e-04 m2

r = 26,1 mm

It = 563.5e-09 m4

lwa = 0.0e-02 m6

Crosscheck member C84-V1 (0.000-0.950)

Decisive combination: Pe.C.2 at 0,000 m

Section class = 1

Nx;Ed = 0,0 kN

Vy;Ed = 0,0 kN

My;Ed = 0,1 kNm

Nc;Rd = 166,3 kN

Vz;Ed = -0,1 kN

Mz;Ed = 0,0 kNm

Vy;Rd = 61,1 kN

My;Rd = 3,0 kNm

Vz;Rd = 61,1 kN

Mz;Rd = 3,0 kNm

NVv;Rd = 166,3 kN

NVz;Rd = 166,3 kN

MV;y;Rd = 3,0 kNm

MV;z;Rd = 3,0 kNm

$\alpha_1 = 2,000$

$\beta_1 = 1,000$

MNV;y;Rd = 3,0 kNm

q(y) = 1,030

$\alpha_2 = 2,000$

$\beta_2 = 1,000$

MNV;z;Rd = 3,0kNm

q(z) = 1,030

NEN-EN1993-1-1(NB.33): UC = 0,02 < 1

Lateral Buckling member C84-V1 (0.000-0.950)

Equi. section: N60.3/4

Decisive combination: Pe.C.3

Instab. curve Lat. **5.1.2.e**

Load Attachment Position: 0,000 m from section center

Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported

Restr. end: Supported

b-eff(Begin) = 0,000

b-eff(End) = 0,000

Table used: NB.NB.1 (1)

M = -0,1kN/m

MBeta = 0,0

Upper flange decisive

Xb;lst = 0,000 m

Xe;lst = 0,950 m

lst = 0,950 m

Lsys = 0,950 m

Lg = 0,950 m

S = 0,000 m

lwa = 0.0000e+00 m6

C1 = 1,37

C2 = 0,00 (table)

C2(applied) = 0,00

C = 0,00

Mcr = 0,0 kNm

kred = 1.0

Lam-rel = 0,00

Section class 1

5.12.e, LT(Pe.C.3) = 1,00

M;Ed = 0,0 kNm

UC(y) = 0,00

5.12.e, LT,Z = 1,00

lkip = 0,950 m

UC(z) = 0,00

My;begin = -0,1 kNm

My;end = 0,0 kNm

NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: Flange is under tension

Stability Check C84-V1 (0.000-0.950)

Decisive combination: Pe.C.3

N;Ed = 0,0 kN

Nb;Rd;y = 153,3 kN

Nb;Rd;z = 153,3 kN

Method Y = Cons.Br.

Ca(y) = 0,000

Cb(y) = 0,000

Lbuc Y = 0,950 m

Method Z = Cons.Br.

Ca(z) = N/A

Cb(z) = N/A

Lbuc Z = 0,950 m

5.12.e, y = 0,92

Buckl. curve: A

	Pole 2	
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Chi; z = 0,92

Buckl. curve: A

NEN-EN1993-1-1(6.46): UC = 0,00 < 1

Bending & Compression C84-V1 (0.000-0.950)

Decisive combination: Pe.C.3

N;Ed = 0,0 kN

My;Ed = 0,1 kNm

Delta;My;Ed = 0,0 kNm

Section class = 1

Mz;Ed = 0,0 kNm

Delta;Mz;Ed = 0,0 kNm

My = -0,1 kNm

My;Psi = 0,0 kNm

My;s = 0,0 kNm

Mz = 0,0 kNm

Mz;Psi = 0,0 kNm

Mz;s = 0,0 kNm

Cmy = 0,76

Cmz = 0,53

CmLT = 0,76

Kyy = 0,763

Kyz = 0,319

Kzy = 0,458

Kzz = 0,532

Chi; y = 0,92

Chi; z = 0,92

Chi;LT = 1,00

NEN-EN1993-1-1(6.61&6.62): UC = 0,01 < 1

Section data member C85-V1 (0.000-0.860)

N60.3/4

Analysis

Steel S235 fyd(applied) = 235 N/mm2

h = 60,3 mm

A = 0,71e-03 m2

Wy;el = 934.4e-08 m3

Wy;pl = 127.0e-07 m3

b = 60,3 mm

Iy = 281.7e-09 m4

Wz;el = 934.4e-08 m3

Wz;pl = 127.0e-07 m3

tf = 4,0 mm

Iz = 281.7e-09 m4

Aw;y;el = 4.50e-04 m2

Aw;y;pl = 4.50e-04 m2

tw = 4,0 mm

Mass/m = 5,6 kg/m

Aw;z;el = 4.50e-04 m2

Aw;z;pl = 4.50e-04 m2

r = 26,1 mm

It = 563.5e-09 m4

Iwa = 0.0e-02 m6

Crosscheck member C85-V1 (0.000-0.860)

Decisive combination: Pe.C.3 at 0,860 m

Section class = 1

Nx;Ed = -1,3 kN

Vy;Ed = 0,0 kN

My;Ed = 0,1 kNm

Vz;Ed = 0,1 kN

Mz;Ed = 0,0 kNm

Nc;Rd = 166,3 kN

Vy;Rd = 61,1 kN

My;Rd = 3,0 kNm

Vz;Rd = 61,1 kN

Mz;Rd = 3,0 kNm

NVy;Rd = 166,3 kN

NVz;Rd = 166,3 kN

MV;y;Rd = 3,0 kNm

MV;z;Rd = 3,0 kNm

$\alpha_1 = 2,000$

$\beta_1 = 1,000$

MNV;y;Rd = 3,0 kNm

q(y) = 1,030

$\alpha_2 = 2,000$

$\beta_2 = 1,000$

MNV;z;Rd = 3,0 kNm

q(z) = 1,030

NEN-EN1993-1-1(NB.33): UC = 0,05 < 1

Lateral Buckling member C85-V1 (0.000-0.860)

Equi. section: N60.3/4

Decisive combination: Pe.C.3

Instab. curve Lat. Buck.: d

Load Attachment Position: 0,000 m from section center

Lateral buckling supports upper flange: None

Lateral buckling supports lower flange: None

Restr. begin: Supported

Restr. end: Supported

b-eff(Begin) = 0,000

b-eff(End) = 0,000

Table used: NB.NB.4

M = 0,1 kN/m

MBeta = 0,0

q = 0,0

Upper flange decisive

Xb;lst = 0,000 m

Xe;lst = 0,860 m

lst = 0,860 m

Lsys = 0,860 m

Lg = 0,860 m

S = 0,000 m

Iwa = 0.0000e+00 m6

C1 = 1,60

C2 = 0,00 (table)

C2(applied) = 0,00

C = 0,00

Mcr = 0,0 kNm

kred = 1.0

Lam-rel = 0,00

Section class 1

Chi;LT(Pe.C.3) = 1,00

M;Ed = 0,1 kNm

UC(y) = 0,00

Chi;LT,Z = 1,00

lkip = 0,860 m

UC(z) = 0,00

My;begin = 0,0 kNm

My;end = 0,1 kNm

NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)

Stability Check C85-V1 (0.000-0.860)

Decisive combination: Pe.C.3

N;Ed = -1,3 kN

Nb;Rd;y = 155,7 kN

Nb;Rd;z = 155,7 kN

Method Y = Cons.Br.

Ca(y) = 0,000

Cb(y) = 0,000

Lbuc Y = 0,860 m

Method Z = Cons.Br.

Ca(z) = N/A

Cb(z) = N/A

Lbuc Z = 0,860 m

Chi;y = 0,94

Buckl. curve: A

Chi;z = 0,94

Buckl. curve: A

NEN-EN1993-1-1(6.46): UC = 0,01 < 1

Bending & Compression C85-V1 (0.000-0.860)

Decisive combination: Pe.C.3

Section class = 1

	Pole 2	
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N;Ed = -1,3 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = 0,1 kNm	My;Psi = 0,0 kNm	My;s = 0,1 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,68	Cmz = 0,59	CmLT = 0,68	
Kyy = 0,685	Kyz = 0,356	Kzy = 0,411	Kzz = 0,593
§12a ;y = 0,94	§12a ;z = 0,94	§12a ;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,04 < 1			

Section data member C86-V1 (0.000-0.600)

N60.3/4	Analysis	Steel S235	f _{yd} (applied) = 235 N/mm ²
h = 60,3 mm	A = 0,71e-03 m ²	Wy;el = 934.4e-08 m ³	Wy;pl = 127.0e-07 m ³
b = 60,3 mm	I _y = 281.7e-09 m ⁴	Wz;el = 934.4e-08 m ³	Wz;pl = 127.0e-07 m ³
tf = 4,0 mm	I _z = 281.7e-09 m ⁴	Aw;y;el = 4.50e-04 m ²	Aw;y;pl = 4.50e-04 m ²
tw = 4,0 mm	Mass/m = 5,6 kg/m	Aw;z;el = 4.50e-04 m ²	Aw;z;pl = 4.50e-04 m ²
r = 26,1 mm		I _t = 563.5e-09 m ⁴	I _{wa} = 0.0e-02 m ⁶

Crosscheck member C86-V1 (0.000-0.600)

Decisive combination: Pe.C.2 at 0,000 m		Section class = 1	
N;Ed = 0,6 kN	Vy;Ed = 0,0 kN	My;Ed = -0,1 kNm	
	Vz;Ed = 0,4 kN	Mz;Ed = 0,0 kNm	
N;Rd = 166,3 kN	Vy;Rd = 61,1 kN	MyRd = 3,0 kNm	
	Vz;Rd = 61,1 kN	MzRd = 3,0 kNm	
NEN-EN1993-1-1(NB.33): UC = 0,05 < 1			

Lateral Buckling member C86-V1 (0.000-0.600)

Equi. section: N60.3/4		Instab. curve Lat. 5.1.2.e	
Decisive combination: Pe.C.3			
Load Attachment Position: 0,000 m from section center			
Lateral buckling supports upper flange: None			
Lateral buckling supports lower flange: None			
Restr. begin: Supported	Restr. end: Supported	b-eff(Begin) = 0,000	b-eff(End) = 0,000
Table used: NB.NB.4	M = -0,1kN/m	MBeta = 0,0	q = 0,0
Upper flange decisive	Xb;lst = 0,000 m	Xe;lst = 0,600 m	lst = 0,600 m
Lsys = 0,600 m	Lg = 0,600 m	S = 0,000 m	Iwa = 0.0000e+00 m6
C1 = 1,88	C2 = 0,00 (table)	C2(applied) = 0,00	C = 0,00
Mcr = 0,0 kNm	kred = 1.0	Lam-rel = 0,00	Section class 1
5.12.a;LT(Pe.C.3) = 1,00	M;Ed = 0,0 kNm		UC(y) = 0,00
5.12.a;LT,Z = 1,00	lkip = 0,600 m		UC(z) = 0,00
My;begin = 0,0 kNm	My;end = -0,1 kNm		
NEN-EN1993-1-1(6.54): UC = 0,00 < 1 Lat.buc.N/A: hollow section NEN-EN 1993-1-1 #6.3.2.1(2)			

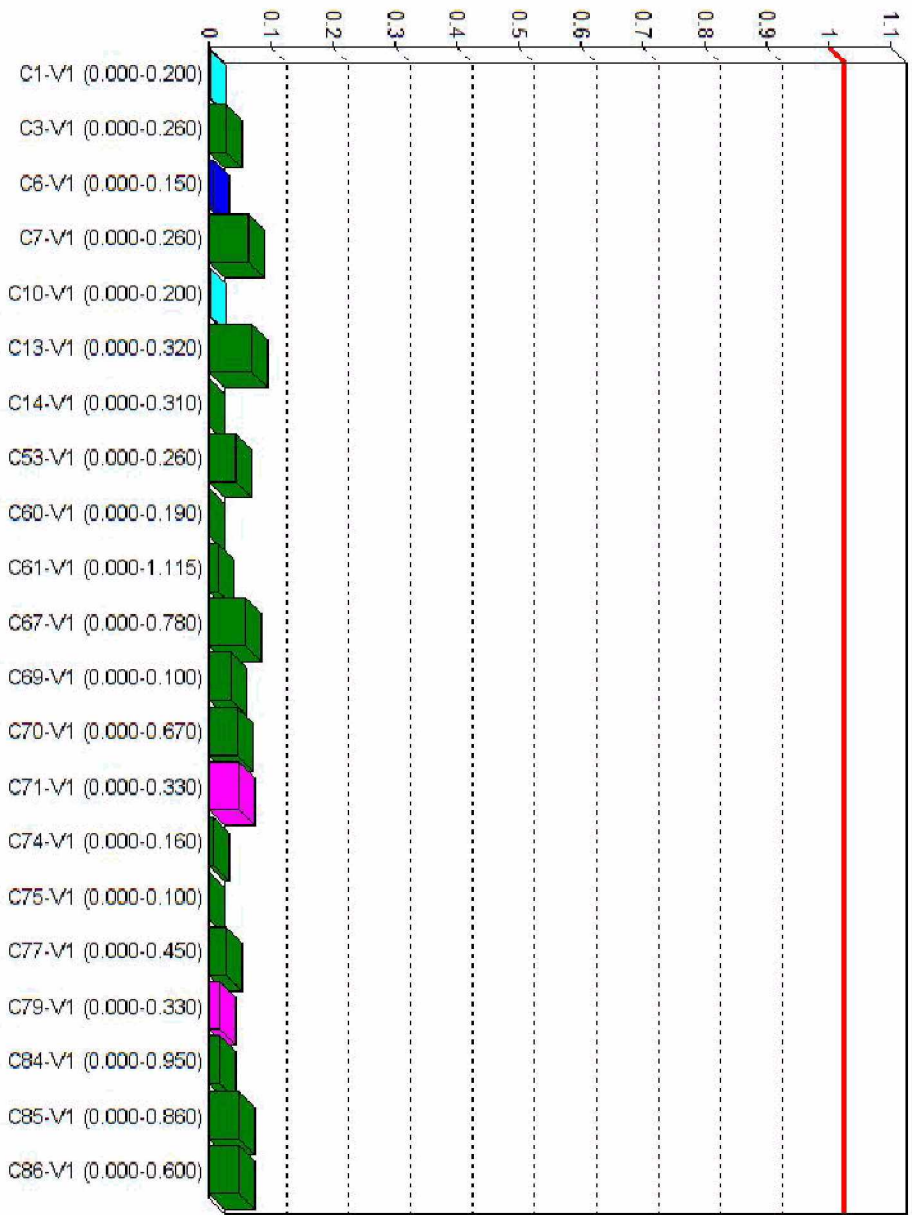
Stability Check C86-V1 (0.000-0.600)

Decisive combination: Pe.C.3			
N;Ed = -0,6 kN	Nb;Rd;y = 161,7 kN	Nb;Rd;z = 161,7 kN	
Method Y = Cons.Br.	Ca(y) = 0,000	Cb(y) = 0,000	L _{buc} Y = 0,600 m
Method Z = Cons.Br.	Ca(z) = N/A	Cb(z) = N/A	L _{buc} Z = 0,600 m
§12a ;y = 0,97		Buckl. curve: A	
§12a ;z = 0,97		Buckl. curve: A	
NEN-EN1993-1-1(6.46): UC = 0,00 < 1			

Bending & Compression C86-V1 (0.000-0.600)

Decisive combination: Pe.C.3		Section class = 1	
N;Ed = -0,6 kN	My;Ed = 0,1 kNm	Mz;Ed = 0,0 kNm	
	Delta;My;Ed = 0,0 kNm	Delta;Mz;Ed = 0,0 kNm	
My = -0,1 kNm	My;Psi = 0,0 kNm	My;s = -0,1 kNm	
Mz = 0,0 kNm	Mz;Psi = 0,0 kNm	Mz;s = 0,0 kNm	
Cmy = 0,57	Cmz = 0,61	CmLT = 0,57	
Kyy = 0,568	Kyz = 0,367	Kzy = 0,341	Kzz = 0,612
§12a ;y = 0,97	§12a ;z = 0,97	§12a ;LT = 1,00	
NEN-EN1993-1-1(6.61&6.62): UC = 0,03 < 1			

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UNITY CHECK NEN-EN1993-1-1:2016/NB:2016

Field	Check Type	Combination	Formula	UC max
C1-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0.00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0.00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0.00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0.00
C1-V1 (0.000-0.200)	Lat Buck	Pe.C.3	NEN-EN1993-1-1(6.54)	0.00
C3-V1 (0.000-0.260)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0.03
C3-V1 (0.000-0.260)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0.00
C3-V1 (0.000-0.260)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0.00
C3-V1 (0.000-0.260)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0.00
C3-V1 (0.000-0.260)	Lat Buck	Pe.C.3	NEN-EN1993-1-1(6.54)	0.02
C6-V1 (0.000-0.150)	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0.01
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.00
C6-V1 (0.000-0.150)	Lat Buck	Pe.C.3	NEN-EN1993-1-1(6.54)	0.01
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.06
C7-V1 (0.000-0.260)	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0.00
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.00
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.00
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0.02
C10-V1 (0.000-0.200)	Lat Buck	Pe.C.3	NEN-EN1993-1-1(6.54)	0.00
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0.00

Field	Check Type	Combination	Formula	UC max
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C10-V1 (0.000-0.200)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C13-V1 (0.000-0.320)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,07
C13-V1 (0.000-0.320)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.320)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.320)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,05
C13-V1 (0.000-0.320)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C14-V1 (0.000-0.310)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C14-V1 (0.000-0.310)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C53-V1 (0.000-0.260)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,04
C53-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C53-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C53-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C53-V1 (0.000-0.260)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C60-V1 (0.000-0.190)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C60-V1 (0.000-0.190)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C61-V1 (0.000-1.115)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,01
C61-V1 (0.000-1.115)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C61-V1 (0.000-1.115)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C61-V1 (0.000-1.115)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C61-V1 (0.000-1.115)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C67-V1 (0.000-0.780)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,06
C67-V1 (0.000-0.780)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,01
C67-V1 (0.000-0.780)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,01
C67-V1 (0.000-0.780)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C67-V1 (0.000-0.780)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C69-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C69-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C70-V1 (0.000-0.670)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,05
C70-V1 (0.000-0.670)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C71-V1 (0.000-0.330)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,05
C71-V1 (0.000-0.330)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C74-V1 (0.000-0.160)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,01
C74-V1 (0.000-0.160)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C74-V1 (0.000-0.160)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C74-V1 (0.000-0.160)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C74-V1 (0.000-0.160)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C75-V1 (0.000-0.100)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C75-V1 (0.000-0.100)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C75-V1 (0.000-0.100)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C75-V1 (0.000-0.100)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C75-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C77-V1 (0.000-0.450)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C77-V1 (0.000-0.450)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C77-V1 (0.000-0.450)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C77-V1 (0.000-0.450)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,01
C77-V1 (0.000-0.450)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C79-V1 (0.000-0.330)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,02
C79-V1 (0.000-0.330)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C84-V1 (0.000-0.950)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C84-V1 (0.000-0.950)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C84-V1 (0.000-0.950)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00

	Pole 2	
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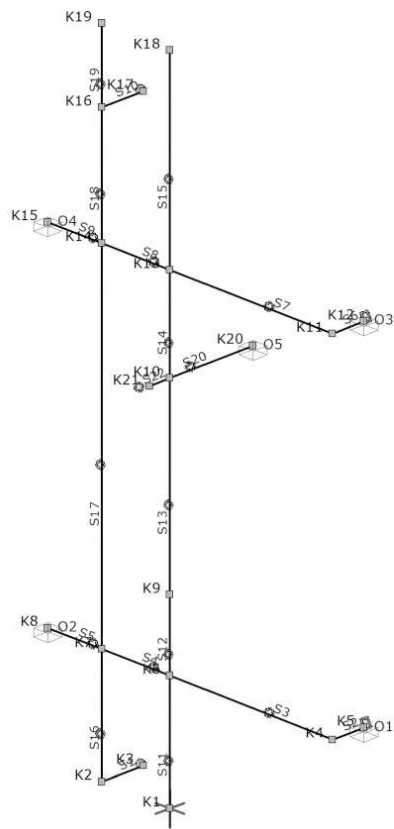
Field	Check Type	Combination	Formula	UC max
C84-V1 (0.000-0.950)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C84-V1 (0.000-0.950)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C85-V1 (0.000-0.860)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,05
C85-V1 (0.000-0.860)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,01
C85-V1 (0.000-0.860)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,01
C85-V1 (0.000-0.860)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,04
C85-V1 (0.000-0.860)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C86-V1 (0.000-0.600)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,05
C86-V1 (0.000-0.600)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C86-V1 (0.000-0.600)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C86-V1 (0.000-0.600)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C86-V1 (0.000-0.600)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

STEEL CONSTRUCTION WEIGHT

Member	Section	Lsys	Mass
C71-V1 (0.000-0.330)	B48.3x3.2	0,330	1,175
C79-V1 (0.000-0.330)	B48.3x3.2	0,330	1,175
SubTotal:	B48.3x3.2	0,660	2,349
C10-V1 (0.000-0.200)	C100	0,200	12,331
C1-V1 (0.000-0.200)	C100	0,200	12,331
SubTotal:	C100	0,400	24,662
C6-V1 (0.000-0.150)	K60x60x4x4	0,150	1,055
SubTotal:	K60x60x4x4	0,150	1,055
C13-V1 (0.000-0.320)	N60.3/4	0,320	1,777
C14-V1 (0.000-0.310)	N60.3/4	0,310	1,722
C3-V1 (0.000-0.260)	N60.3/4	0,260	1,444
C53-V1 (0.000-0.260)	N60.3/4	0,260	1,444
C60-V1 (0.000-0.190)	N60.3/4	0,190	1,055
C61-V1 (0.000-1.115)	N60.3/4	1,115	6,192
C67-V1 (0.000-0.780)	N60.3/4	0,780	4,332
C69-V1 (0.000-0.100)	N60.3/4	0,100	0,555
C70-V1 (0.000-0.670)	N60.3/4	0,670	3,721
C74-V1 (0.000-0.160)	N60.3/4	0,160	0,889
C75-V1 (0.000-0.100)	N60.3/4	0,100	0,555
C77-V1 (0.000-0.450)	N60.3/4	0,450	2,499
C7-V1 (0.000-0.260)	N60.3/4	0,260	1,444
C84-V1 (0.000-0.950)	N60.3/4	0,950	5,276
C85-V1 (0.000-0.860)	N60.3/4	0,860	4,776
C86-V1 (0.000-0.600)	N60.3/4	0,600	3,332
SubTotal:	N60.3/4	7,385	41,015
Total:		8,595 m	69,080 kg

Pole 3			
Project name	L025673	Project number	
Part description	Pole 3	Structural engineer	5.1.2.e
Client		Units	5.1.2.e 5.1.2.e 5.1.2.e

PIC. GEOMETRIE RAAMWERK



MEMBERS

Member	Node B	Node E	X-B	Y-B	Z-B	X-E	Y-E	Z-E	Length	Section	Pos
S1	K3	K2	0,200	-0,330	0,000	0,000	-0,330	0,000	0,200	P3	0,000 - L(0,
S2	K5	K4	0,150	0,785	-0,490	0,000	0,785	-0,490	0,150	P2	0,000 - L(0,
S3	K4	K6	0,000	0,785	-0,490	0,000	0,000	-0,490	0,785	P1	0,000 - L(0,
S4	K6	K7	0,000	0,000	-0,490	0,000	-0,330	-0,490	0,330	P1	0,000 - L(0,
S5	K7	K8	0,000	-0,330	-0,490	0,000	-0,590	-0,490	0,260	P1	0,000 - L(0,
S6	K12	K11	0,150	0,785	-1,990	0,000	0,785	-1,990	0,150	P2	0,000 - L(0,
S7	K11	K13	0,000	0,785	-1,990	0,000	0,000	-1,990	0,785	P1	0,000 - L(0,
S8	K13	K14	0,000	0,000	-1,990	0,000	-0,330	-1,990	0,330	P1	0,000 - L(0,
S9	K14	K15	0,000	-0,330	-1,990	0,000	-0,590	-1,990	0,260	P1	0,000 - L(0,
S10	K17	K16	0,200	-0,330	-2,490	0,000	-0,330	-2,490	0,200	P3	0,000 - L(0,
S11	K1	K6	0,000	0,000	0,000	0,000	0,000	-0,490	0,490	P1	0,000 - L(0,
S12	K6	K9	0,000	0,000	-0,490	0,000	0,000	-0,790	0,300	P1	0,000 - L(0,
S13	K9	K10	0,000	0,000	-0,790	0,000	0,000	-1,590	0,800	P1	0,000 - L(0,
S14	K10	K13	0,000	0,000	-1,590	0,000	0,000	-1,990	0,400	P1	0,000 - L(0,
S15	K13	K18	0,000	0,000	-1,990	0,000	0,000	-2,800	0,810	P1	0,000 - L(0,
S16	K2	K7	0,000	-0,330	0,000	0,000	-0,330	-0,490	0,490	P1	0,000 - L(0,
S17	K7	K14	0,000	-0,330	-0,490	0,000	-0,330	-1,990	1,500	P1	0,000 - L(1,
S18	K14	K16	0,000	-0,330	-1,990	0,000	-0,330	-2,490	0,500	P1	0,000 - L(0,
S19	K16	K19	0,000	-0,330	-2,490	0,000	-0,330	-2,800	0,310	P1	0,000 - L(0,
S20	K10	K20	0,000	0,000	-1,590	0,400	0,000	-1,590	0,400	P1	0,000 - L(0,
S22	K21	K10	-0,100	0,000	-1,590	0,000	0,000	-1,590	0,100	P1	0,000 - L(0,
-	-	-	m	m	m	m	m	m	m	-	

SECTIONS

Section	Section Name	Area	It	Iy	Iz	Material	Angle
P1	N60.3/4	7.0749e-04	5.6346e-07	2.8173e-07	2.8173e-07	S235H(EN10210-1)	0,0

	Pole 3	
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Section	Section Name	Area	It	Iy	Iz Material	Angle
P2	K60x60x4x4	8.9600e-04	7.0246e-07	4.7070e-07	4.7070e-07 S235	0,0
P3	C100	7.8540e-03	9.8175e-06	4.9087e-06	4.9087e-06 S235	0,0
-	-	m2	m4	m4	m4 -	°

SECTION SHAPES

Section	Tapered	hB	hE	tf	tw	tf2	B	b1	b2 Castellate	Height
P2	No	0,060	0,060	0,0040	0,0040	0,0000	0,060	0,000	0,000 No	0,000
P3	No	0,100	0,100	0,0000	0,0000	0,0000	0,100	0,000	0,000 No	0,000
-	-	m	m	m	m	m	m	m	m -	m

MATERIALS

Material Name	Poison	Density	Youngs mod.	Exp.
S235H(EN10210-1)	0.30	78.50	2.1000e+08	12.0000e-06
S235	0.30	78.50	2.1000e+08	12.0000e-06
-	-	kN/m3	kN/m2	C°m

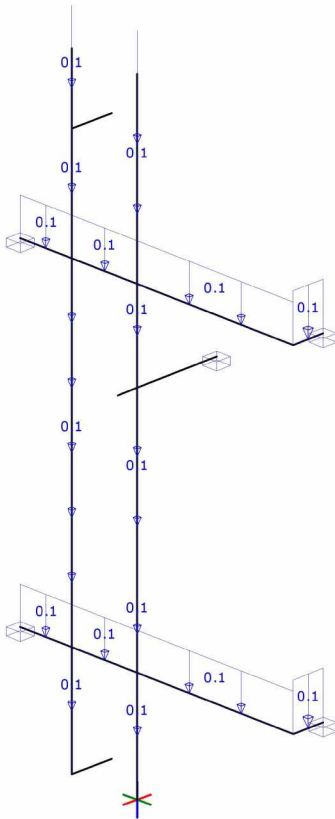
SUPPORTS

Support	Object	Position	X	Y	Z	Xr	Yr	Zr	AngleXr	AngleYr	AngleZr
O1	K5	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O2	K8	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O3	K12	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O4	K15	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O5	K20	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
-	-	m	kN/m	kN/m	kN/m	kNm/rad	kNm/rad	kNm/rad	°	°	°

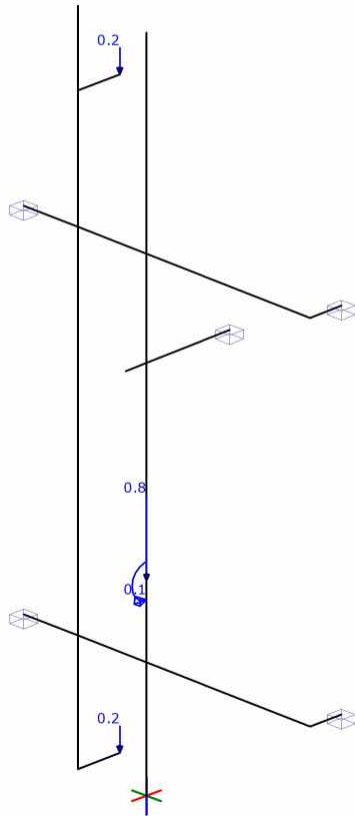
LOADS CASES TYPES

Label	Description	L.C. Type	Fav./Unfav.	Element	Level	Field	Psi0	Psi1	Psi2	Cprob ULS/SLS
B.G.1	Permanent actions	Permanent actions	-		None	None				
B.G.2	Permanent actions	Permanent actions	-		None	None				
B.G.3	wind x	Wind load	-		None	None	0.20			1,00/1,00

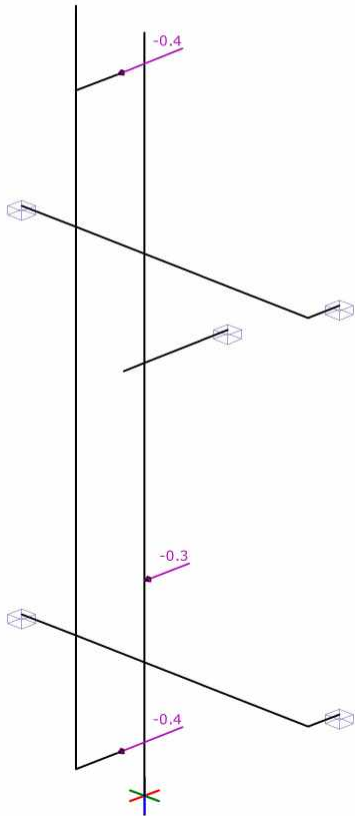
PIC. LASTEN B.G.1 PERMANENT ACTIONS



PIC. LASTEN B.G.2 PERMANENT ACTIONS



PIC. LASTEN B.G.3 WIND X



PERSISTENT LOADS COMBINATIONS (TABLE)

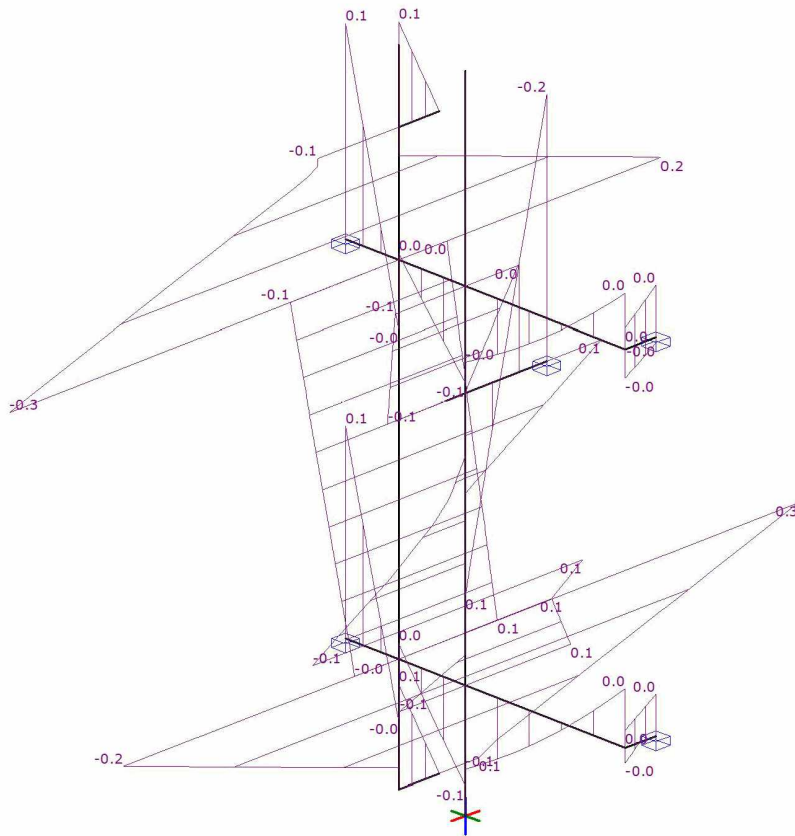
L.C.	Description	Pe.C.1	Pe.C.2	Pe.C.3
B.G.1	Permanent actions	1.35	1.20	0.90
B.G.2	Permanent actions	1.35	1.20	0.90
B.G.3	wind x	0.00	1.27	-1.27

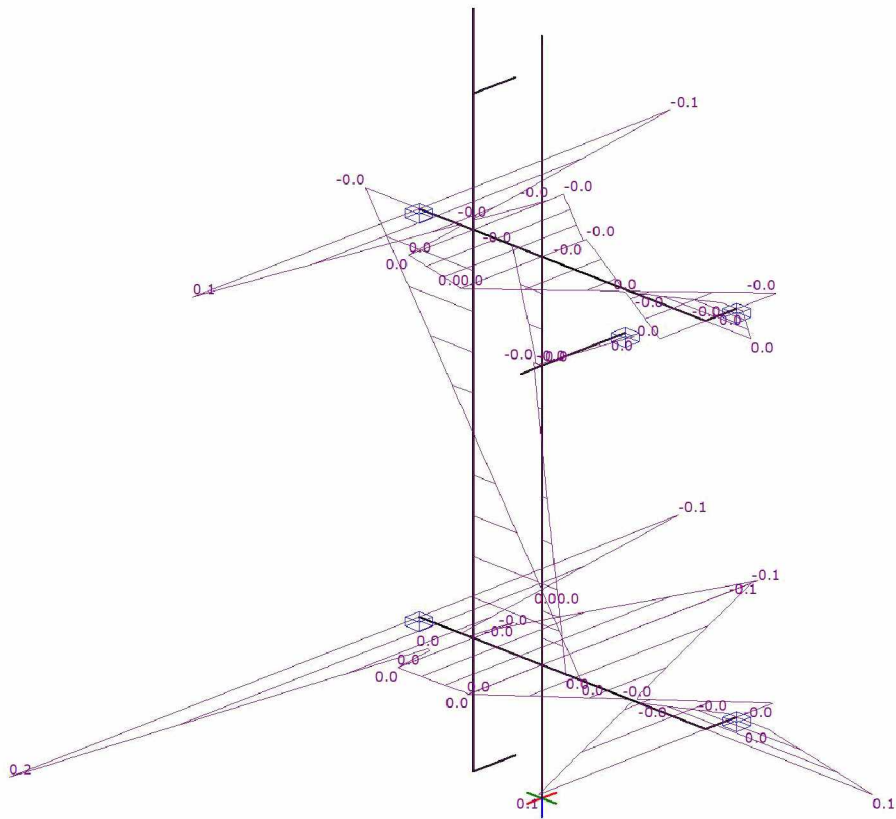
CHARACTERISTIC LOADS COMBINATIONS (TABLE)

L.C.	Description	Ch.C.(w1)	Ch.C.1
B.G.1	Permanent actions	1.00	1.00
B.G.2	Permanent actions	1.00	1.00
B.G.3	wind x	0.00	0.57

ANALYSIS ASSUMPTIONS

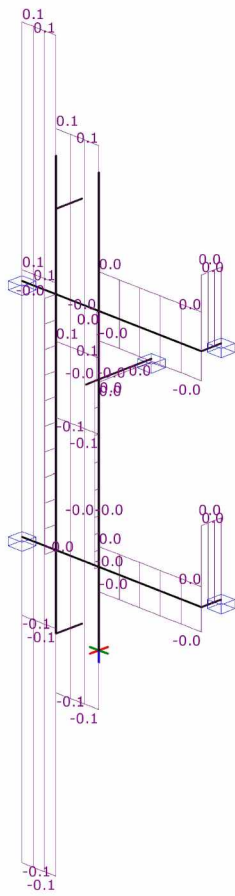
Linear Elastic Analysis performed

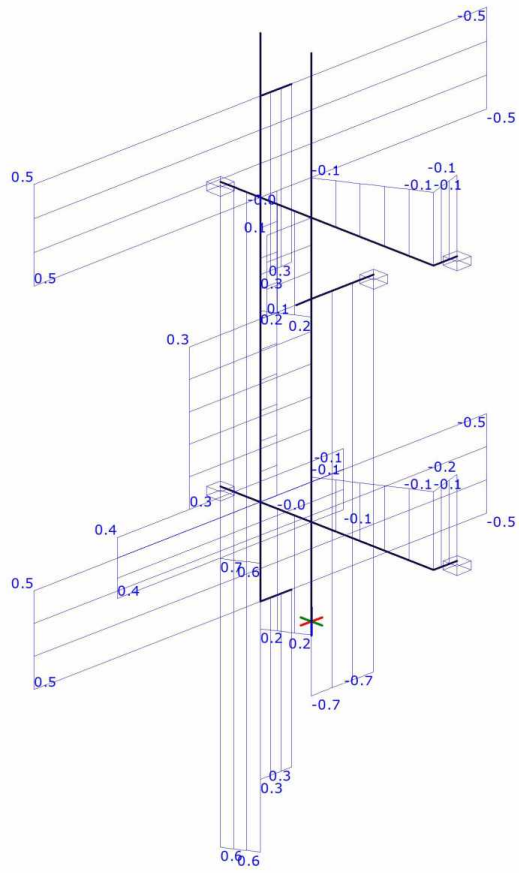


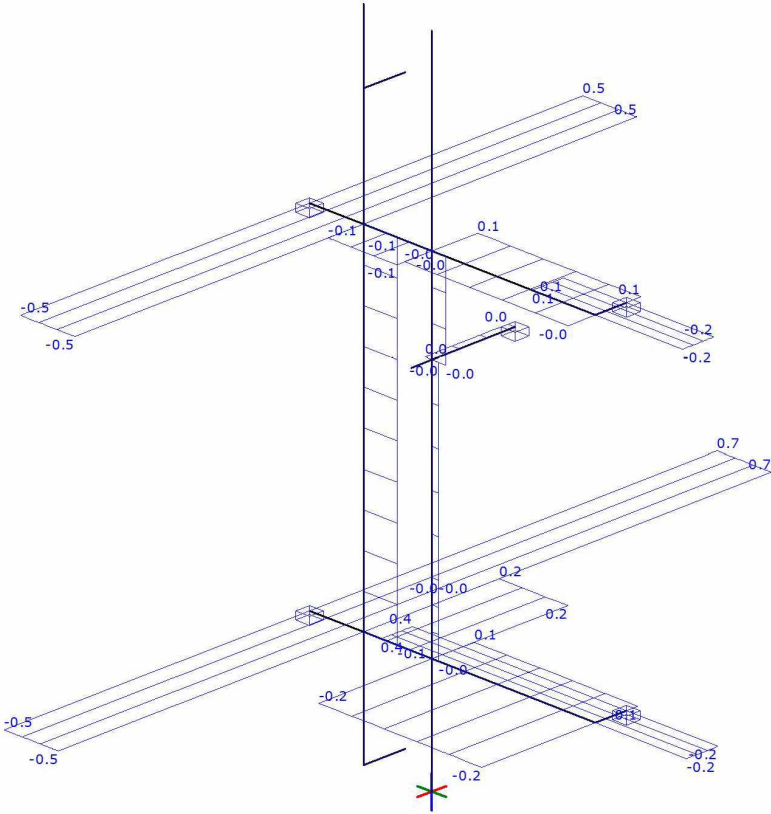


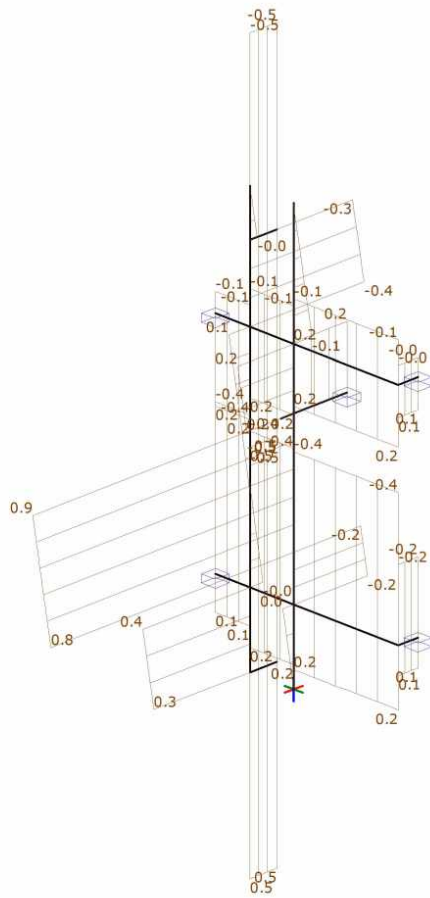
PIC. PE.C. MOMENT (MX) ENVELOPE

Persistent Loads Combinations







**PE.C. MEMBER FORCES (MY, MZ)**

Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S1	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.04	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
S2	Pe.C.1	My	0.02			0.00	0.000	0.000
		Mz	0.01			0.03	0.000	0.000
	Pe.C.2	My	0.01			-0.01	0.079	0.000
		Mz	0.00			-0.03	0.000	0.000
	Pe.C.3	My	0.03			0.01	0.000	0.000
		Mz	0.01			0.07	0.000	0.000
S3	Pe.C.1	My	0.04			-0.05	0.279	0.000
		Mz	0.03			-0.04	0.333	0.000
	Pe.C.2	My	0.03			-0.05	0.279	0.000
		Mz	-0.03			0.03	0.372	0.000
	Pe.C.3	My	0.02			-0.03	0.277	0.000
		Mz	0.07			-0.09	0.348	0.000
S4	Pe.C.1	My	-0.06			0.01	0.287	0.000
		Mz	-0.04			0.01	0.245	0.000
	Pe.C.2	My	-0.05			0.01	0.286	0.000
		Mz	0.03			0.03	0.000	0.000
	Pe.C.3	My	-0.04			0.01	0.288	0.000
		Mz	-0.09			-0.01	0.000	0.000
S5	Pe.C.1	My	-0.04			0.13	0.059	0.000
		Mz	0.01			0.04	0.000	0.000
	Pe.C.2	My	-0.03			0.11	0.058	0.000

	Pole 3					
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Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S6	Pe.C.3	Mz	0.03			-0.11	0.058	0.000
		My	-0.02			0.08	0.059	0.000
		Mz	-0.02			0.17	0.023	0.000
	Pe.C.1	My	0.02			0.00	0.129	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.2	My	0.03			0.01	0.000	0.000
S7	Pe.C.3	Mz	-0.01			-0.03	0.000	0.000
		My	0.00			-0.02	0.000	0.000
		Mz	0.00			0.02	0.000	0.000
	Pe.C.1	My	0.03			-0.05	0.282	0.000
		Mz	-0.01			0.01	0.319	0.000
	Pe.C.2	My	0.03			-0.04	0.280	0.000
S8	Pe.C.3	Mz	-0.03			0.03	0.364	0.000
		My	0.02			-0.03	0.283	0.000
		Mz	0.02			-0.02	0.395	0.000
	Pe.C.1	My	-0.06			0.00	0.308	0.000
		Mz	0.01			-0.01	0.196	0.000
	Pe.C.2	My	-0.05			0.00	0.308	0.000
S9	Pe.C.3	Mz	0.03			0.03	0.000	0.000
		My	-0.04			0.00	0.309	0.000
		Mz	-0.02			-0.04	0.000	0.000
	Pe.C.1	My	-0.04			0.13	0.064	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.2	My	-0.04			0.12	0.064	0.000
S10	Pe.C.3	Mz	0.03			-0.11	0.053	0.000
		My	-0.03			0.09	0.064	0.000
		Mz	-0.03			0.10	0.066	0.000
	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000
S11	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.04	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.1	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.00	0.000	0.000
S12	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.1	My	0.02			0.09	0.000	0.000
		Mz	0.01			0.01	0.000	0.000
	Pe.C.2	My	0.08			0.06	0.000	0.000
S13	Pe.C.3	Mz	0.01			0.01	0.000	0.000
		My	-0.05			0.08	0.116	0.000
		Mz	0.01			0.00	0.000	0.000
	Pe.C.1	My	-0.11			0.08	0.459	0.000
		Mz	0.01			0.00	0.515	0.000
	Pe.C.2	My	-0.12			0.11	0.423	0.000
S14	Pe.C.3	Mz	0.01			0.00	0.501	0.000
		My	-0.05			0.02	0.568	0.000
		Mz	0.00			0.00	0.536	0.000
	Pe.C.1	My	-0.04			-0.01	0.000	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.2	My	0.00			0.04	0.003	0.000
S15	Pe.C.3	Mz	0.00			-0.01	0.000	0.000
		My	-0.06			-0.06	0.000	0.000
		Mz	0.00			-0.01	0.000	0.000
	Pe.C.1	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.00	0.000	0.000
S16	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.1	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.00	0.000	0.000

			Pole 3					
Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S17	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.06			0.31	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.04			-0.21	0.081	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.03			-0.03	0.724	0.000
	Pe.C.1	Mz	0.05			-0.05	0.754	0.000
		My	0.08			0.04	0.000	0.000
		Mz	0.04			-0.04	0.754	0.000
	Pe.C.3	My	-0.03			-0.08	0.000	0.000
Mz		0.03			-0.03	0.754	0.000	
S18		Pe.C.1	My	-0.06			-0.06	0.000
	Mz		0.00			0.00	0.000	0.000
	My		0.21			-0.06	0.393	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	-0.30			-0.04	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
My		0.00			0.00	0.000	0.000	
Mz		0.00			0.00	0.000	0.000	
S19	Pe.C.2	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.1	My	0.12			-0.16	0.173	0.000
		Mz	0.00			0.00	0.105	0.000
		My	0.11			-0.14	0.172	0.000
	Pe.C.2	Mz	0.00			0.00	0.093	0.000
My		0.08			-0.11	0.174	0.000	
Mz		0.00			0.00	0.118	0.000	
S20	Pe.C.1	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.00	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
My		0.00			0.00	0.000	0.000	
Mz		0.00			0.00	0.000	0.000	
			kNm	kNm	m	kNm	m	m

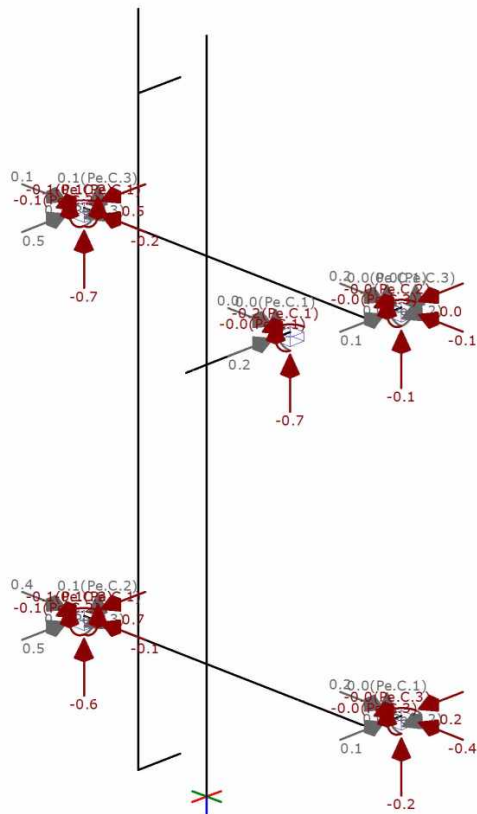
PE.C. MEMBER FORCES (NX, VY, VZ, MX)

Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S1	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.52 Vz	0.28	0.28	0.28	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.52 Vz	0.21	0.21	0.21	0.00	0.00
			Vy	0.00	0.00	0.00		
S2	Pe.C.1	C	-0.08 Vz	-0.15	-0.15	-0.14	0.04	0.04
			Vy	0.14	0.14	0.14		
	Pe.C.2	T	0.08 Vz	-0.14	-0.14	-0.12	0.03	0.03
			Vy	-0.16	-0.16	-0.16		
	Pe.C.3	C	-0.20 Vz	-0.10	-0.10	-0.09	0.02	0.02
			Vy	0.39	0.39	0.39		
S3	Pe.C.1	C	-0.14 Vz	-0.14	-0.14	-0.08	0.00	0.00
			Vy	-0.08	-0.08	-0.08		
	Pe.C.2	T	0.16 Vz	-0.12	-0.12	-0.07	0.01	0.01
			Vy	0.08	0.08	0.08		
	Pe.C.3	C	-0.39 Vz	-0.09	-0.09	-0.05	-0.01	-0.01
			Vy	-0.20	-0.20	-0.20		
S4	Pe.C.1	C	-0.16 Vz	0.20	0.23	0.23	0.02	0.02
			Vy	0.16	0.16	0.16		
	Pe.C.2	T	0.15 Vz	0.18	0.20	0.20	0.09	0.09
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.39 Vz	0.13	0.15	0.15	-0.06	-0.06
			Vy	0.24	0.24	0.24		
S5	Pe.C.1	C	-0.22 Vz	0.62	0.64	0.64	-0.02	-0.02
			Vy	0.12	0.12	0.12		

	Pole 3		
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Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S5	Pe.C.2	T	0.10 Vz	0.55	0.57	0.57	-0.14	-0.14
			Vy	-0.55	-0.55	-0.55		
	Pe.C.3	C	-0.43 Vz	0.41	0.43	0.43	0.12	0.12
S6	Pe.C.1	T	Vy	0.73	0.73	0.73		
			0.02 Vz	-0.14	-0.14	-0.13	0.03	0.03
	Pe.C.2	T	Vy	-0.03	-0.03	-0.03		
			0.08 Vz	-0.13	-0.13	-0.11	0.03	0.03
	Pe.C.3	C	Vy	-0.16	-0.16	-0.16		
			-0.05 Vz	-0.10	-0.10	-0.09	0.02	0.02
S7	Pe.C.1	T	Vy	0.11	0.11	0.11		
			0.03 Vz	-0.13	-0.13	-0.07	0.00	0.00
	Pe.C.2	T	Vy	0.02	0.02	0.02		
			0.16 Vz	-0.11	-0.11	-0.06	-0.01	-0.01
	Pe.C.3	C	Vy	0.08	0.08	0.08		
			-0.11 Vz	-0.09	-0.09	-0.05	0.02	0.02
S8	Pe.C.1	T	Vy	-0.05	-0.05	-0.05		
			0.05 Vz	0.18	0.20	0.20	0.01	0.01
	Pe.C.2	T	Vy	-0.05	-0.05	-0.05		
			0.18 Vz	0.16	0.18	0.18	-0.05	-0.05
	Pe.C.3	C	Vy	-0.02	-0.02	-0.02		
			-0.10 Vz	0.12	0.13	0.13	0.07	0.07
S9	Pe.C.1	T	Vy	-0.06	-0.06	-0.06		
			0.11 Vz	0.65	0.67	0.67	-0.02	-0.02
	Pe.C.2	T	Vy	-0.02	-0.02	-0.02		
			0.23 Vz	0.58	0.59	0.59	0.11	0.11
	Pe.C.3	C	Vy	-0.52	-0.52	-0.52		
			-0.06 Vz	0.43	0.44	0.44	-0.15	-0.15
S10	Pe.C.1	-	Vy	0.49	0.49	0.49		
			0.00 Vz	0.32	0.32	0.32	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.52 Vz	0.28	0.28	0.28	0.00	0.00
	Pe.C.3	T	Vy	0.00	0.00	0.00		
			0.52 Vz	0.21	0.21	0.21	0.00	0.00
S11	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.04 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	T	Vy	0.00	0.00	0.00		
			0.03 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.3	T	Vy	0.00	0.00	0.00		
			0.02 Vz	0.00	0.00	0.00	0.00	0.00
S12	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.24 Vz	0.24	0.24	0.24	0.00	0.00
	Pe.C.2	C	Vy	-0.01	-0.01	-0.01		
			-0.22 Vz	-0.07	-0.07	-0.07	0.00	0.00
	Pe.C.3	C	Vy	-0.01	-0.01	-0.01		
			-0.16 Vz	0.45	0.45	0.45	0.00	0.00
S13	Pe.C.1	T	Vy	-0.01	-0.01	-0.01		
			0.86 Vz	0.24	0.24	0.24	0.00	0.00
	Pe.C.2	T	Vy	-0.01	-0.01	-0.01		
			0.76 Vz	0.28	0.28	0.28	0.00	0.00
	Pe.C.3	T	Vy	-0.01	-0.01	-0.01		
			0.58 Vz	0.09	0.09	0.09	0.00	0.00
S14	Pe.C.1	T	Vy	-0.01	-0.01	-0.01		
			0.19 Vz	0.08	0.08	0.08	0.00	0.00
	Pe.C.2	T	Vy	-0.03	-0.03	-0.03		
			0.16 Vz	0.10	0.10	0.10	0.00	0.00
	Pe.C.3	T	Vy	-0.02	-0.02	-0.02		
			0.12 Vz	0.01	0.01	0.01	0.00	0.00
S15	Pe.C.1	C	Vy	-0.02	-0.02	-0.02		
			-0.06 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.05 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.3	C	Vy	0.00	0.00	0.00		
			-0.04 Vz	0.00	0.00	0.00	0.00	0.00
S16	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.35 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	T	0.31 Vz	0.52	0.52	0.52	0.00	0.00

			Pole 3					
Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S17	Pe.C.3	T	Vy	0.00	0.00	0.00		
			0.24 Vz	-0.52	-0.52	-0.52	0.00	0.00
	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.07 Vz	-0.04	-0.04	-0.04	0.00	0.00
	Pe.C.2	T	Vy	-0.06	-0.06	-0.06		
			0.06 Vz	-0.03	-0.03	-0.03	0.00	0.00
S18	Pe.C.3	T	Vy	-0.05	-0.05	-0.05		
			0.04 Vz	-0.03	-0.03	-0.03	-0.01	-0.01
	Pe.C.1	C	Vy	-0.04	-0.04	-0.04		
			-0.38 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.33 Vz	-0.52	-0.52	-0.52	0.00	0.00
S19	Pe.C.3	C	Vy	0.00	0.00	0.00		
			-0.25 Vz	0.52	0.52	0.52	0.00	0.00
	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.02 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.02 Vz	0.00	0.00	0.00	0.00	0.00
S20	Pe.C.3	C	Vy	0.00	0.00	0.00		
			-0.02 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.17 Vz	-0.70	-0.70	-0.70	0.00	0.00
	Pe.C.2	T	Vy	0.01	0.01	0.01		
			0.18 Vz	-0.62	-0.62	-0.62	0.00	0.00
S22	Pe.C.3	T	Vy	0.01	0.01	0.01		
			0.08 Vz	-0.47	-0.47	-0.47	0.00	0.00
	Pe.C.1	-	Vy	0.01	0.01	0.01		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	-	Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
-	-	-	Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
-	-	-	kN -	kN	kN	kN	kNm	kNm



PE.C. SUPPORT REACTIONS

LComb	Support	Node	X	Y	Z	Mx	My	Mz
Pe.C.1	O1	K5	-0.08	-0.14	-0.15	0.04	-0.02	-0.01
Pe.C.1	O2	K8	-0.12	0.22	-0.64	-0.13	0.02	0.04
Pe.C.1	O3	K12	0.02	0.03	-0.14	0.03	-0.02	0.00
Pe.C.1	O4	K15	0.02	-0.11	-0.67	-0.13	0.02	-0.01
Pe.C.1	O5	K20	0.17	0.01	-0.70	0.00	-0.16	0.00
Sum Reactions			0.00	0.00	-2.31			
Sum Loads			0.00	0.00	2.31			
Pe.C.2	O1	K5	0.08	0.16	-0.14	0.03	-0.01	0.00
Pe.C.2	O2	K8	0.55	-0.10	-0.57	-0.11	0.14	-0.11
Pe.C.2	O3	K12	0.08	0.16	-0.13	0.03	-0.03	0.01
Pe.C.2	O4	K15	0.52	-0.23	-0.59	-0.12	-0.11	-0.11
Pe.C.2	O5	K20	0.18	0.01	-0.62	0.00	-0.14	0.00
Sum Reactions			1.40	0.00	-2.05			
Sum Loads			-1.40	0.00	2.05			
Pe.C.3	O1	K5	-0.20	-0.39	-0.10	0.02	-0.03	-0.01
Pe.C.3	O2	K8	-0.73	0.43	-0.43	-0.08	-0.12	0.17
Pe.C.3	O3	K12	-0.05	-0.11	-0.10	0.02	0.00	0.00
Pe.C.3	O4	K15	-0.49	0.06	-0.44	-0.09	0.15	0.10
Pe.C.3	O5	K20	0.08	0.01	-0.47	0.00	-0.11	0.00
Sum Reactions			-1.40	0.00	-1.54			
Sum Loads			1.40	0.00	1.54			
-	-	-	kN	kN	kN	kNm	kNm	kNm

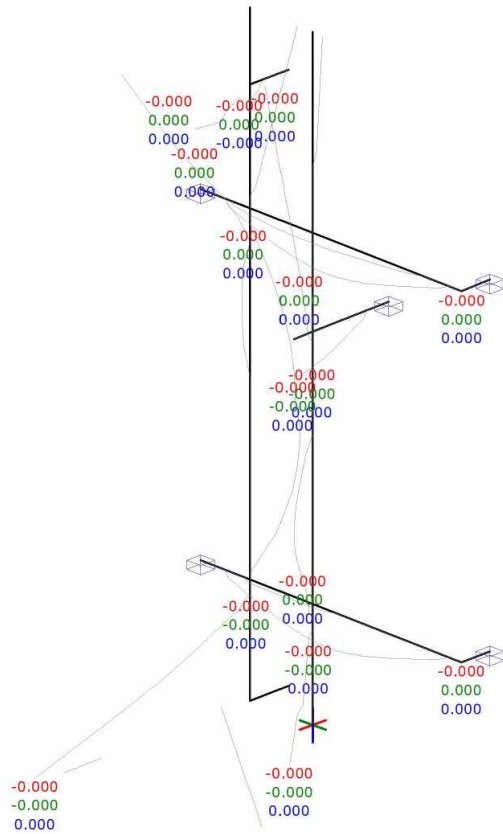
L.C. SUPPORT REACTIONS

L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O1	K5	0.00	0.00	-0.05	0.01	-0.01	0.00
B.G.1	O2	K8	0.00	0.02	-0.13	-0.03	0.00	0.00
B.G.1	O3	K12	0.01	0.01	-0.05	0.01	-0.01	0.00

Pole 3								
L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O4	K15	0.02	-0.03	-0.14	-0.03	0.00	0.00
B.G.1	O5	K20	-0.02	0.00	-0.11	0.00	-0.03	0.00
	Sum Reactions		0.00	0.00	-0.48			
	Sum Loads		0.00	0.00	0.48			
B.G.2	O1	K5	-0.06	-0.10	-0.06	0.02	-0.01	0.00
B.G.2	O2	K8	-0.08	0.14	-0.34	-0.07	0.01	0.03
B.G.2	O3	K12	0.01	0.01	-0.06	0.02	-0.01	0.00
B.G.2	O4	K15	0.00	-0.05	-0.36	-0.07	0.02	0.00
B.G.2	O5	K20	0.14	0.01	-0.41	0.00	-0.09	0.00
	Sum Reactions		0.00	0.00	-1.22			
	Sum Loads		0.00	0.00	1.22			
B.G.3	O1	K5	0.12	0.23	0.00	0.00	0.01	0.01
B.G.3	O2	K8	0.51	-0.23	0.00	0.00	0.10	-0.11
B.G.3	O3	K12	0.05	0.10	0.00	0.00	-0.01	0.00
B.G.3	O4	K15	0.39	-0.10	0.00	0.00	-0.10	-0.08
B.G.3	O5	K20	0.03	0.00	0.00	0.00	0.00	0.00
	Sum Reactions		1.10	0.00	0.00			
	Sum Loads		-1.10	0.00	0.00			
-	-	-	kN	kN	kN	kNm	kNm	kNm

PIC. CH.C. VERPLAATSINGEN ENVELOPE

Characteristic Loads Combinations



CH.C. NODAL DISPLACEMENTS

Node	LComb	X	Y	Z	Xr	Yr	Zr
K1	Ch.C.(w1)	0.0001	0.0000	0.0001	-0.038e-03	0.028e-03	0.029e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.038e-03	-0.056e-03	0.015e-03
K2	Ch.C.(w1)	-0.0001	-0.0001	0.0000	-0.146e-03	-0.459e-03	0.082e-03
	Ch.C.1	-0.0005	-0.0001	0.0000	-0.147e-03	-1.260e-03	-0.039e-03
K3	Ch.C.(w1)	-0.0001	-0.0001	0.0001	-0.146e-03	-0.463e-03	0.082e-03
	Ch.C.1	-0.0005	-0.0001	0.0003	-0.147e-03	-1.265e-03	-0.039e-03

		Pole 3				
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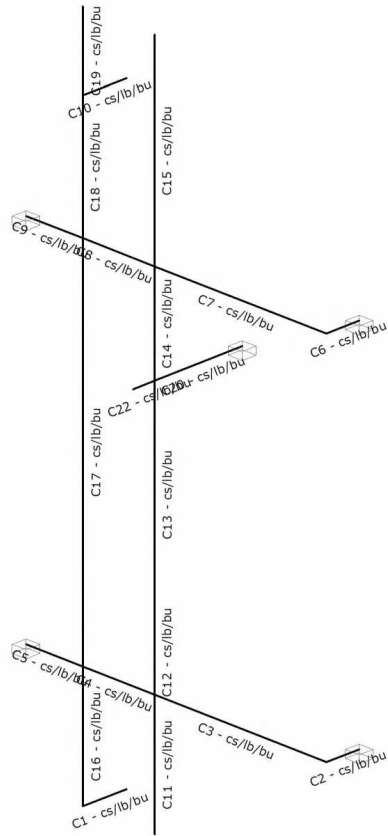
Node	LComb	X	Y	Z	Xr	Yr	Zr
K4	Ch.C.(w1)	0.0000	0.0000	0.0000	0.070e-03	0.013e-03	-0.019e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.071e-03	0.007e-03	0.002e-03
K5	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K6	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.038e-03	0.028e-03	0.029e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.038e-03	-0.056e-03	0.015e-03
K7	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.146e-03	-0.071e-03	0.082e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.147e-03	-0.398e-03	-0.039e-03
K8	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K9	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.006e-03	0.238e-03	0.020e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.006e-03	0.203e-03	0.010e-03
K10	Ch.C.(w1)	0.0000	0.0000	0.0001	0.009e-03	0.095e-03	-0.006e-03
	Ch.C.1	0.0000	0.0000	0.0001	0.009e-03	0.097e-03	-0.006e-03
K11	Ch.C.(w1)	0.0000	0.0000	0.0000	0.066e-03	0.008e-03	0.005e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.065e-03	0.019e-03	0.015e-03
K12	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K13	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.029e-03	-0.026e-03	-0.010e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.030e-03	0.102e-03	-0.006e-03
K14	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.142e-03	-0.090e-03	-0.017e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.142e-03	0.248e-03	-0.086e-03
K15	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K16	Ch.C.(w1)	0.0001	0.0001	0.0000	-0.142e-03	-0.486e-03	-0.017e-03
	Ch.C.1	-0.0002	0.0001	0.0000	-0.142e-03	0.346e-03	-0.086e-03
K17	Ch.C.(w1)	0.0001	0.0001	0.0001	-0.142e-03	-0.490e-03	-0.017e-03
	Ch.C.1	-0.0002	0.0001	0.0000	-0.142e-03	0.342e-03	-0.086e-03
K18	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.029e-03	-0.026e-03	-0.010e-03
	Ch.C.1	-0.0001	0.0000	0.0001	-0.030e-03	0.102e-03	-0.006e-03
K19	Ch.C.(w1)	0.0003	0.0001	0.0000	-0.142e-03	-0.486e-03	-0.017e-03
	Ch.C.1	-0.0003	0.0001	0.0000	-0.142e-03	0.346e-03	-0.086e-03
K20	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K21	Ch.C.(w1)	0.0000	0.0000	0.0001	0.009e-03	0.095e-03	-0.006e-03
	Ch.C.1	0.0000	0.0000	0.0001	0.009e-03	0.097e-03	-0.006e-03
-	-	m	m	m	rad	rad	rad

CH.C. EXTREME DEFLECTIONS

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S3	Ch.C. (w1)	0,000	0,000	0,000	0.531	0.0000	0.567	0.0000	0,000	0,000	0,000
S3	Ch.C.1	0,000	0,000	0,000	0.531	0.0000	0.275	0.0000	0,000	0,000	0,000
S4	Ch.C. (w1)	0,000	0,000	0,000	0.133	0.0000	0.121	0.0000	0,000	0,000	0,000
S4	Ch.C.1	0,000	0,000	0,000	0.133	0.0000	0.189	0.0000	0,000	0,000	0,000
S5	Ch.C. (w1)	0,000	0,000	0,000	0.162	0.0000	0.143	0.0000	0,000	0,000	0,000
S5	Ch.C.1	0,000	0,000	0,000	0.162	0.0000	0.173	0.0000	0,000	0,000	0,000
S7	Ch.C.1	0,000	0,000	0,000	0.532	0.0000	0.586	0.0000	0,000	0,000	0,000
S8	Ch.C. (w1)	0,000	0,000	0,000	0.137	0.0000	0.000	0.0000	0,000	0,000	0,000
S8	Ch.C.1	0,000	0,000	0,000	0.137	0.0000	0.156	0.0000	0,000	0,000	0,000
S9	Ch.C.1	0,000	0,000	0,000	0.164	0.0000	0.159	0.0000	0,000	0,000	0,000
S12	Ch.C.1	0,000	0,000	0,000	0.154	0.0000	0.144	0.0000	0,000	0,000	0,000
S13	Ch.C. (w1)	0,000	0,000	0,000	0.221	0.0000	0.257	0.0000	0,000	0,000	0,000
S14	Ch.C. (w1)	0,000	0,000	0,000	0.180	0.0000	0.222	0.0000	0,000	0,000	0,000
S14	Ch.C.1	0,000	0,000	0,000	0.000	0.0000	0.221	0.0000	0,000	0,000	0,000
S16	Ch.C.1	0,000	0,000	0,000	0.267	0.0001	0.000	0.0000	0,000	0,000	0,000
S17	Ch.C. (w1)	0,000	0,000	0,000	1.157	0.0000	0.321	0.0000	0,000	0,000	0,000
S17	Ch.C.1	0,000	0,000	0,000	0.661	0.0001	0.321	0.0000	0,000	0,000	0,000

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S18	Ch.C. (w1)	0,000	0,000	0,000	0.250	0.0000	0.000	0.0000	0,000	0,000	0,000
S18	Ch.C.1	0,000	0,000	0,000	0.147	0.0000	0.000	0.0000	0,000	0,000	0,000
S20	Ch.C.1	0,000	0,000	0,000	0.291	0.0000	0.000	0.0000	0,000	0,000	0,000
-	-	m	m	m	m	m	m	m	m	m	m

PIC. STAALCONTROLE

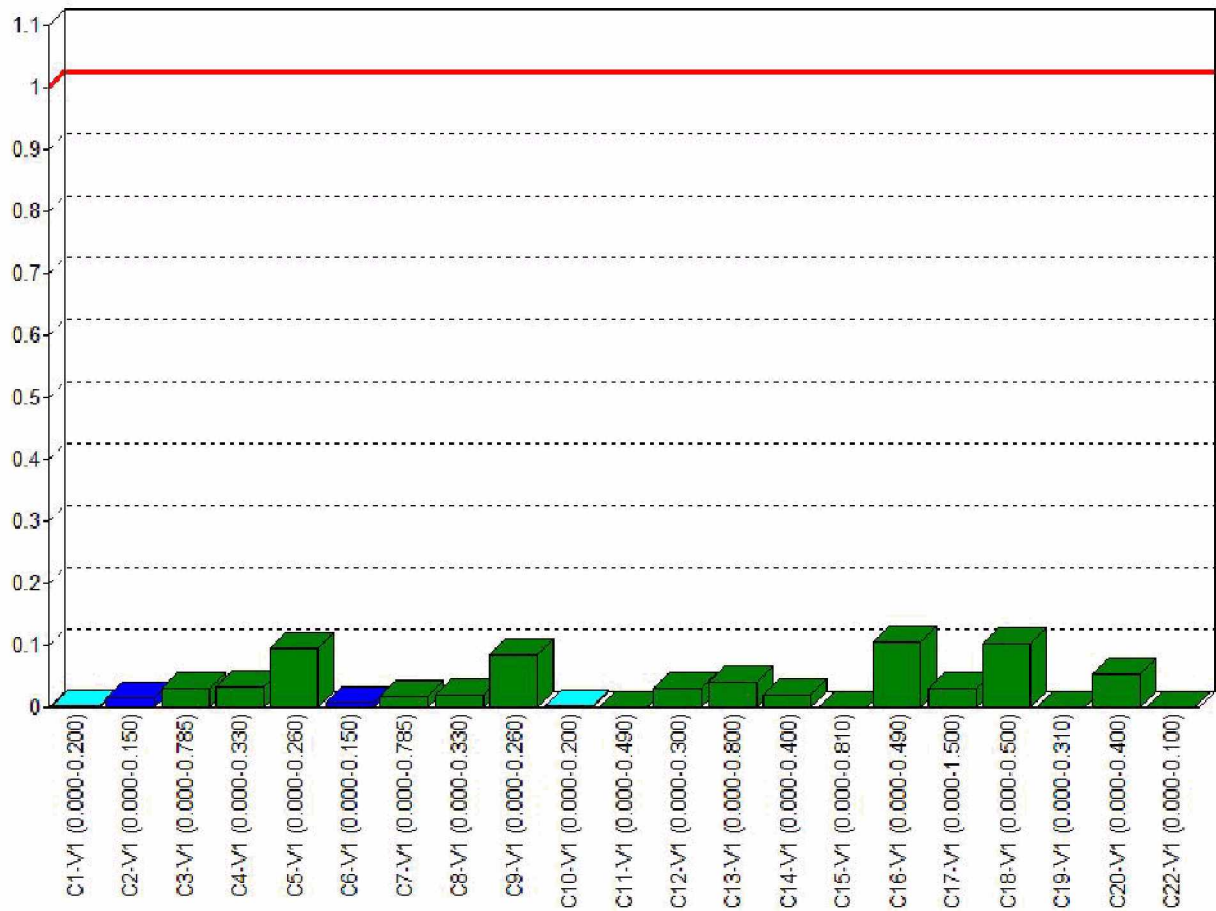


SM DEF.

Structural member	Member (s)
C1	S1
C2	S2
C3	S3
C4	S4
C5	S5
C6	S6
C7	S7
C8	S8
C9	S9
C10	S10
C11	S11
C12	S12
C13	S13
C14	S14
C15	S15
C16	S16
C17	S17
C18	S18
C19	S19

C20 S20
C22 S22

PIC. STEEL UC CHART



UNITY CHECK NEN-EN1993-1-1:2016/NB:2016

Field	Check Type	Combination	Formula	UC max
C1-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C1-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C1-V1 (0.000-0.200)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C2-V1 (0.000-0.150)	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,02
C2-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C2-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C2-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C2-V1 (0.000-0.150)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C3-V1 (0.000-0.785)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C3-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C3-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C3-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C3-V1 (0.000-0.785)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C4-V1 (0.000-0.330)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C4-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C4-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C4-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C4-V1 (0.000-0.330)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C5-V1 (0.000-0.260)	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10

Field	Check Type	Combination	Formula	UC max
C5-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,04
C5-V1 (0.000-0.260)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C6-V1 (0.000-0.150)	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,00
C6-V1 (0.000-0.150)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C7-V1 (0.000-0.785)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C7-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C7-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C7-V1 (0.000-0.785)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C7-V1 (0.000-0.785)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C8-V1 (0.000-0.330)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C8-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C8-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C8-V1 (0.000-0.330)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C8-V1 (0.000-0.330)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C9-V1 (0.000-0.260)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C9-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C9-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C9-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C9-V1 (0.000-0.260)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C10-V1 (0.000-0.200)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C11-V1 (0.000-0.490)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C11-V1 (0.000-0.490)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C12-V1 (0.000-0.300)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C12-V1 (0.000-0.300)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-0.300)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-0.300)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,03
C12-V1 (0.000-0.300)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C13-V1 (0.000-0.800)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C13-V1 (0.000-0.800)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C14-V1 (0.000-0.400)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,02
C14-V1 (0.000-0.400)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C15-V1 (0.000-0.810)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C15-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C15-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C15-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C15-V1 (0.000-0.810)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C16-V1 (0.000-0.490)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C16-V1 (0.000-0.490)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C17-V1 (0.000-1.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C17-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C17-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C17-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,02
C17-V1 (0.000-1.500)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C18-V1 (0.000-0.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C18-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C18-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C18-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,07
C18-V1 (0.000-0.500)	Lat.Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

	Pole 3	
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Field	Check Type	Combination	Formula	UC max
C19-V1 (0.000-0.310)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C19-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C19-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C19-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C19-V1 (0.000-0.310)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C20-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,05
C20-V1 (0.000-0.400)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C22-V1 (0.000-0.100)	Cross	Pe.C.1	NEN-EN1993-1-1(6.17)	0,00
C22-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

EXTREME UNITY CHECK NEN-EN1993-1-1:2016/NB:2016

Field	Check Type	Combination	Formula	UC max
C1-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C2-V1 (0.000-0.150)	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,02
C3-V1 (0.000-0.785)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C4-V1 (0.000-0.330)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C5-V1 (0.000-0.260)	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
C6-V1 (0.000-0.150)	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C7-V1 (0.000-0.785)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C8-V1 (0.000-0.330)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C9-V1 (0.000-0.260)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C11-V1 (0.000-0.490)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C12-V1 (0.000-0.300)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C13-V1 (0.000-0.800)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C14-V1 (0.000-0.400)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,02
C15-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C16-V1 (0.000-0.490)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C17-V1 (0.000-1.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C18-V1 (0.000-0.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C19-V1 (0.000-0.310)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C19-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C20-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,05
C22-V1 (0.000-0.100)	Cross	Pe.C.1	NEN-EN1993-1-1(6.17)	0,00
C22-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

UNITY CHECK FOR STRUCTURAL MEMBERS NEN-EN1993-1-1:2016/NB:2016

Label	Check Type	Combination	Formula	UC max
C1	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C2	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,02
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C3	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C4	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C5	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,04

	Pole 3		
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Label	Check Type	Combination	Formula	UC max
C6	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,00
C7	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C8	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C9	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C10	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C11	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C12	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,03
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C13	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C14	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,02
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C15	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C16	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C17	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,02
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C18	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,07
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C19	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C20	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,05
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C22	Cross	Pe.C.1	NEN-EN1993-1-1(6.17)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

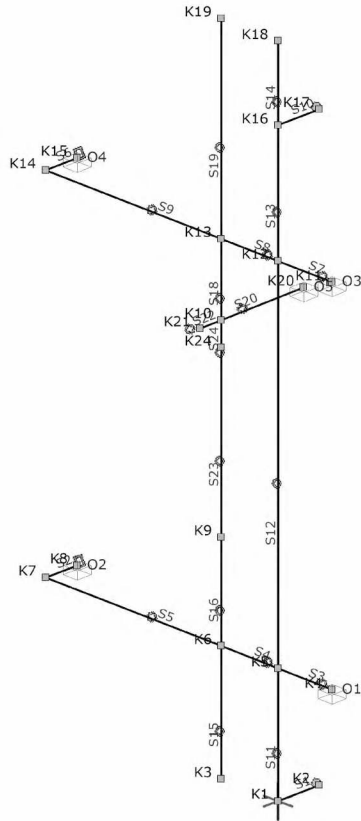
EXTREME UNITY CHECK FOR STRUCTURAL MEMBERS NEN-EN1993-1-1:2016/NB:2016

Label	Check Type	Combination	Formula	UC max
C1	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C2	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,02

Label	Check Type	Combination	Formula	UC max
C3	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C4	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C5	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
C6	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C7	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C8	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C9	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C10	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C11	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C12	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C13	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C14	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,02
C15	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C16	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C17	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C18	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C19	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C19	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C20	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,05
C22	Cross	Pe.C.1	NEN-EN1993-1-1(6.17)	0,00
C22	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

Pole 4			
Project name	L025673	Project number	
Part description	Pole 4	Structural engineer	5.1.2.e
Client		Units	m, kN, kNm

PIC. GEOMETRIE RAAMWERK



MEMBERS

Member	Node B	Node E	X-B	Y-B	Z-B	X-E	Y-E	Z-E	Length	Section	Pos
S1	K2	K1	0,200	0,000	0,000	0,000	0,000	0,000	0,200	P3	0,000 - L(0,
S2	K8	K7	0,150	-1,115	-0,490	0,000	-1,115	-0,490	0,150	P2	0,000 - L(0,
S3	K4	K5	0,000	0,260	-0,490	0,000	0,000	-0,490	0,260	P1	0,000 - L(0,
S4	K5	K6	0,000	0,000	-0,490	0,000	-0,275	-0,490	0,275	P1	0,000 - L(0,
S5	K6	K7	0,000	-0,275	-0,490	0,000	-1,115	-0,490	0,840	P1	0,000 - L(0,
S6	K15	K14	0,150	-1,115	-1,990	0,000	-1,115	-1,990	0,150	P2	0,000 - L(0,
S7	K11	K12	0,000	0,260	-1,990	0,000	0,000	-1,990	0,260	P1	0,000 - L(0,
S8	K12	K13	0,000	0,000	-1,990	0,000	-0,275	-1,990	0,275	P1	0,000 - L(0,
S9	K13	K14	0,000	-0,275	-1,990	0,000	-1,115	-1,990	0,840	P1	0,000 - L(0,
S10	K17	K16	0,200	0,000	-2,490	0,000	0,000	-2,490	0,200	P3	0,000 - L(0,
S11	K1	K5	0,000	0,000	0,000	0,000	0,000	-0,490	0,490	P1	0,000 - L(0,
S12	K5	K12	0,000	0,000	-0,490	0,000	0,000	-1,990	1,500	P1	0,000 - L(1,
S13	K12	K16	0,000	0,000	-1,990	0,000	0,000	-2,490	0,500	P1	0,000 - L(0,
S14	K16	K18	0,000	0,000	-2,490	0,000	0,000	-2,800	0,310	P1	0,000 - L(0,
S15	K3	K6	0,000	-0,275	0,000	0,000	-0,275	-0,490	0,490	P1	0,000 - L(0,
S16	K6	K9	0,000	-0,275	-0,490	0,000	-0,275	-0,890	0,400	P1	0,000 - L(0,
S18	K10	K13	0,000	-0,275	-1,690	0,000	-0,275	-1,990	0,300	P1	0,000 - L(0,
S19	K13	K19	0,000	-0,275	-1,990	0,000	-0,275	-2,800	0,810	P1	0,000 - L(0,
S20	K10	K20	0,000	-0,275	-1,690	0,400	-0,275	-1,690	0,400	P1	0,000 - L(0,
S22	K21	K10	-0,100	-0,275	-1,690	0,000	-0,275	-1,690	0,100	P1	0,000 - L(0,
S23	K9	K24	0,000	-0,275	-0,890	0,000	-0,275	-1,590	0,700	P1	0,000 - L(0,
S24	K24	K10	0,000	-0,275	-1,590	0,000	-0,275	-1,690	0,100	P1	0,000 - L(0,
-	-	-	m	m	m	m	m	m	m	-	

	Pole 4	
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SECTIONS

Section	Section Name	Area	It	Iy	Iz	Material	Angle
P1	N60.3/4	7.0749e-04	5.6346e-07	2.8173e-07	2.8173e-07	S235H(EN10210-1)	0,0
P2	K60x60x4x4	8.9600e-04	7.0246e-07	4.7070e-07	4.7070e-07	S235	0,0
P3	C100	7.8540e-03	9.8175e-06	4.9087e-06	4.9087e-06	S235	0,0
-	-	m2	m4	m4	m4	-	°

SECTION SHAPES

Section	Tapered	hB	hE	tf	tw	tf2	B	b1	b2	Castellate	Height
P2	No	0,060	0,060	0,0040	0,0040	0,0000	0,060	0,000	0,000	No	0,000
P3	No	0,100	0,100	0,0000	0,0000	0,0000	0,100	0,000	0,000	No	0,000
-	-	m	m	m	m	m	m	m	m	-	m

MATERIALS

Material Name	Poison	Density	Youngs mod.	Exp.
S235H(EN10210-1)	0.30	78.50	2.1000e+08	12.0000e-06
S235	0.30	78.50	2.1000e+08	12.0000e-06
-	-	kN/m3	kN/m2	C°m

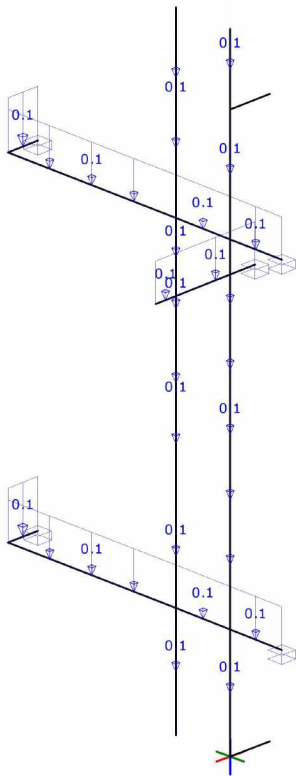
SUPPORTS

Support	Object	Position	X	Y	Z	Xr	Yr	Zr	AngleXr	AngleYr	AngleZr
O1	K4	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O2	K8	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O3	K11	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O4	K15	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
O5	K20	0,000	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed	0	0	0
-	-	m	kN/m	kN/m	kN/m	kNm/rad	kNm/rad	kNm/rad	°	°	°

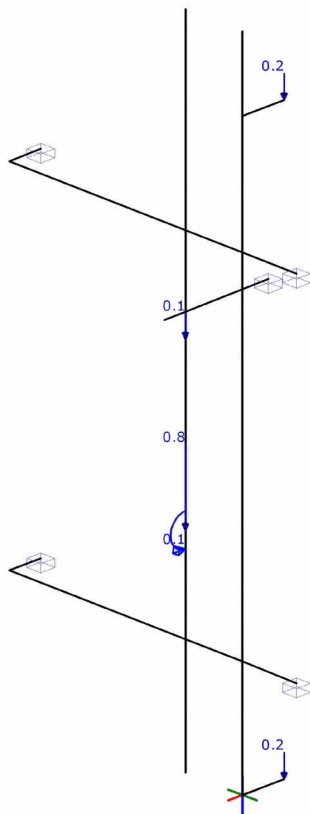
LOADS CASES TYPES

Label	Description	L.C. Type	Fav./Unfav.	Element	Level	Field	Psi0	Psi1	Psi2	Cprob
B.G.1	Permanent actions	Permanent actions	-		None	None				ULS/SLS
B.G.2	Permanent actions	Permanent actions	-		None	None				
B.G.3	wind x	Wind load	-		None	None		0.20		1,00/1,00

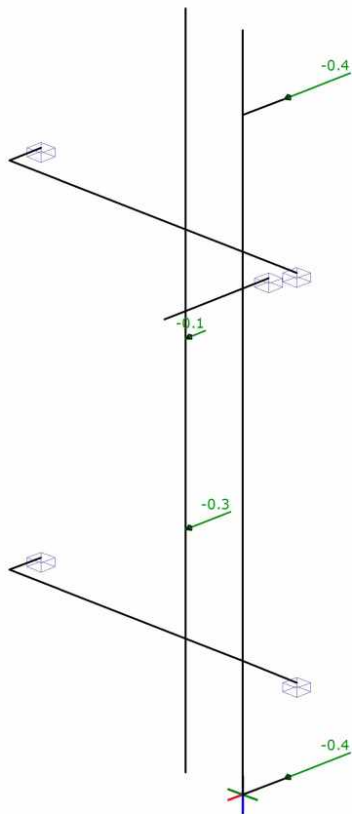
PIC. LASTEN B.G.1 PERMANENT ACTIONS



PIC. LASTEN B.G.2 PERMANENT ACTIONS



PIC. LASTEN B.G.3 WIND X

**PERSISTENT LOADS COMBINATIONS (TABLE)**

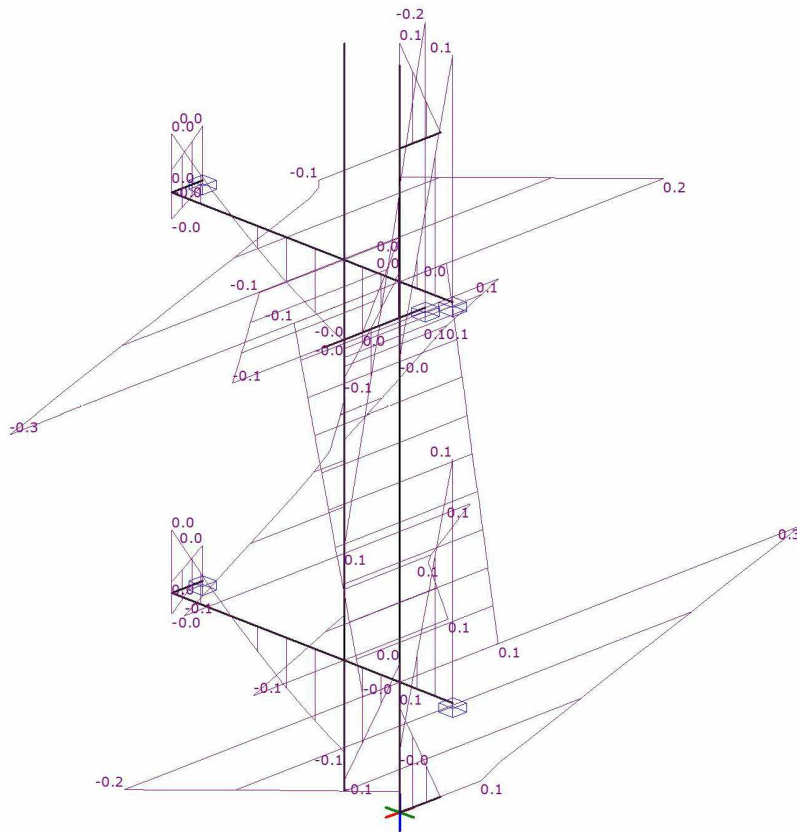
L.C.	Description	Pe.C.1	Pe.C.2	Pe.C.3
B.G.1	Permanent actions	1.35	1.20	0.90
B.G.2	Permanent actions	1.35	1.20	0.90
B.G.3	wind x	0.00	1.27	-1.27

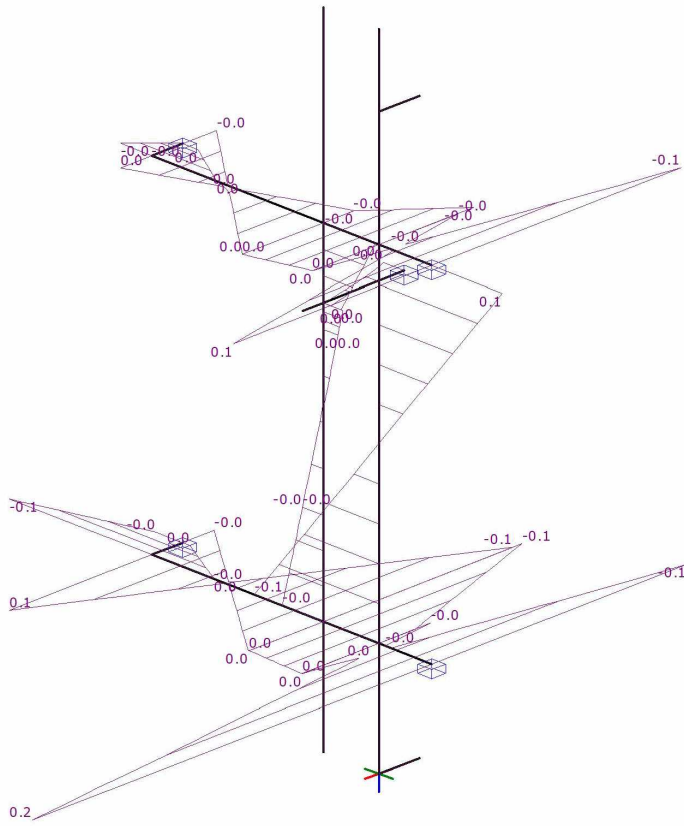
CHARACTERISTIC LOADS COMBINATIONS (TABLE)

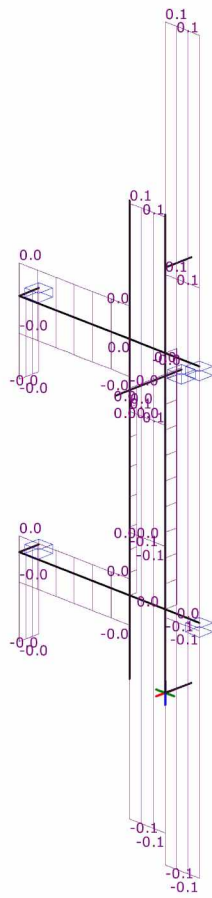
L.C.	Description	Ch.C.(w1)	Ch.C.1
B.G.1	Permanent actions	1.00	1.00
B.G.2	Permanent actions	1.00	1.00
B.G.3	wind x	0.00	0.57

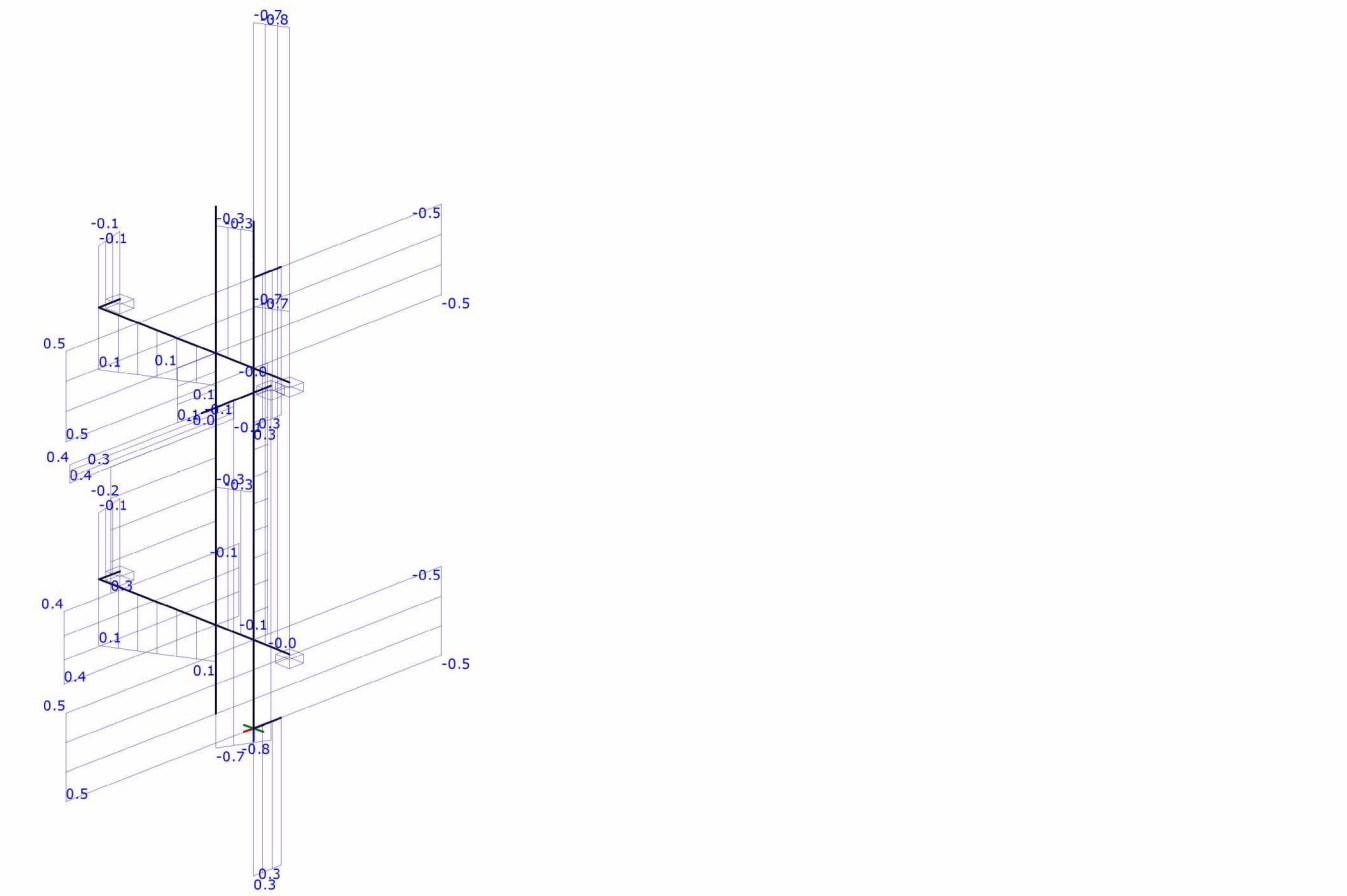
ANALYSIS ASSUMPTIONS

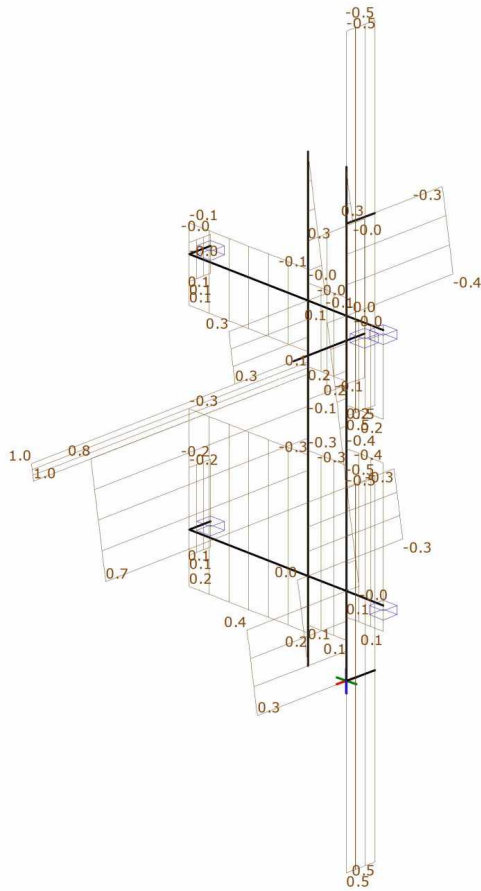
Linear Elastic Analysis performed









**PE.C. MEMBER FORCES (MY, MZ)**

Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S1	Pe.C.1	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.2	My	0.00			0.06	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
	Pe.C.3	My	0.00			0.04	0.000	0.000
		Mz	0.00			0.00	0.000	0.000
S2	Pe.C.1	My	0.02			0.00	0.124	0.000
		Mz	-0.01			-0.02	0.000	0.000
	Pe.C.2	My	0.01			-0.01	0.055	0.000
		Mz	0.00			0.03	0.000	0.000
	Pe.C.3	My	0.02			0.01	0.000	0.000
		Mz	-0.01			-0.06	0.000	0.000
S3	Pe.C.1	My	0.15			-0.04	0.202	0.000
		Mz	0.04			0.01	0.000	0.000
	Pe.C.2	My	0.13			-0.04	0.202	0.000
		Mz	-0.11			0.03	0.200	0.000
	Pe.C.3	My	0.10			-0.03	0.201	0.000
		Mz	0.17			-0.02	0.230	0.000
S4	Pe.C.1	My	0.01			-0.07	0.037	0.000
		Mz	0.01			-0.03	0.068	0.000
	Pe.C.2	My	0.01			-0.07	0.038	0.000
		Mz	0.03			0.03	0.000	0.000
	Pe.C.3	My	0.01			-0.05	0.035	0.000
		Mz	-0.02			-0.09	0.000	0.000
S5	Pe.C.1	My	-0.05			0.04	0.552	0.000
		Mz	-0.03			0.02	0.496	0.000
	Pe.C.2	My	-0.05			0.03	0.551	0.000

	Pole 4				
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Member	LComb	Value	Mb	Mmax	xMmax	Me	x-M0	x-M0
S18	Pe.C.2	Mz	-0.02			-0.01	0.000	0.000
		My	0.08			0.06	0.000	0.000
	Pe.C.3	Mz	-0.02			-0.01	0.000	0.000
		My	-0.07			0.10	0.167	0.000
	Pe.C.1	Mz	-0.01			-0.01	0.000	0.000
		My	-0.05			-0.01	0.000	0.000
	Pe.C.2	Mz	0.01			0.02	0.000	0.000
		My	0.01			0.04	0.000	0.000
	Pe.C.3	Mz	0.01			0.02	0.000	0.000
		My	-0.09			-0.07	0.000	0.000
S19	Pe.C.1	Mz	0.00			0.02	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S20	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.13			-0.17	0.172	0.000
		My	0.00			-0.01	0.085	0.000
	Pe.C.2	Mz	0.11			-0.15	0.167	0.000
		My	0.00			-0.01	0.015	0.000
S22	Pe.C.3	Mz	0.09			-0.12	0.177	0.000
		My	0.00			-0.01	0.141	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
S23	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.2	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.3	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
	Pe.C.1	Mz	-0.10			0.06	0.451	0.000
		My	-0.01			0.01	0.410	0.000
	Pe.C.2	Mz	-0.12			0.08	0.424	0.000
		My	-0.01			0.01	0.396	0.000
S24	Pe.C.3	Mz	-0.04			0.01	0.571	0.000
		My	-0.01			0.00	0.433	0.000
	Pe.C.1	Mz	0.06			0.08	0.000	0.000
		My	0.01			0.01	0.000	0.000
	Pe.C.2	Mz	0.08			0.12	0.000	0.000
		My	0.01			0.01	0.000	0.000
	Pe.C.3	Mz	0.01			0.00	0.000	0.000
		My	0.01			0.00	0.000	0.000
	Pe.C.1	Mz	0.00			0.00	0.000	0.000
		My	0.00			0.00	0.000	0.000
-	-	-	kNm	kNm	m	kNm	m	m

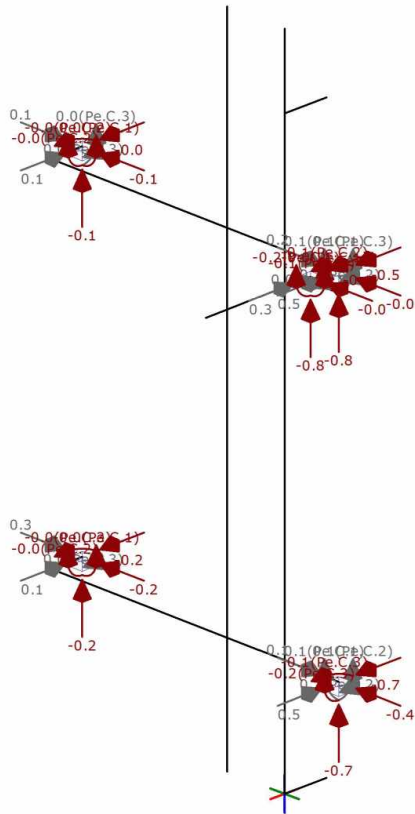
PE.C. MEMBER FORCES (NX, VY, VZ, MX)

Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S1	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.52 Vz	0.28	0.28	0.28	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.52 Vz	0.21	0.21	0.21	0.00	0.00
			Vy	0.00	0.00	0.00		
S2	Pe.C.1	C	-0.06 Vz	-0.16	-0.16	-0.14	-0.04	-0.04
			Vy	-0.11	-0.11	-0.11		
	Pe.C.2	T	0.07 Vz	-0.14	-0.14	-0.13	-0.03	-0.03
			Vy	0.15	0.15	0.15		
	Pe.C.3	C	-0.17 Vz	-0.10	-0.10	-0.09	-0.02	-0.02
			Vy	-0.33	-0.33	-0.33		
S3	Pe.C.1	C	-0.20 Vz	-0.73	-0.73	-0.71	0.02	0.02
			Vy	-0.13	-0.13	-0.13		
	Pe.C.2	T	0.07 Vz	-0.66	-0.66	-0.64	0.14	0.14
			Vy	0.54	0.54	0.54		
	Pe.C.3	C	-0.39 Vz	-0.48	-0.48	-0.47	-0.11	-0.11
			Vy	-0.74	-0.74	-0.74		
S4	Pe.C.1	C	-0.13 Vz	-0.31	-0.31	-0.29	-0.01	-0.01
			Vy	-0.17	-0.17	-0.17		

	Pole 4		
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Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S4	Pe.C.2	T	0.13 Vz	-0.29	-0.29	-0.27	-0.09	-0.09
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.34 Vz	-0.20	-0.20	-0.19	0.08	0.08
			Vy	-0.25	-0.25	-0.25		
S5	Pe.C.1	C	-0.11 Vz	0.08	0.14	0.14	0.00	0.00
			Vy	0.06	0.06	0.06		
	Pe.C.2	T	0.15 Vz	0.07	0.13	0.13	-0.01	-0.01
			Vy	-0.07	-0.07	-0.07		
S6	Pe.C.3	C	-0.33 Vz	0.05	0.09	0.09	0.01	0.01
			Vy	0.17	0.17	0.17		
	Pe.C.1	T	0.03 Vz	-0.15	-0.15	-0.13	-0.04	-0.04
			Vy	0.04	0.04	0.04		
	Pe.C.2	T	0.07 Vz	-0.13	-0.13	-0.12	-0.03	-0.03
			Vy	0.14	0.14	0.14		
	Pe.C.3	C	-0.03 Vz	-0.10	-0.10	-0.09	-0.02	-0.02
			Vy	-0.08	-0.08	-0.08		
S7	Pe.C.1	T	0.15 Vz	-0.76	-0.76	-0.74	0.02	0.02
			Vy	0.04	0.04	0.04		
	Pe.C.2	T	0.24 Vz	-0.68	-0.68	-0.66	-0.11	-0.11
			Vy	0.52	0.52	0.52		
	Pe.C.3	C	0.00 Vz	-0.50	-0.50	-0.48	0.14	0.14
			Vy	-0.46	-0.46	-0.46		
S8	Pe.C.1	T	0.08 Vz	-0.29	-0.29	-0.27	-0.02	-0.02
			Vy	0.08	0.08	0.08		
	Pe.C.2	T	0.18 Vz	-0.26	-0.26	-0.25	0.06	0.06
			Vy	0.02	0.02	0.02		
	Pe.C.3	C	-0.05 Vz	-0.19	-0.19	-0.18	-0.08	-0.08
			Vy	0.10	0.10	0.10		
S9	Pe.C.1	T	0.04 Vz	0.07	0.13	0.13	0.00	0.00
			Vy	-0.03	-0.03	-0.03		
	Pe.C.2	T	0.14 Vz	0.06	0.12	0.12	0.01	0.01
			Vy	-0.07	-0.07	-0.07		
	Pe.C.3	C	-0.08 Vz	0.04	0.09	0.09	-0.02	-0.02
			Vy	0.03	0.03	0.03		
S10	Pe.C.1	-	0.00 Vz	0.32	0.32	0.32	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.52 Vz	0.28	0.28	0.28	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.52 Vz	0.21	0.21	0.21	0.00	0.00
			Vy	0.00	0.00	0.00		
S11	Pe.C.1	T	0.35 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	T	0.31 Vz	0.52	0.52	0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	T	0.24 Vz	-0.52	-0.52	-0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
S12	Pe.C.1	T	0.07 Vz	-0.04	-0.04	-0.04	0.00	0.00
			Vy	0.07	0.07	0.07		
	Pe.C.2	T	0.06 Vz	-0.03	-0.03	-0.03	0.00	0.00
			Vy	0.06	0.06	0.06		
	Pe.C.3	T	0.04 Vz	-0.04	-0.04	-0.04	0.01	0.01
			Vy	0.05	0.05	0.05		
S13	Pe.C.1	C	-0.38 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.33 Vz	-0.52	-0.52	-0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.25 Vz	0.52	0.52	0.52	0.00	0.00
			Vy	0.00	0.00	0.00		
S14	Pe.C.1	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.3	C	-0.02 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
S15	Pe.C.1	T	0.04 Vz	0.00	0.00	0.00	0.00	0.00
			Vy	0.00	0.00	0.00		
	Pe.C.2	T	0.03 Vz	0.00	0.00	0.00	0.00	0.00

			Pole 4					
Member	LComb	TC	Nmax Value	Vb	Vmax	Ve	Mxb	Mxe
S16	Pe.C.3	T	Vy	0.00	0.00	0.00		
			0.02 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.1	C	Vy	0.00	0.00	0.00		
			-0.34 Vz	0.23	0.23	0.23	0.00	0.00
	Pe.C.2	C	Vy	0.02	0.02	0.02		
			-0.31 Vz	-0.06	-0.06	-0.06	0.00	0.00
S18	Pe.C.3	C	Vy	0.02	0.02	0.02		
			-0.21 Vz	0.42	0.42	0.42	0.00	0.00
	Pe.C.1	T	Vy	0.01	0.01	0.01		
			0.28 Vz	0.11	0.11	0.11	0.00	0.00
	Pe.C.2	T	Vy	0.05	0.05	0.05		
			0.25 Vz	0.10	0.10	0.10	0.00	0.00
S19	Pe.C.3	T	Vy	0.04	0.04	0.04		
			0.18 Vz	0.07	0.07	0.07	0.00	0.00
	Pe.C.1	C	Vy	0.04	0.04	0.04		
			-0.06 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.2	C	Vy	0.00	0.00	0.00		
			-0.05 Vz	0.00	0.00	0.00	0.00	0.00
S20	Pe.C.3	C	Vy	0.00	0.00	0.00		
			-0.04 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.12 Vz	-0.73	-0.76	-0.76	0.00	0.00
	Pe.C.2	T	Vy	-0.03	-0.03	-0.03		
			0.31 Vz	-0.63	-0.66	-0.66	0.00	0.00
S22	Pe.C.3	C	Vy	-0.02	-0.02	-0.02		
			-0.12 Vz	-0.50	-0.52	-0.52	0.00	0.00
	Pe.C.1	-	Vy	-0.02	-0.02	-0.02		
			0.00 Vz	0.00	-0.01	-0.01	0.00	0.00
	Pe.C.2	-	Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	-0.01	-0.01	0.00	0.00
S23	Pe.C.3	-	Vy	0.00	0.00	0.00		
			0.00 Vz	0.00	0.00	0.00	0.00	0.00
	Pe.C.1	T	Vy	0.00	0.00	0.00		
			0.77 Vz	0.23	0.23	0.23	0.00	0.00
	Pe.C.2	T	Vy	0.02	0.02	0.02		
			0.67 Vz	0.29	0.29	0.29	0.00	0.00
S24	Pe.C.3	T	Vy	0.02	0.02	0.02		
			0.52 Vz	0.06	0.06	0.06	0.00	0.00
	Pe.C.1	T	Vy	0.01	0.01	0.01		
			0.98 Vz	0.23	0.23	0.23	0.00	0.00
	Pe.C.2	T	Vy	0.02	0.02	0.02		
			0.86 Vz	0.41	0.41	0.41	0.00	0.00
	Pe.C.3	T	Vy	0.02	0.02	0.02		
			0.66 Vz	-0.05	-0.05	-0.05	0.00	0.00
-	-	-	Vy	0.01	0.01	0.01		
-	-	-	kN -	kN	kN	kN	kNm	kNm



PE.C. SUPPORT REACTIONS

LComb	Support	Node	X	Y	Z	Mx	My	Mz
Pe.C.1	O1	K4	-0.13	-0.20	-0.73	0.15	0.02	-0.04
Pe.C.1	O2	K8	-0.06	0.11	-0.16	-0.04	-0.02	0.01
Pe.C.1	O3	K11	0.04	0.15	-0.76	0.15	0.02	0.01
Pe.C.1	O4	K15	0.03	-0.04	-0.15	-0.04	-0.02	0.00
Pe.C.1	O5	K20	0.12	-0.03	-0.76	0.00	-0.17	0.01
Sum Reactions			0.00	0.00	-2.55			
Sum Loads			0.00	0.00	2.55			
Pe.C.2	O1	K4	0.54	0.07	-0.66	0.13	0.14	0.11
Pe.C.2	O2	K8	0.07	-0.15	-0.14	-0.03	-0.01	0.00
Pe.C.2	O3	K11	0.52	0.24	-0.68	0.13	-0.11	0.11
Pe.C.2	O4	K15	0.07	-0.14	-0.13	-0.03	-0.03	-0.01
Pe.C.2	O5	K20	0.31	-0.02	-0.66	0.00	-0.15	0.01
Sum Reactions			1.52	0.00	-2.26			
Sum Loads			-1.52	0.00	2.26			
Pe.C.3	O1	K4	-0.74	-0.39	-0.48	0.10	-0.11	-0.17
Pe.C.3	O2	K8	-0.17	0.33	-0.10	-0.02	-0.02	0.01
Pe.C.3	O3	K11	-0.46	0.00	-0.50	0.10	0.14	-0.08
Pe.C.3	O4	K15	-0.03	0.08	-0.10	-0.02	0.00	0.00
Pe.C.3	O5	K20	-0.12	-0.02	-0.52	0.00	-0.12	0.01
Sum Reactions			-1.52	0.00	-1.70			
Sum Loads			1.52	0.00	1.70			
-	-	-	kN	kN	kN	kNm	kNm	kNm

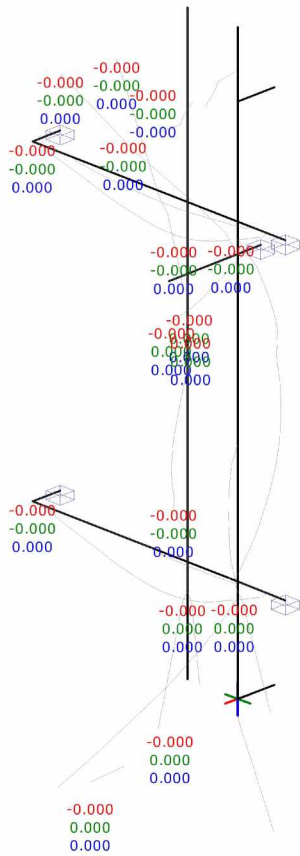
L.C. SUPPORT REACTIONS

L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O1	K4	0.00	-0.02	-0.14	0.03	0.00	0.00
B.G.1	O2	K8	0.00	0.00	-0.05	-0.01	-0.01	0.00
B.G.1	O3	K11	0.02	0.04	-0.15	0.03	0.00	0.01

L.C.	Support	Node	X	Y	Z	Mx	My	Mz
B.G.1	O4	K15	0.01	-0.01	-0.05	-0.01	-0.01	0.00
B.G.1	O5	K20	-0.02	-0.01	-0.12	0.00	-0.03	0.00
Sum Reactions			0.00	0.00	-0.51			
Sum Loads			0.00	0.00	0.51			
B.G.2	O1	K4	-0.09	-0.13	-0.40	0.08	0.02	-0.03
B.G.2	O2	K8	-0.05	0.08	-0.06	-0.02	-0.01	0.00
B.G.2	O3	K11	0.01	0.08	-0.42	0.08	0.01	0.00
B.G.2	O4	K15	0.01	-0.01	-0.06	-0.02	-0.01	0.00
B.G.2	O5	K20	0.11	-0.02	-0.44	0.00	-0.10	0.00
Sum Reactions			0.00	0.00	-1.37			
Sum Loads			0.00	0.00	1.37			
B.G.3	O1	K4	0.51	0.20	0.00	0.00	0.10	0.11
B.G.3	O2	K8	0.10	-0.20	0.00	0.00	0.01	-0.01
B.G.3	O3	K11	0.38	0.08	-0.01	0.00	-0.10	0.07
B.G.3	O4	K15	0.04	-0.08	0.00	0.00	-0.01	0.00
B.G.3	O5	K20	0.16	0.00	0.01	0.00	0.00	0.00
Sum Reactions			1.19	0.00	0.00			
Sum Loads			-1.19	0.00	0.00			
-	-	-	kN	kN	kN	kNm	kNm	kNm

PIC. CH.C. VERPLAATSINGEN ENVELOPE

Characteristic Loads Combinations



CH.C. NODAL DISPLACEMENTS

Node	LComb	X	Y	Z	Xr	Yr	Zr
K1	Ch.C.(w1)	-0.0001	0.0001	0.0000	0.168e-03	-0.478e-03	-0.076e-03
	Ch.C.1	-0.0005	0.0001	0.0000	0.170e-03	-1.264e-03	0.040e-03
K2	Ch.C.(w1)	-0.0001	0.0001	0.0001	0.168e-03	-0.483e-03	-0.076e-03
	Ch.C.1	-0.0005	0.0001	0.0003	0.170e-03	-1.268e-03	0.040e-03
K3	Ch.C.(w1)	0.0000	0.0000	0.0001	0.063e-03	-0.042e-03	-0.036e-03
	Ch.C.1	-0.0001	0.0000	0.0001	0.063e-03	-0.125e-03	-0.009e-03

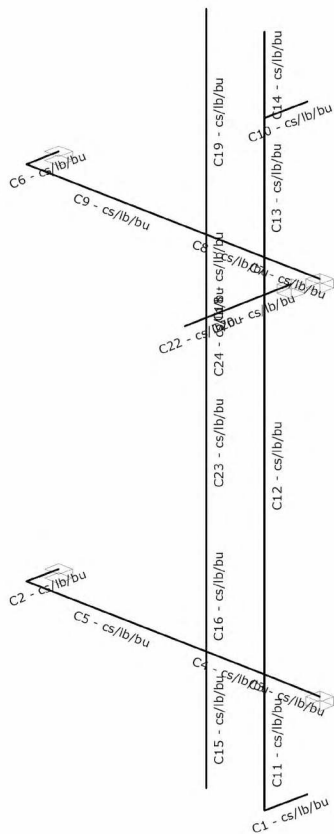
Pole 4							
Node	LComb	X	Y	Z	Xr	Yr	Zr
K4	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K5	Ch.C.(w1)	0.0000	0.0000	0.0000	0.168e-03	-0.090e-03	-0.076e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.170e-03	-0.402e-03	0.040e-03
K6	Ch.C.(w1)	0.0000	0.0000	0.0001	0.063e-03	-0.042e-03	-0.036e-03
	Ch.C.1	0.0000	0.0000	0.0001	0.063e-03	-0.125e-03	-0.009e-03
K7	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.074e-03	0.008e-03	0.015e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.075e-03	0.002e-03	-0.003e-03
K8	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K9	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.003e-03	0.224e-03	-0.018e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.004e-03	0.207e-03	0.000e-03
K10	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.006e-03	0.107e-03	0.016e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.005e-03	0.114e-03	0.018e-03
K11	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K12	Ch.C.(w1)	0.0000	0.0000	0.0000	0.164e-03	-0.077e-03	0.028e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.165e-03	0.249e-03	0.089e-03
K13	Ch.C.(w1)	0.0000	0.0000	0.0001	0.051e-03	-0.007e-03	0.017e-03
	Ch.C.1	0.0000	0.0000	0.0001	0.052e-03	0.127e-03	0.019e-03
K14	Ch.C.(w1)	0.0000	0.0000	0.0000	-0.069e-03	0.010e-03	-0.007e-03
	Ch.C.1	0.0000	0.0000	0.0000	-0.069e-03	0.020e-03	-0.015e-03
K15	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K16	Ch.C.(w1)	0.0001	-0.0001	0.0000	0.164e-03	-0.472e-03	0.028e-03
	Ch.C.1	-0.0002	-0.0001	0.0000	0.165e-03	0.347e-03	0.089e-03
K17	Ch.C.(w1)	0.0001	-0.0001	0.0001	0.164e-03	-0.477e-03	0.028e-03
	Ch.C.1	-0.0002	-0.0001	0.0000	0.165e-03	0.342e-03	0.089e-03
K18	Ch.C.(w1)	0.0003	-0.0001	0.0000	0.164e-03	-0.472e-03	0.028e-03
	Ch.C.1	-0.0003	-0.0001	0.0000	0.165e-03	0.347e-03	0.089e-03
K19	Ch.C.(w1)	0.0000	0.0000	0.0001	0.051e-03	-0.007e-03	0.017e-03
	Ch.C.1	-0.0001	0.0000	0.0001	0.052e-03	0.127e-03	0.019e-03
K20	Ch.C.(w1)	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
	Ch.C.1	0.0000	0.0000	0.0000	0.000e-03	0.000e-03	0.000e-03
K21	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.006e-03	0.108e-03	0.016e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.005e-03	0.114e-03	0.018e-03
K24	Ch.C.(w1)	0.0000	0.0000	0.0001	-0.015e-03	0.021e-03	0.012e-03
	Ch.C.1	0.0000	0.0000	0.0001	-0.014e-03	-0.002e-03	0.016e-03
-	-	m	m	m	rad	rad	rad

CH.C. EXTREME DEFLECTIONS

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S3	Ch.C. (w1)	0,000	0,000	0,000	0.098	0.0000	0.115	0.0000	0,000	0,000	0,000
S3	Ch.C.1	0,000	0,000	0,000	0.098	0.0000	0.087	0.0000	0,000	0,000	0,000
S4	Ch.C. (w1)	0,000	0,000	0,000	0.165	0.0000	0.173	0.0000	0,000	0,000	0,000
S4	Ch.C.1	0,000	0,000	0,000	0.165	0.0000	0.121	0.0000	0,000	0,000	0,000
S5	Ch.C. (w1)	0,000	0,000	0,000	0.278	0.0000	0.242	0.0000	0,000	0,000	0,000
S5	Ch.C.1	0,000	0,000	0,000	0.277	0.0000	0.620	0.0000	0,000	0,000	0,000
S7	Ch.C.1	0,000	0,000	0,000	0.096	0.0000	0.101	0.0000	0,000	0,000	0,000
S8	Ch.C. (w1)	0,000	0,000	0,000	0.162	0.0000	0.000	0.0000	0,000	0,000	0,000
S8	Ch.C.1	0,000	0,000	0,000	0.162	0.0000	0.145	0.0000	0,000	0,000	0,000
S9	Ch.C.1	0,000	0,000	0,000	0.277	0.0000	0.227	0.0000	0,000	0,000	0,000
S11	Ch.C.1	0,000	0,000	0,000	0.267	0.0001	0.000	0.0000	0,000	0,000	0,000
S12	Ch.C.1	0,000	0,000	0,000	0.660	0.0001	0.320	0.0000	0,000	0,000	0,000
S13	Ch.C. (w1)	0,000	0,000	0,000	0.250	0.0000	0.000	0.0000	0,000	0,000	0,000
S13	Ch.C.1	0,000	0,000	0,000	0.147	0.0000	0.000	0.0000	0,000	0,000	0,000
S16	Ch.C.1	0,000	0,000	0,000	0.207	0.0000	0.189	0.0000	0,000	0,000	0,000

Member	LComb	Node Begin				Member			Node End		
		X	Y	Z	Z' dist	Z'	Y' dist	Y'	X	Y	Z
S18	Ch.C. (w1)	0,000	0,000	0,000	0.137	0.0000	0.162	0.0000	0,000	0,000	0,000
S18	Ch.C.1	0,000	0,000	0,000	0.000	0.0000	0.161	0.0000	0,000	0,000	0,000
S20	Ch.C.1	0,000	0,000	0,000	0.289	0.0000	0.000	0.0000	0,000	0,000	0,000
S23	Ch.C. (w1)	0,000	0,000	0,000	0.225	0.0000	0.200	0.0000	0,000	0,000	0,000
S23	Ch.C.1	0,000	0,000	0,000	0.215	0.0000	0.194	0.0000	0,000	0,000	0,000
S24	Ch.C.1	0,000	0,000	0,000	0.052	0.0000	0.000	0.0000	0,000	0,000	0,000
-	-	m	m	m	m	m	m	m	m	m	m

PIC. STAALCONTROLE

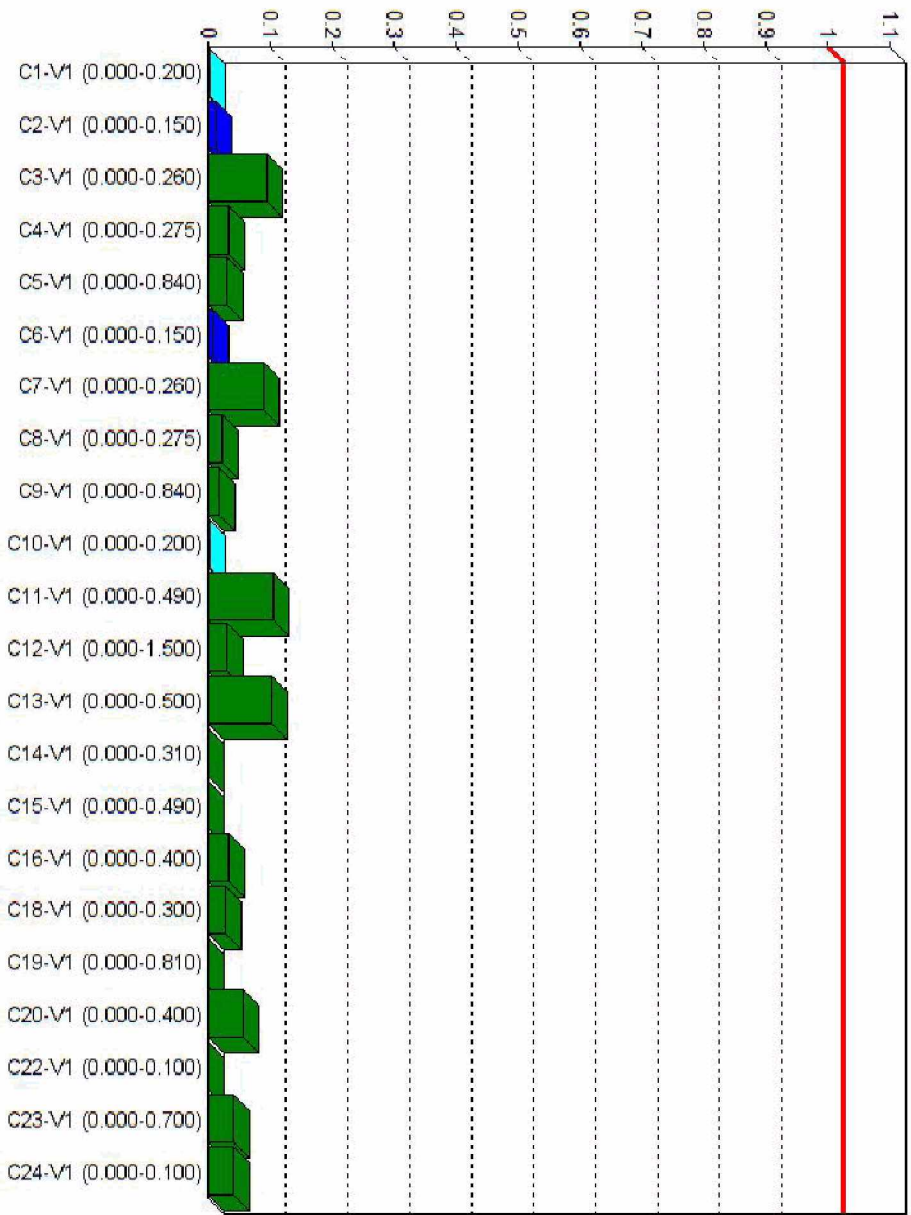


SM DEF.

Structural member	Member (s)
C1	S1
C2	S2
C3	S3
C4	S4
C5	S5
C6	S6
C7	S7
C8	S8
C9	S9
C10	S10
C11	S11
C12	S12
C13	S13
C14	S14
C15	S15
C16	S16

C18	S18
C19	S19
C20	S20
C22	S22
C23	S23
C24	S24

PIC. STEEL UC CHART



	Pole 4		
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Field	Check Type	Combination	Formula	UC max
C4-V1 (0.000-0.275)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C4-V1 (0.000-0.275)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C4-V1 (0.000-0.275)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C5-V1 (0.000-0.840)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C5-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C5-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C5-V1 (0.000-0.840)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C6-V1 (0.000-0.150)	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C6-V1 (0.000-0.150)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,00
C6-V1 (0.000-0.150)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C7-V1 (0.000-0.260)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C7-V1 (0.000-0.260)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C7-V1 (0.000-0.260)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C8-V1 (0.000-0.275)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C8-V1 (0.000-0.275)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C8-V1 (0.000-0.275)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C8-V1 (0.000-0.275)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C8-V1 (0.000-0.275)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C9-V1 (0.000-0.840)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C9-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C9-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C9-V1 (0.000-0.840)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C9-V1 (0.000-0.840)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C10-V1 (0.000-0.200)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C10-V1 (0.000-0.200)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C11-V1 (0.000-0.490)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C11-V1 (0.000-0.490)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C12-V1 (0.000-1.500)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,03
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C12-V1 (0.000-1.500)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,02
C12-V1 (0.000-1.500)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C13-V1 (0.000-0.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C13-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C13-V1 (0.000-0.500)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,07
C13-V1 (0.000-0.500)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C14-V1 (0.000-0.310)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C14-V1 (0.000-0.310)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C15-V1 (0.000-0.490)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C15-V1 (0.000-0.490)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C16-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C16-V1 (0.000-0.400)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C16-V1 (0.000-0.400)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
C16-V1 (0.000-0.400)	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,03
C16-V1 (0.000-0.400)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

Field	Check Type	Combination	Formula	UC max
C18-V1 (0.000-0.300)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C18-V1 (0.000-0.300)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C19-V1 (0.000-0.810)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C19-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C19-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C19-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
C19-V1 (0.000-0.810)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C20-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,06
C20-V1 (0.000-0.400)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C20-V1 (0.000-0.400)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
C20-V1 (0.000-0.400)	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C20-V1 (0.000-0.400)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C22-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(6.12)	0,00
C22-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C23-V1 (0.000-0.700)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C23-V1 (0.000-0.700)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C24-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C24-V1 (0.000-0.100)	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

EXTREME UNITY CHECK NEN-EN1993-1-1:2016/NB:2016

Field	Check Type	Combination	Formula	UC max
C1-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C2-V1 (0.000-0.150)	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,01
C3-V1 (0.000-0.260)	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
C4-V1 (0.000-0.275)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C5-V1 (0.000-0.840)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C6-V1 (0.000-0.150)	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C7-V1 (0.000-0.260)	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C8-V1 (0.000-0.275)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C9-V1 (0.000-0.840)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C10-V1 (0.000-0.200)	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C11-V1 (0.000-0.490)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C12-V1 (0.000-1.500)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,03
C13-V1 (0.000-0.500)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C14-V1 (0.000-0.310)	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C14-V1 (0.000-0.310)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C15-V1 (0.000-0.490)	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C16-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C18-V1 (0.000-0.300)	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C19-V1 (0.000-0.810)	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C20-V1 (0.000-0.400)	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,06
C22-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(6.12)	0,00
C23-V1 (0.000-0.700)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C24-V1 (0.000-0.100)	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04

UNITY CHECK FOR STRUCTURAL MEMBERS NEN-EN1993-1-1:2016/NB:2016

Label	Check Type	Combination	Formula	UC max
C1	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C2	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,01
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C3	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,04

Label	Check Type	Combination	Formula	UC max
C4	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,03
C5	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C6	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,00
C7	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C8	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
C9	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,01
C10	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,00
C11	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C12	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,02
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C13	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,07
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C14	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C15	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C16	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.2	NEN-EN1993-1-1(6.61&6.62)	0,03
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C18	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C19	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.1	NEN-EN1993-1-1(6.61&6.62)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C20	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,06
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00

	Pole 4	
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Label	Check Type	Combination	Formula	UC max
C22	Stab.	Pe.C.3	NEN-EN1993-1-1(6.46)	0,00
	Stab.	Pe.C.3	NEN-EN1993-1-1(6.61&6.62)	0,02
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
	Cross	Pe.C.2	NEN-EN1993-1-1(6.12)	0,00
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C23	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00
C24	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
	Lat. Buck.	Pe.C.3	NEN-EN1993-1-1(6.54)	0,00

EXTREME UNITY CHECK FOR STRUCTURAL MEMBERS NEN-EN1993-1-1:2016/NB:2016

Label	Check Type	Combination	Formula	UC max
C1	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C2	Cross	Pe.C.3	NEN-EN1993-1-1(6.31)	0,01
C3	Cross	Pe.C.3	NEN-EN1993-1-1(6.1)	0,10
C4	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C5	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C6	Cross	Pe.C.2	NEN-EN1993-1-1(6.31)	0,01
C7	Cross	Pe.C.2	NEN-EN1993-1-1(6.1)	0,09
C8	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,02
C9	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,02
C10	Cross	Pe.C.1	NEN-EN1993-1-1(6.12)	0,00
C11	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,10
C12	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,03
C13	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,10
C14	Cross	Pe.C.1	NEN-EN1993-1-1(6.9)	0,00
C14	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C15	Cross	Pe.C.1	NEN-EN1993-1-1(6.5)	0,00
C16	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,03
C18	Cross	Pe.C.3	NEN-EN1993-1-1(NB.33)	0,03
C19	Stab.	Pe.C.1	NEN-EN1993-1-1(6.46)	0,00
C20	Cross	Pe.C.1	NEN-EN1993-1-1(NB.33)	0,06
C22	Cross	Pe.C.2	NEN-EN1993-1-1(6.12)	0,00
C23	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04
C24	Cross	Pe.C.2	NEN-EN1993-1-1(NB.33)	0,04